

REFERENCE KEYNOTES

DIVISION 01 - GENERAL REQUIREMENTS

GENERAL NOTES

A. ALL CASEWORK TO HAVE PLASTIC LAMINATE FINISH, U.N.O.

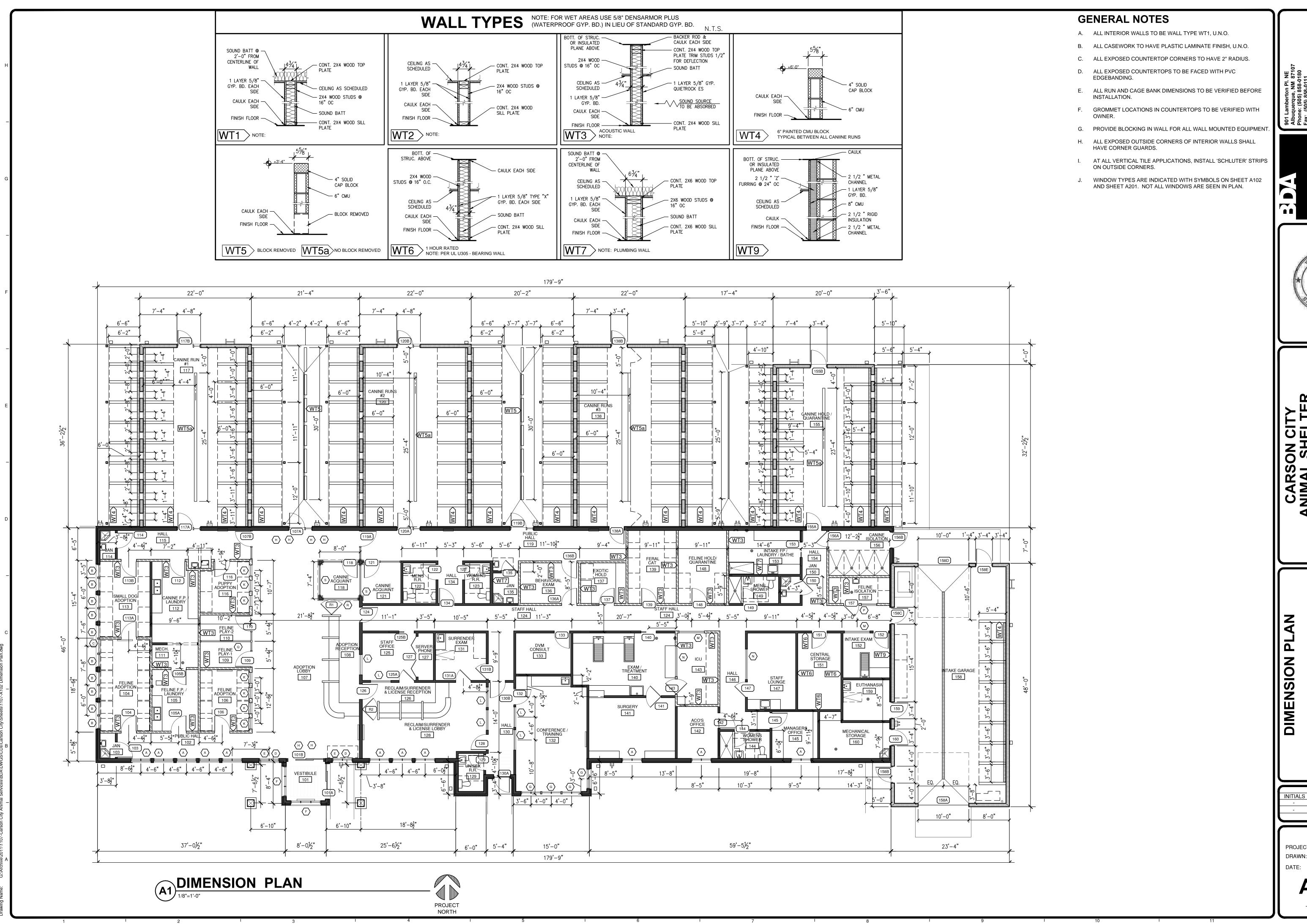
B. ALL EXPOSED COUNTERTOP CORNERS TO HAVE 2" RADIUS.

BDA DSGN. REV. BDA TECH REV.

PROJECT NO.: 1107 DRAWN: DATE: 2/27/2015

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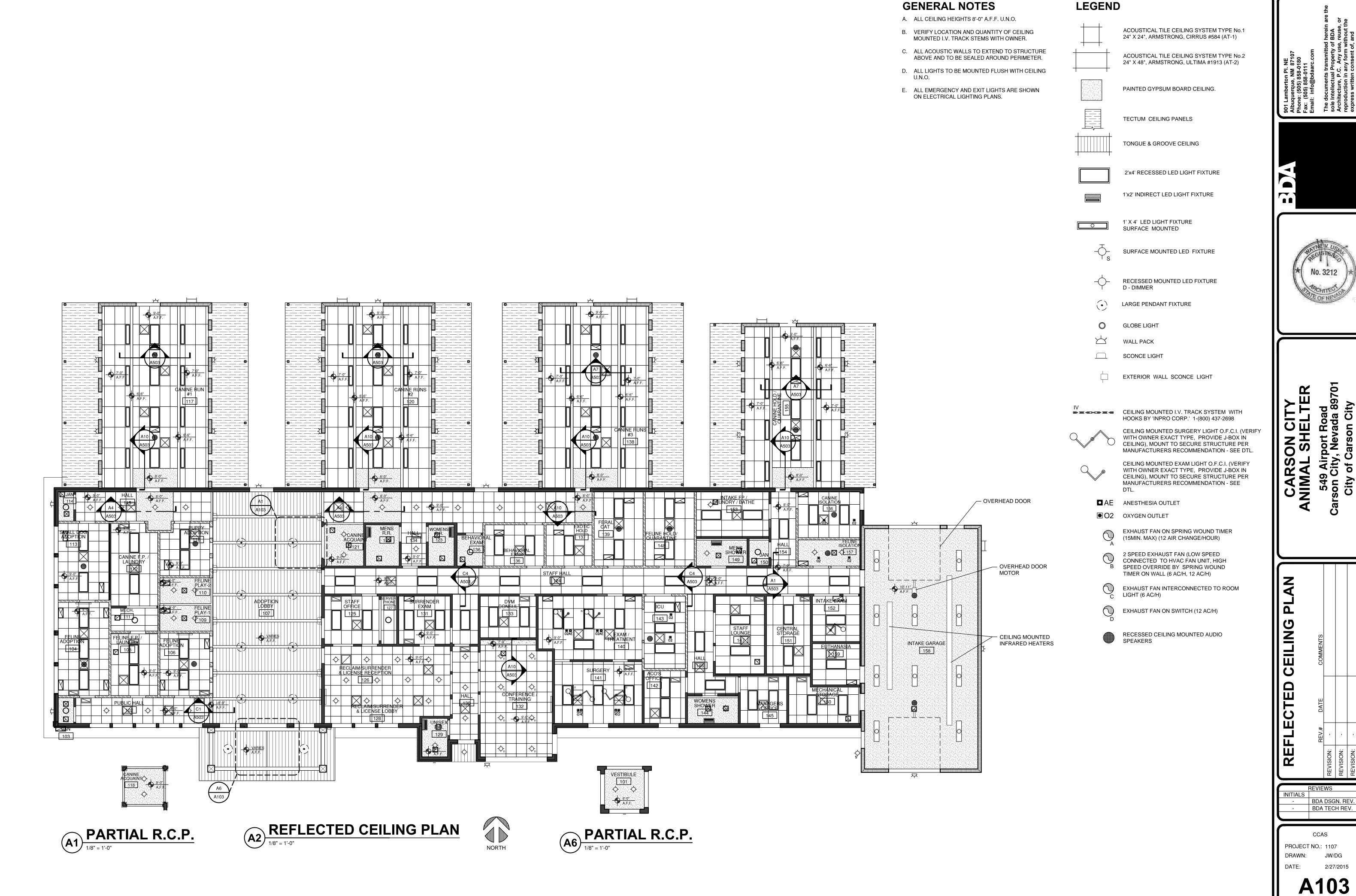
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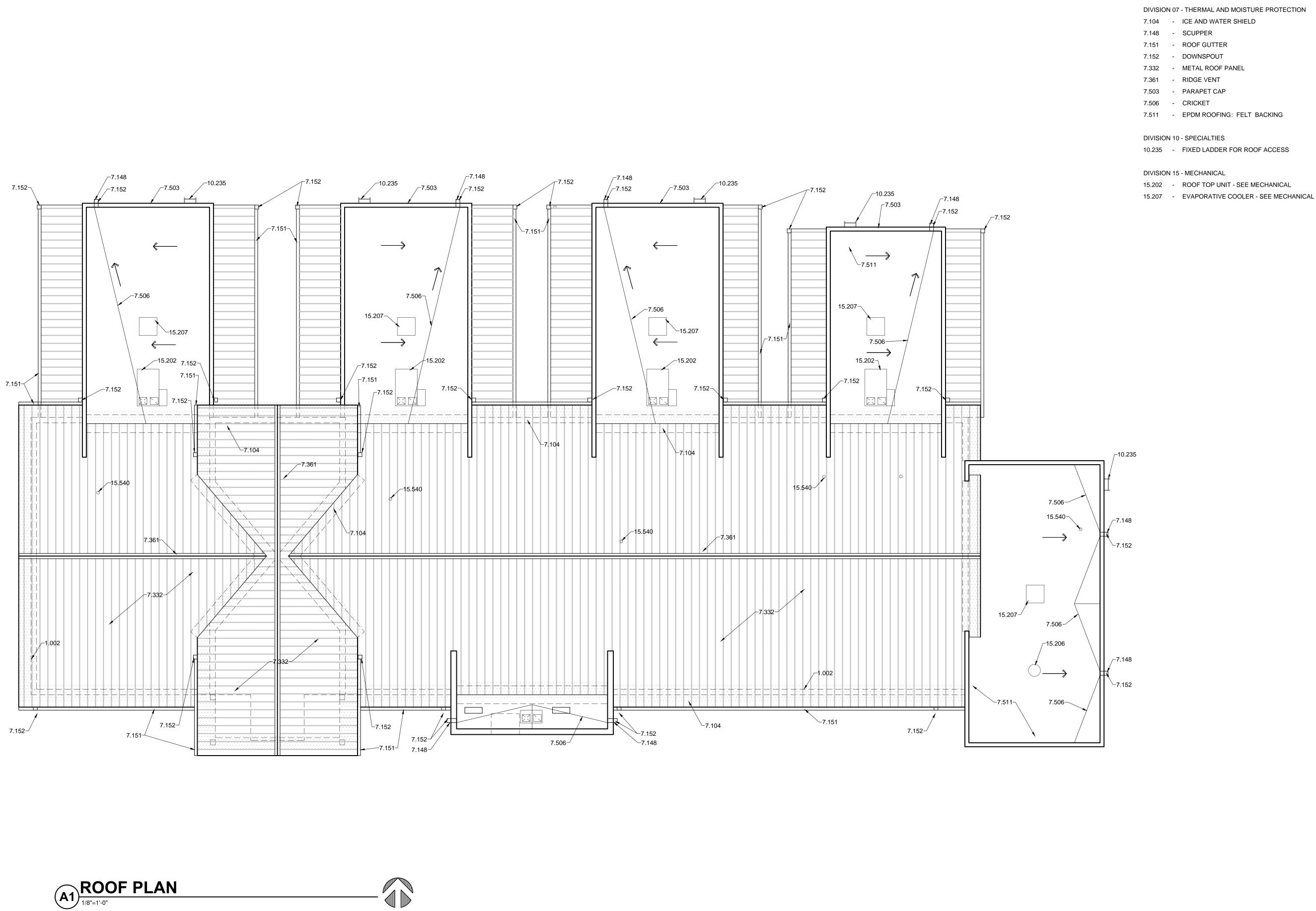
BDA DSGN. REV. BDA TECH REV.

PROJECT NO.: 1107





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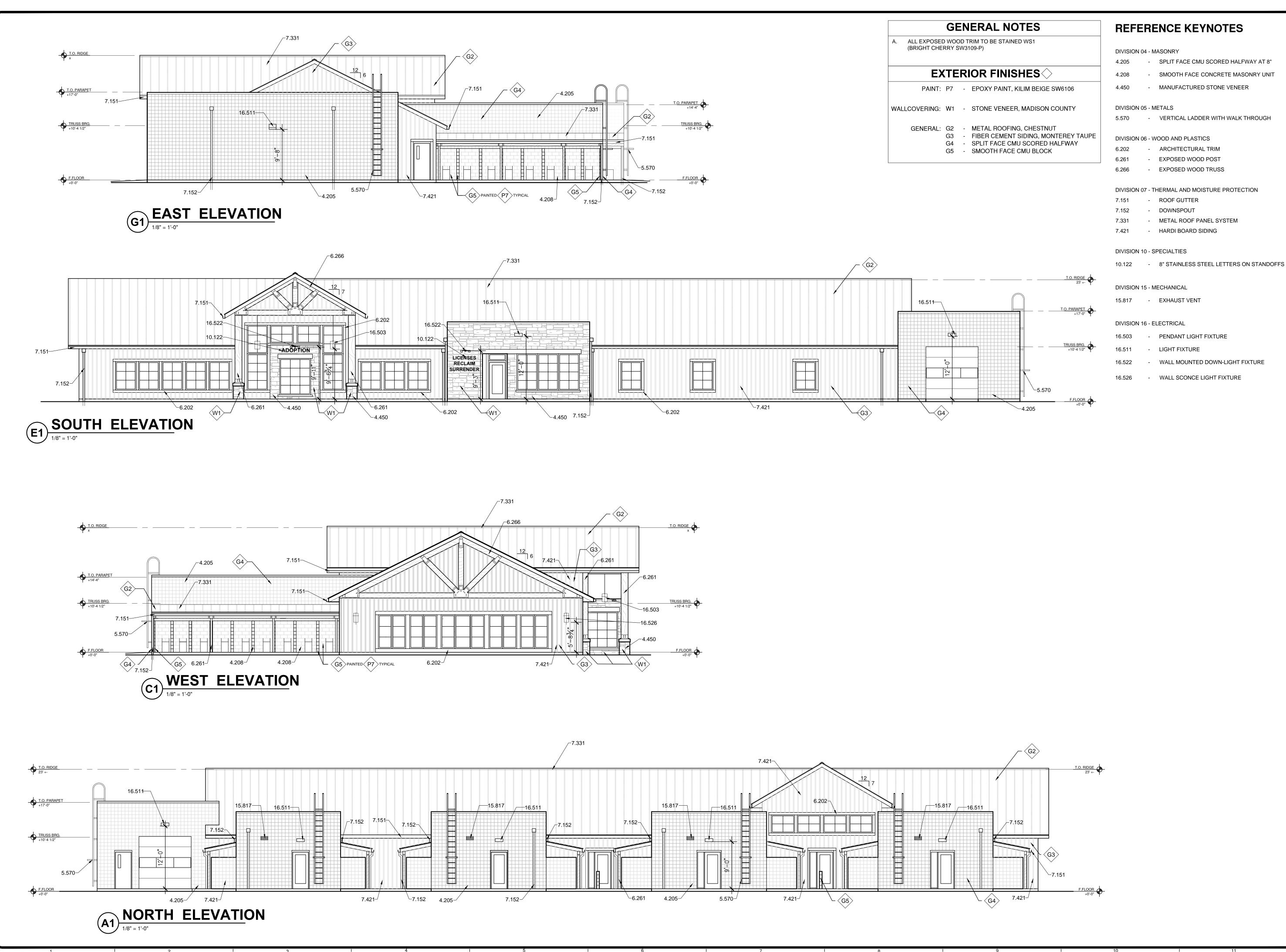


REFERENCE KEYNOTES

DIVISION 01 - GENERAL REQUIREMENTS

1.002 - LINE OF ADJACENT WALL

BDA DSGN. REV. BDA TECH REV.



DIVISION 07 - THERMAL AND MOISTURE PROTECTION

WALL MOUNTED DOWN-LIGHT FIXTURE

MANUFACTURED STONE VENEER

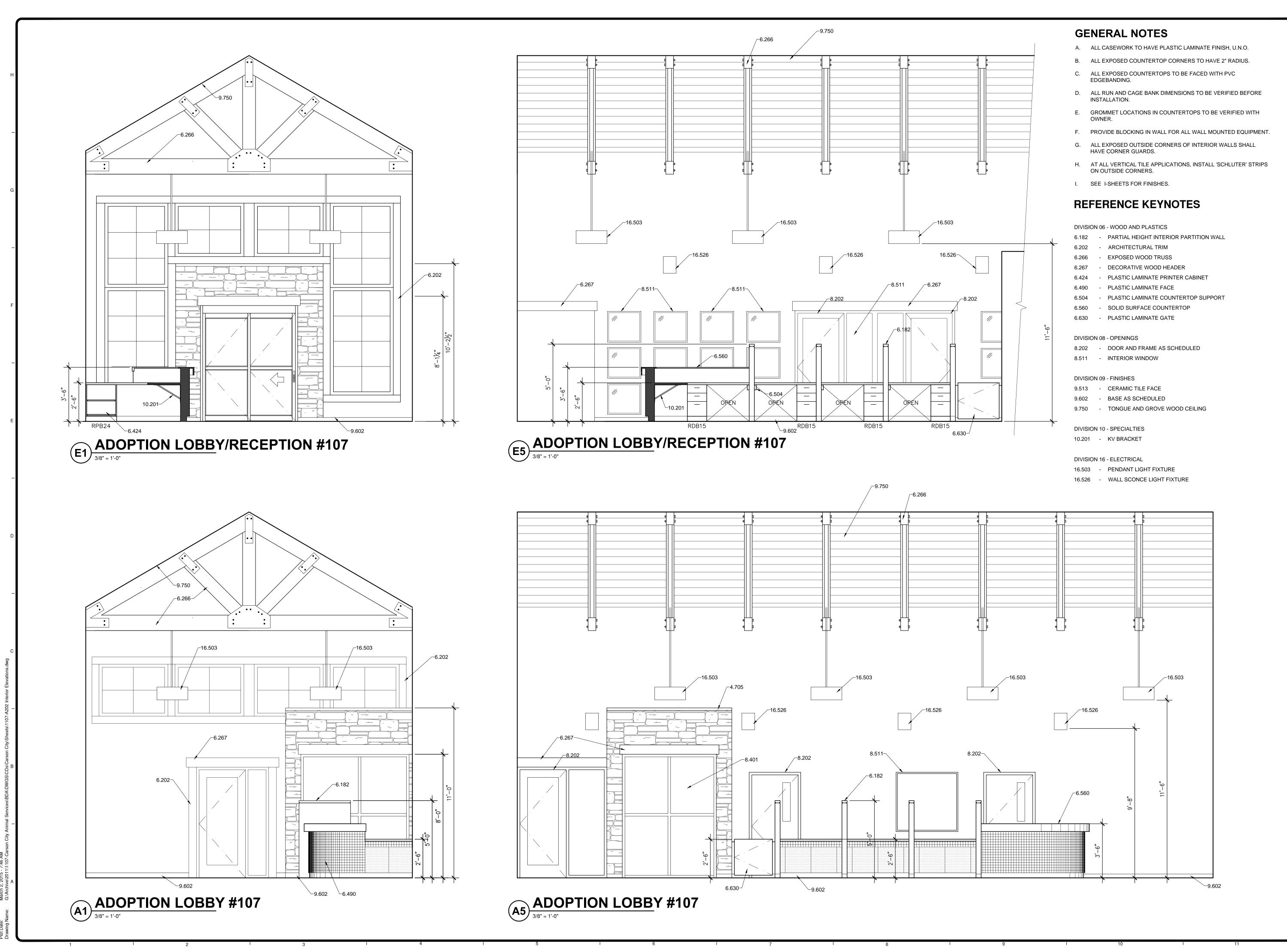
5.570 - VERTICAL LADDER WITH WALK THROUGH

EXTERIOR

BDA DSGN. REV. BDA TECH REV.

PROJECT NO.: 1107 DRAWN: DG/JW

DATE: 2/27/2015

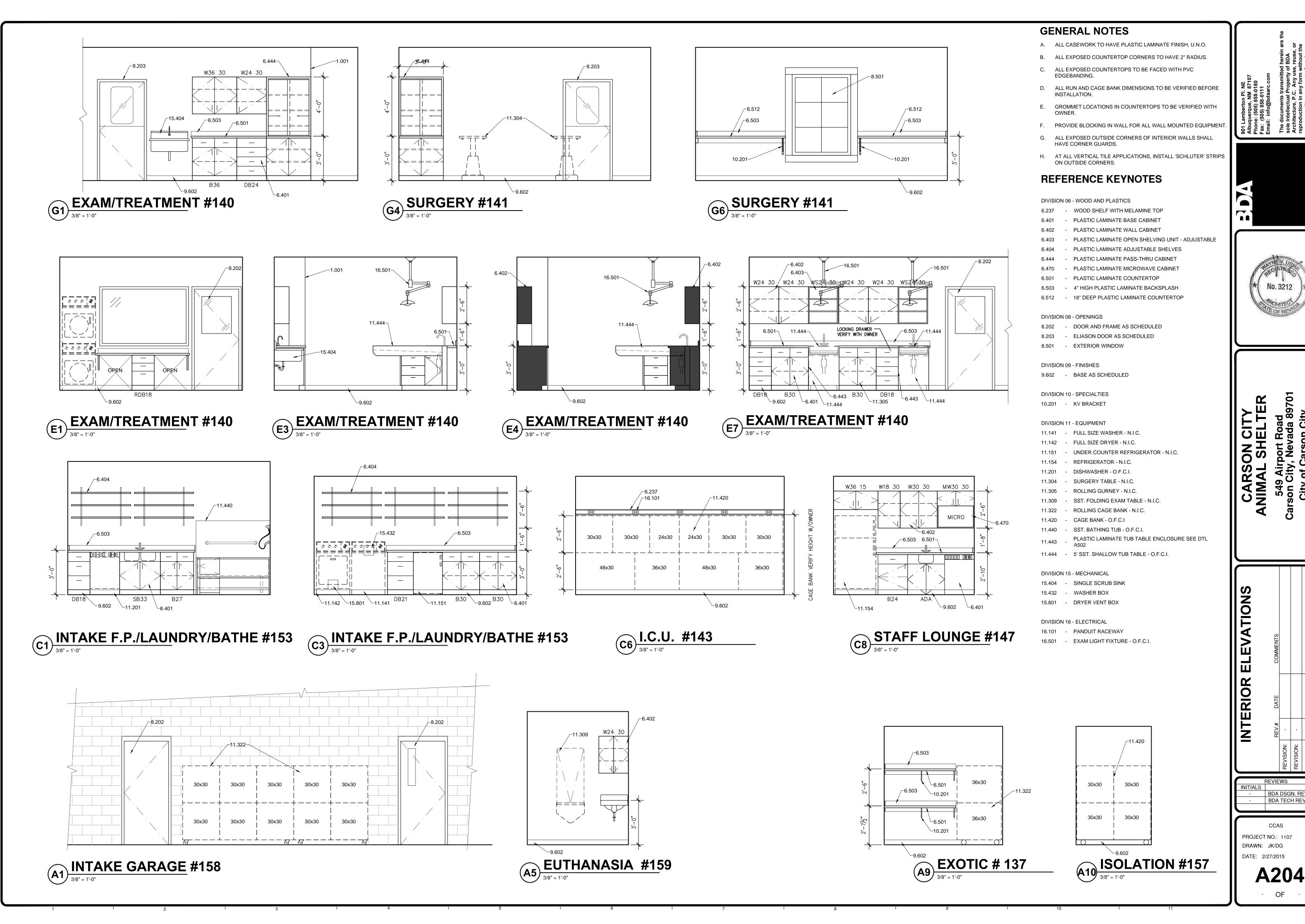


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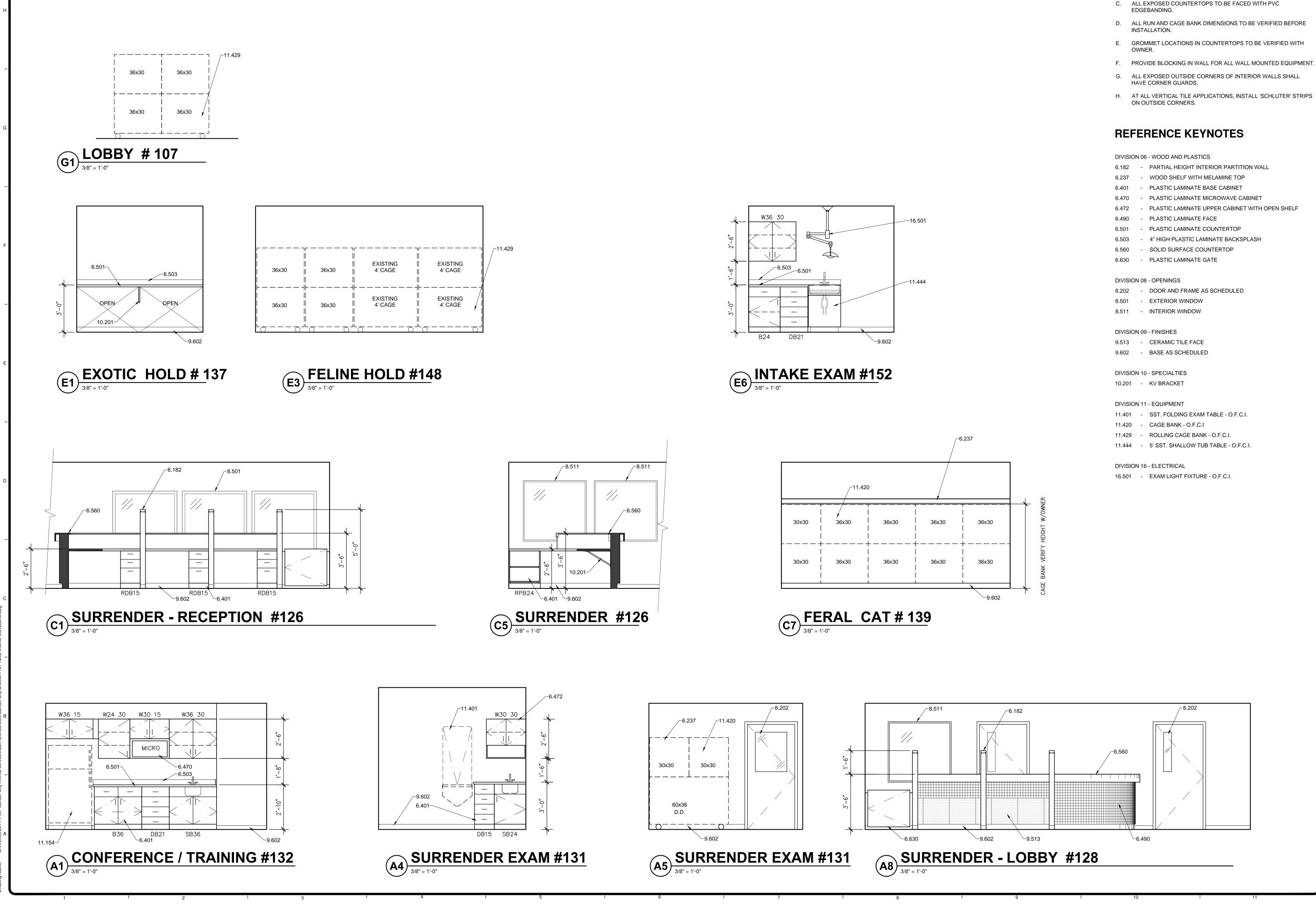
INTERIOR

BDA DSGN. REV. BDA TECH REV.





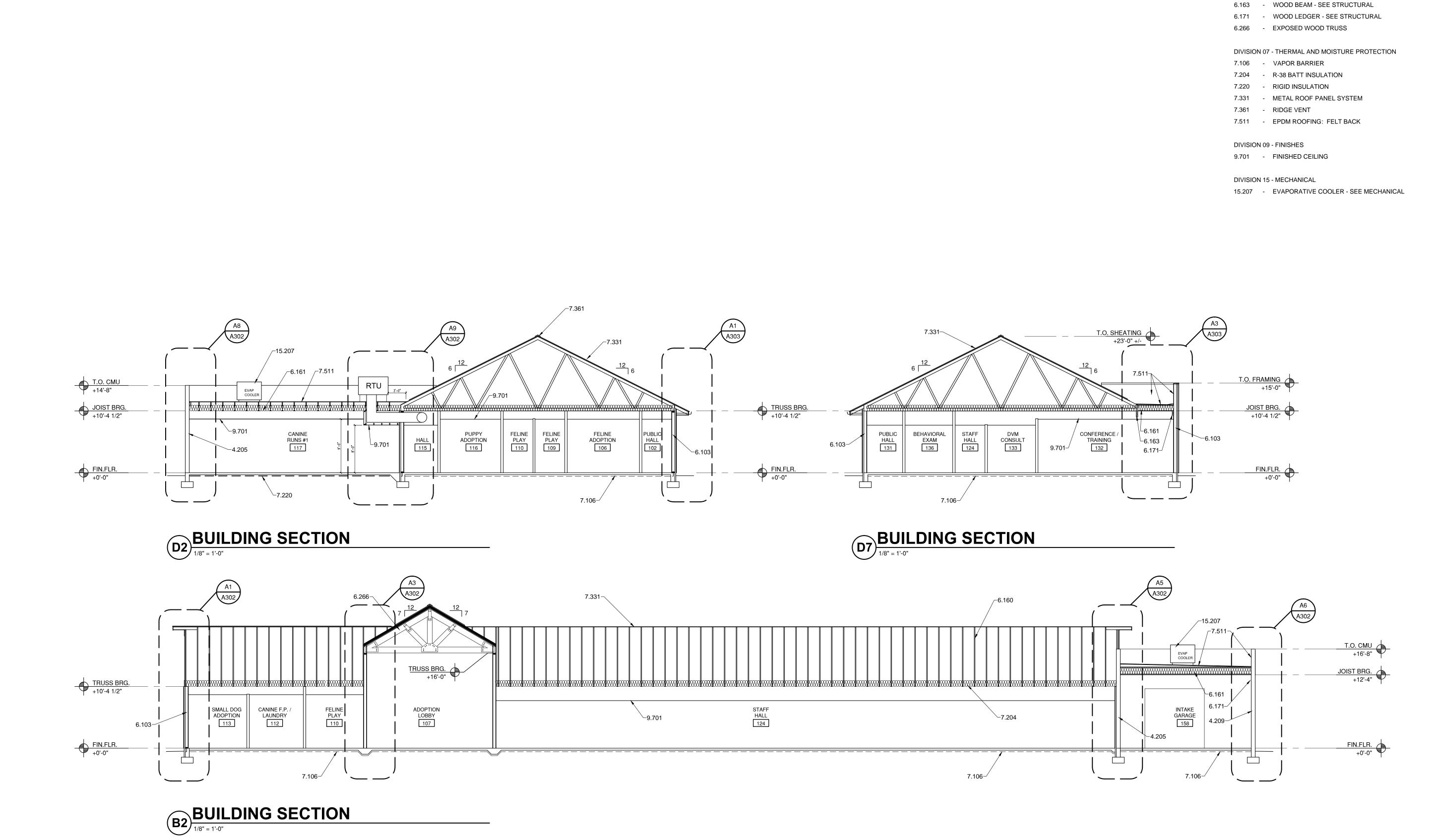
BDA DSGN. REV. BDA TECH REV.



GENERAL NOTES A. ALL CASEWORK TO HAVE PLASTIC LAMINATE FINISH, U.N.O. B. ALL EXPOSED COUNTERTOP CORNERS TO HAVE 2" RADIUS.

INTERIOR

BDA DSGN. REV. BDA TECH REV.



REFERENCE KEYNOTES

DIVISION 04 - MASONRY

4.205 - 8" CONCRETE MASONRY UNIT

4.209 - SPLIT FACE CONCRETE MASONRY UNIT

DIVISION 06 - WOOD AND PLASTICS 6.103 - 2X6 WOOD STUD

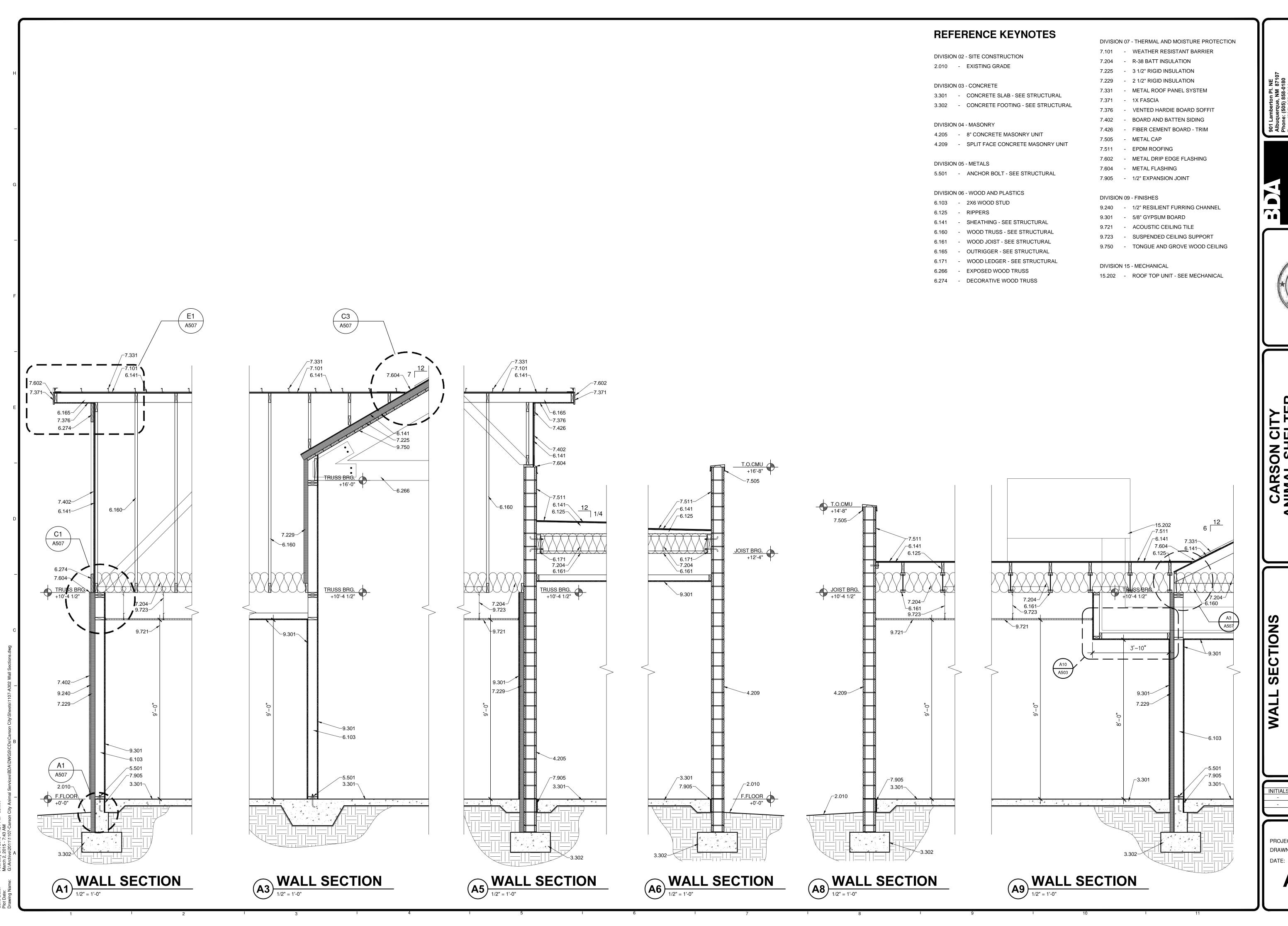
WOOD TRUSS - SEE STRUCTURAL

WOOD JOIST - SEE STRUCTURAL

BUILDING SECTIONS

BDA DSGN. REV. BDA TECH REV.

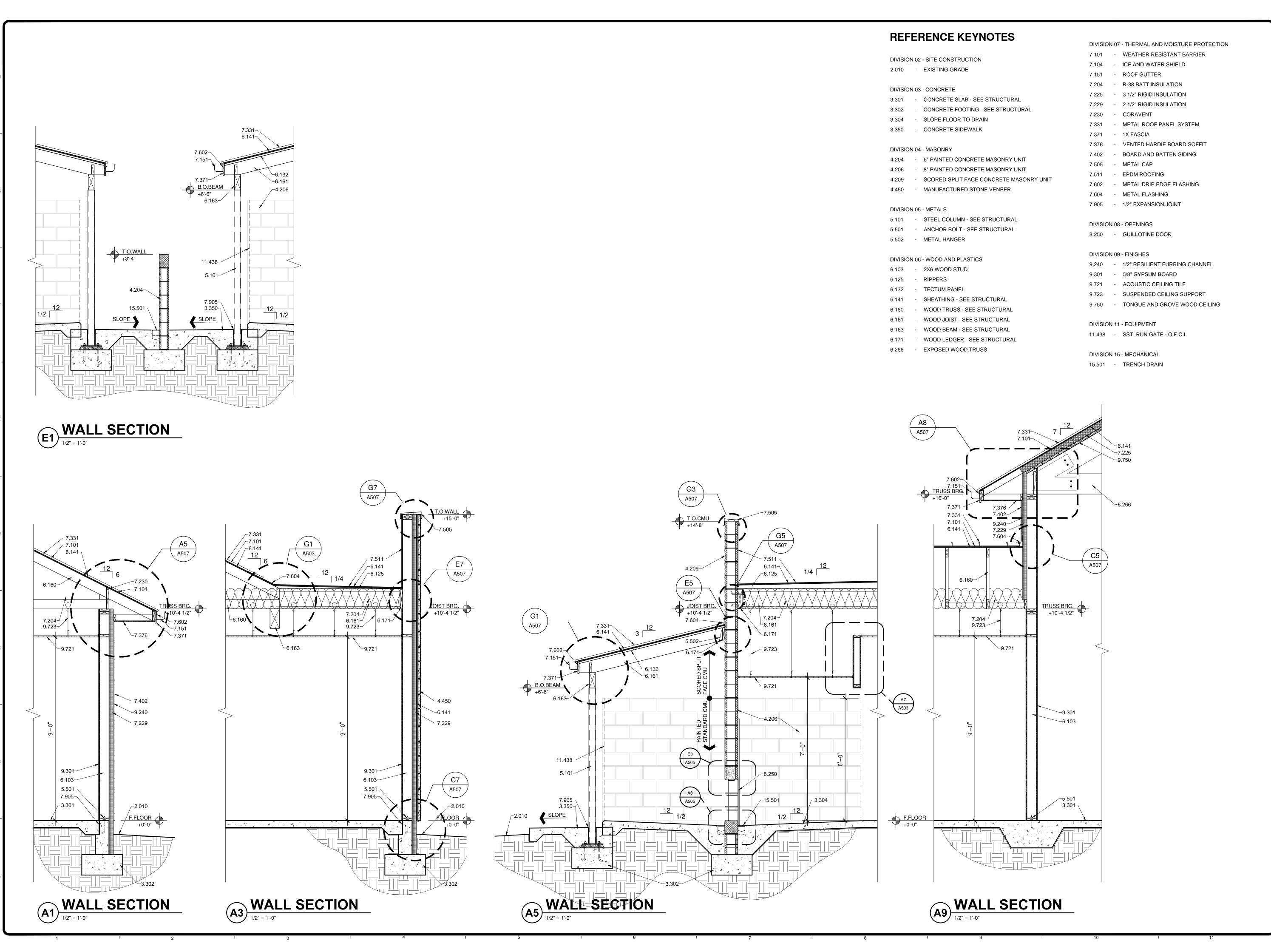
PROJECT NO.: 1107



CAI ANIM, 549 Carson (City

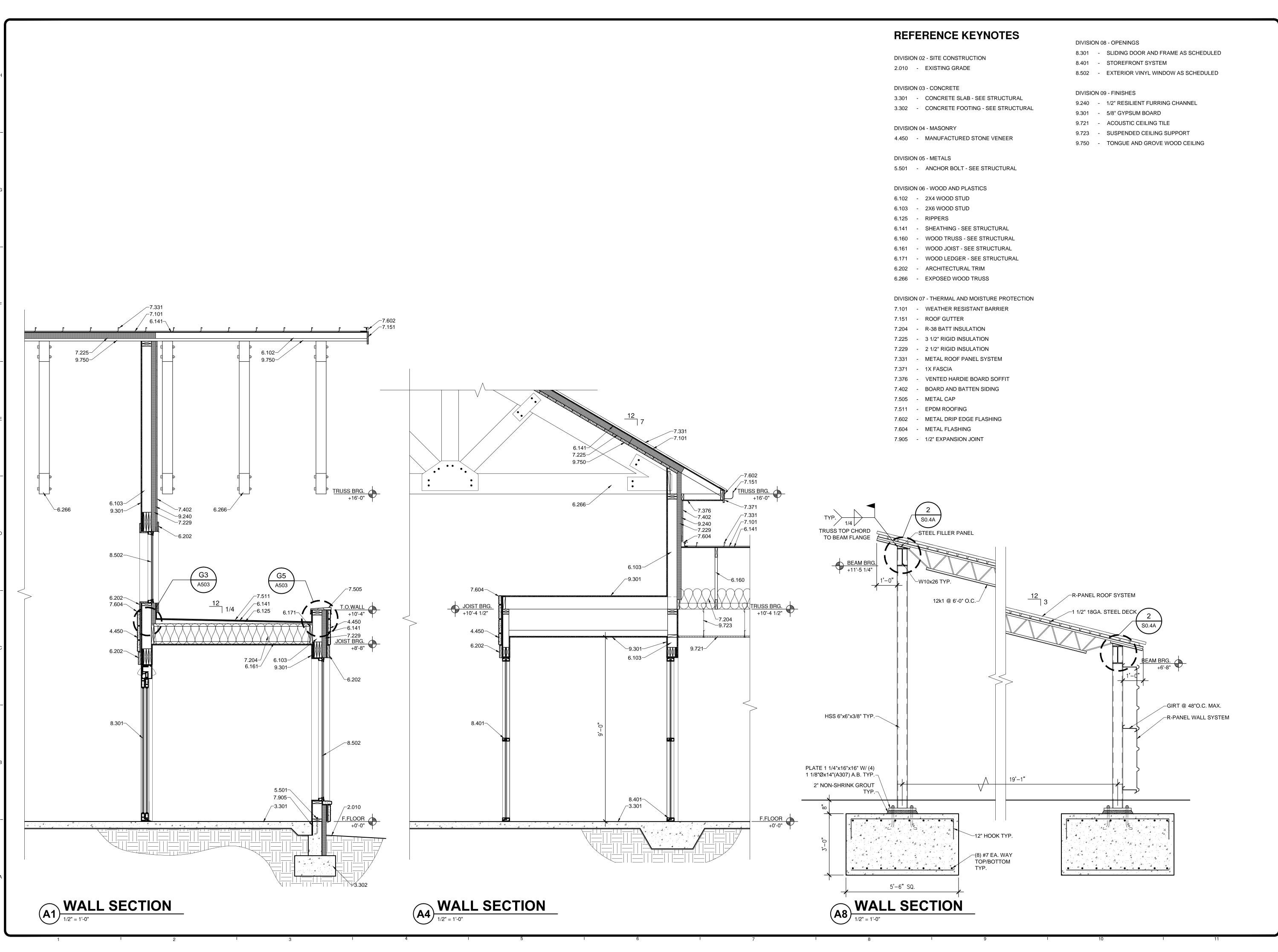
PROJECT NO.: 1107

BDA DSGN. REV. BDA TECH REV.



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PROJECT NO.: 1107



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9 Airport Road

ANIMAL SHE 549 Airport Ro Carson City, Nevad

MALL SECTIONS

REV.# DATE COMMENTS

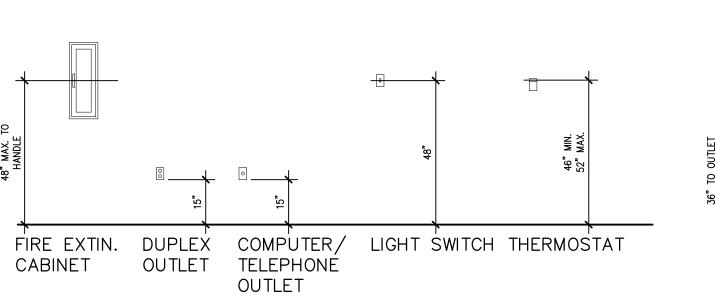
DN: - COMMENTS

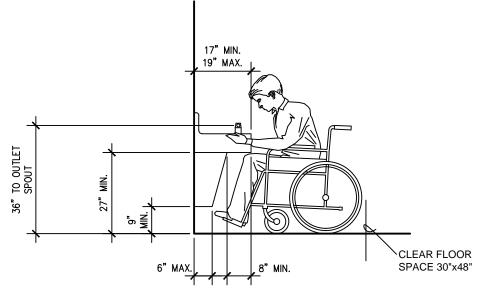
REVIEWS
TIALS
- BDA DSGN. REV.
- BDA TECH REV.

CCAS
PROJECT NO.: 1107
DRAWN: -

A304

Save Date: March 2, 201 Plot Date: March 2, 201





SIDE ELEVATION DRINKING FOUNTAIN

/ EMT CONDUIT @ 12'-0" O.C. MAX, 6'-0" MAX. FROM WALL EXTEND TO

STRUT SCHEDULE

EMT SIZE (NOMINAL) LENGTH (L/r≤ 200)

1 1/4"

1 1/2"

≤ 3'−10"

≤ 5'-0"

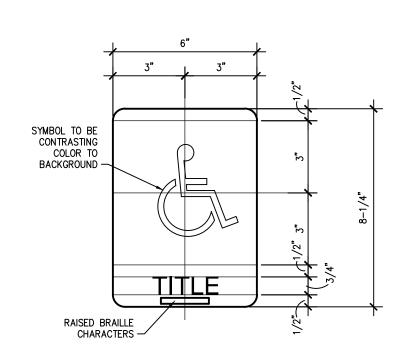
< 6'-6"

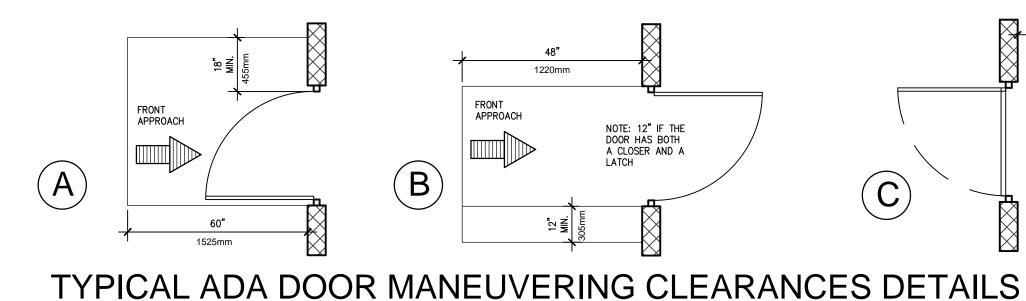
≤ 8'−6"

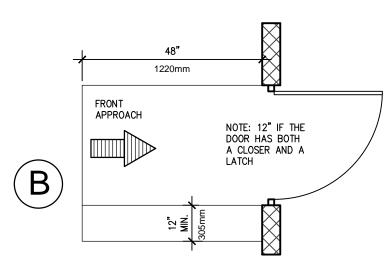
≤ 9'−10"

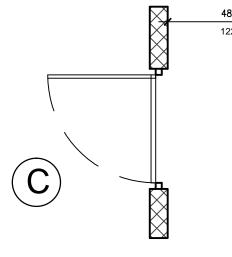
STRUCTURE ABOVE

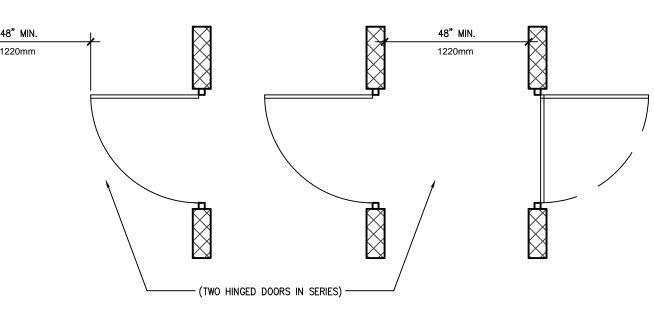
TYPICAL ADA RESTROOM AND MECHANISM MOUNTING HEIGHTS

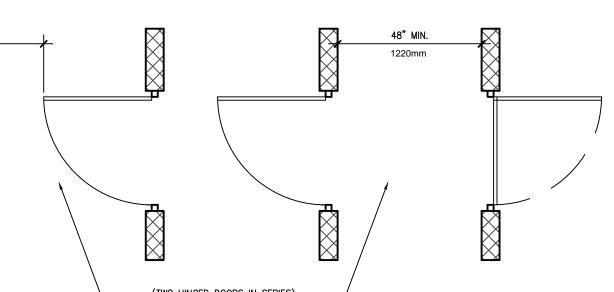




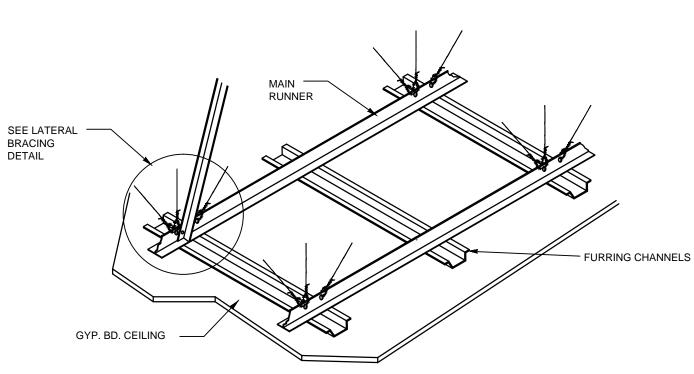


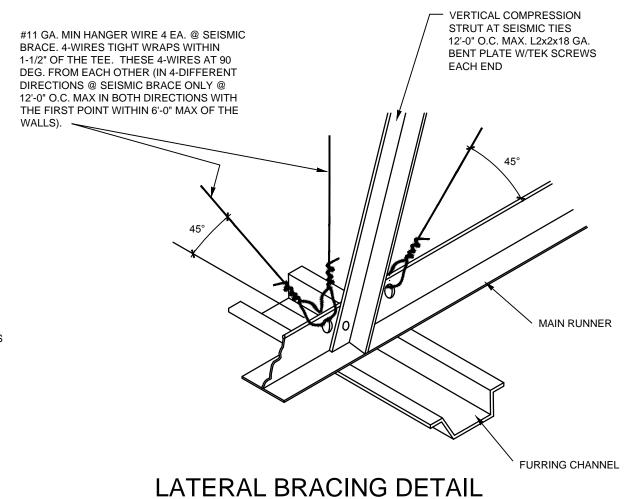




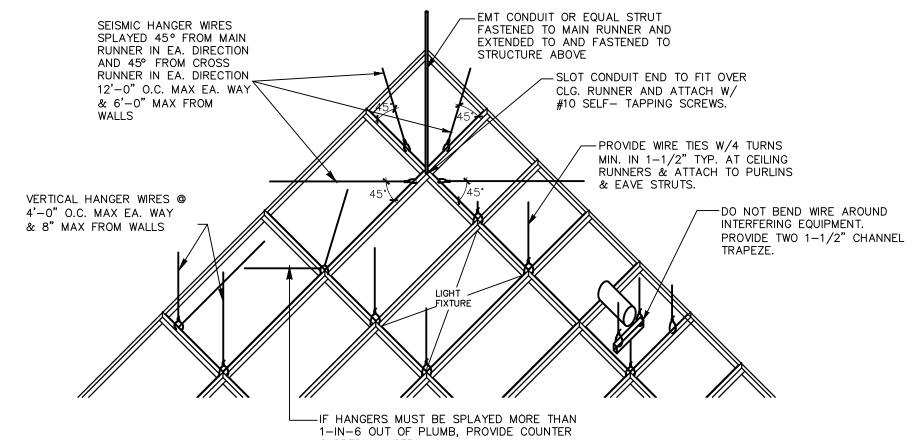


TYP. INT. SYMBOL SIGN





TYPICAL SUSPENDED GYP. BD. BRACING DETAIL

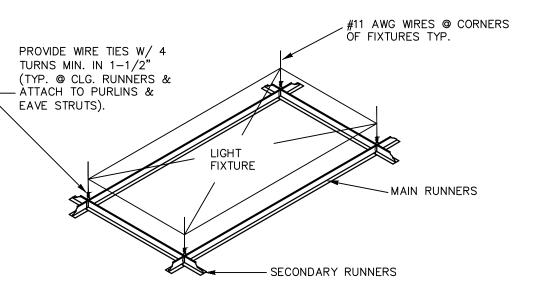


SLOPED HANGER.

- HANGER WIRES MUST BE NO. 12 GA. MIN. SOFT-ANNEALED, MILD STEEL WIRE. WIRES SHALL BE WRAPPED TIGHTLY AT RUNNERS AND SUPPORTS AND TIED WITH A MIN. OF 3 TURNS.
- 2. CLIPS SHALL HOLD RUNNERS TIGHTLY TOGETHER. SPLICE CLIPS SHALL BE CAPABLE OF RESISTING AT LEAST 50 LBS. IN TENSION OR COMPRESSION.
- 3. FOR MANUFACTURED SYSTEMS, SUBMIT SUBSTAN-TIATION IN ACCORDANCE WITH CURRENT BUILDING
- 4. RUNNERS SHALL BE CAPABLE OF SUPPORTING CEILING SYSTEM WITH DEFLECTIONS LESS THAN
- 5. LOCAL KINKS AND BENDS SHALL NOT BE MADE IN HANGER WIRES FOR LEVELING.

/#12 @ 48" O.C. /MAX EA. WAY ALL 8" (MAX) MAIN RUNNERS -CONNECTION DETAIL - MAIN OR CROSS - MAIN RUNNER - MAIN RUNNER WITHIN RUNNER 2" OF CROSS RUNNER

SUSPENDED CEILING



#12 SPLAY WIRE 4 DIRECTIONS

FROM WALL

AT 12'-0" O.C. MAX, 6'-0" MAX

SECONDARY RUNNERS			
FIXTURE SUSPENSION			
MAIN RUNNER SECONDARY RUNNER ATTACH TABS	ANCHOR TO STUD & ALL CORNERS	WALL ANGLE MOLDING (1) POP F RUNNER (SIDES ONI	HANGER WIRE MAIN RUNNER RIVET © EA. 2 ADJACENT -Y)

BEND DOWN (90 DEGREE **RUNNER ATTACHMENT**

T-BAR MAIN/CROSS MEMBER TO WALL CONNECTION

TYPICAL SUSPENDED CEILING BRACING DETAIL

GENERAL NOTES:

- 1. IF THERE ARE ANY CONFLICTS BETWEEN INFORMATION PROVIDED ON THIS SHEET AND INFORMATION PROVIDED ON OTHER CONTRACT DOCUMENTS, THE ARCHITECT IS TO BE NOTIFIED IMMEDIATELY.
- THESE ARE TO BE CONSIDERED MINIMUM REQUIREMENTS. LOCAL ORDINANCES MAY HAVE REQUIREMENTS CONTRADICTING INFORMATION CONTAINED ON THIS SHEET. THIS IS THE CASE, THE ARCHITECT IS TO BE NOTIFIED IMMEDIATELY. IN ANY CASE, THE LOCAL ORDINANCE REQUIREMENTS APPLY.
- SEISMIC BRACING REQUIREMENTS AND DETAILS ARE BASED ON THE INTERNATIONAL BUILDING CODE. FOR JURISDICTIONS NOT COVERED BY THE IBC, LOCAL REQUIREMENTS WILL APPLY.

DOORS:

- CLEAR WIDTH: DOORWAY SHALL HAVE A CLEAR OPENING OF 32" (815mm) MINIMUM WITH DOOR OPEN 90 DEG. CLÈAR OPÉNING SHALL BE MEASURED BETWEEN THE FACE OF DOOR AND STOP (ANSI 4.13.5)
- MANEUVERING CLEARANCES AT DOORS: FRONT APPROACHES TO PULL SIDE OF SWINGING DOORS SHALL HAVE MANEUVERING SPACE THAT EXTENDS 18" (455mm) MINIMUM BEYOND THE LATCH SIDE OF THE DOOR AND 60" (1525mm) MINIMUM PERPENDICULAR TO THE DOORWAY, SEE DTL. A THIS SHEET. (ANSI 4.13.6.1)
- FRONT APPROACHES TO PUSH SIDE OF SWINGING DOORS, EQUIPPED WITH BOTH CLOSER AND LATCH, SHALL HAVE MANEUVERING SPACE THAT EXTENDS 12" (305mm) MINIMUM BEYOND THE LATCH SIDE OF THE DOOR AND 48" (1220mm) MINIMUM PERPENDICULAR TO THE DOORWAY, SEE DTL B THIS SHEET. (ANSI 4.13.6.2)
- FRONT APPROACHES TO PUSH SIDE OF SWINGING DOORS, NOT EQUIPPED WITH LATCH AND CLOSER, SHALL HAVE MANEUVERING SPACE THAT IS THE SAME WIDTH AS DOOR OPENING AND EXTENDS 48" (1220mm) MINIMUM PERPENDICULAR TO THE DOORWAY, SEE DTL E THIS SHEET. (ANSI 4.13.6.3)
- TWO DOORS IN SERIES: DOORS IN SERIES SHALL BE 48" (1220mm) PLUS THE WIDTH OF ANY DOOR SWINGING INTO THE SPACE. DOORS IN SERIES SHALL SWING EITHER IN SAME DIRECTION OR AWAY FROM SPACE BETWEEN DOORS, SEE DTL C THIS SHEET. (ANSI 4.13.7)
- THRESHOLDS AT DOORWAYS: THRESHOLDS, IF PROVIDED, AT DOORWAYS SHALL BE 1/2" (13mm) HIGH MAXIMUM EXCEPT THAT THRESHOLDS FOR EXTERIOR RESIDENTIAL SLIDING DOORS SHALL BE 3/4" (19mm) HIGH MAXIMUM. (ANSI 4.13.8)

DOOR HARDWARE:

HANDLES, PULLS, LATCHES, LOCKS, AND OTHER OPERABLE PARTS ON ACCESSIBLE DOORS SHALL HAVE A SHAPE THAT IS EASY TO GRASP WITH ONE HAND AND DOES NOT REQUIRE TIGHT GRASPING, TIGHT PINCHING, OR TWISTING OF THE WRIST TO OPERATE. (ANSI 4.13.9) [GENERALLY LEVER HANDLE STYLE]

DOOR CLOSERS:

N.T.S.

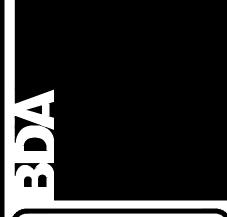
DOOR CLOSERS SHALL BE ADJUSTED SO THAT FROM AN OPEN POSITION OF 90 DEG., THE TIME REQUIRED TO MOVE THE DOOR TO AN OPEN POSITION OF 12 DEG. WILL BE 5 SECONDS MINIMUM. (ANSI 4.13.10)

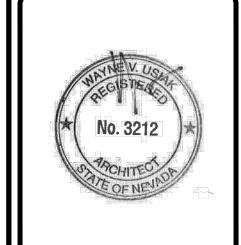
DOOR-OPENING FORCE:

- FIRE DOORS SHALL HAVE THE MINIMUM OPENING FORCE ALLOWABLE BY THE APPROPRIATE ADMINISTRATIVE AUTHORITY. THE REQUIRED FORCE FOR PUSHING OR PULLING OPEN DOORS OTHER THAN FIRE DOORS SHALL BE AS FOLLOWS
- -INTERIOR HINGED DOOR: 5.01b. (22.2N) MAX. -SLIDING/FOLDING DOOR: 5.0lb. (22.2N) MAX. THESE FORCES DO NOT APPLY TO THE FORCE REQUIRED TO RETRACT LATCH BOLTS OR DISENGAGE OTHER DEVICES THAT HOLD THE DOOR IN A CLOSED POSITION. (ANSI 4.13.11)

CHANGES IN LEVEL:

CHANGES IN LEVEL OF 1/4" (6mm) HIGH MAXIMUM SHALL BE PERMITTED TO BE VERTICAL AND WITHOUT EDGE TREATMENT (ANSI 4.5.2.1) CHANGES IN LEVEL BETWEEN 1/4" (6mm) HIGH MINIMUM AND 1/2" (13mm) HIGH MAXIMUM SHALL BE BEVELED WITH A SLOPE NOT STEEPER THAN 1:2. (ANSI 4.5.2.2)





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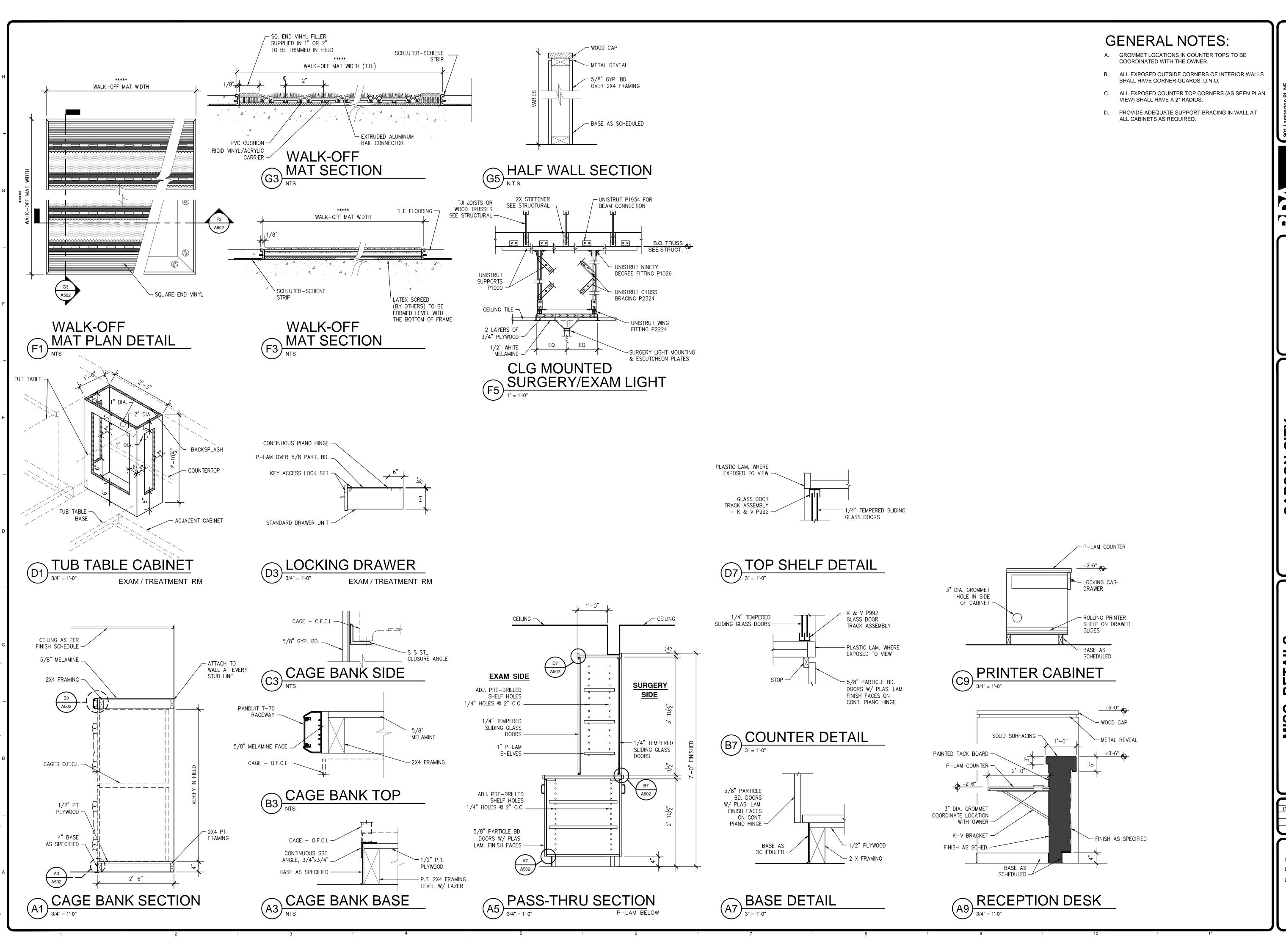
PROJECT NO.: 1107 2/27/2015

A501

TYPICAL REINFORCED SUSPENDED CEILING BRACING DETAIL

N.T.S.

N.T.S.

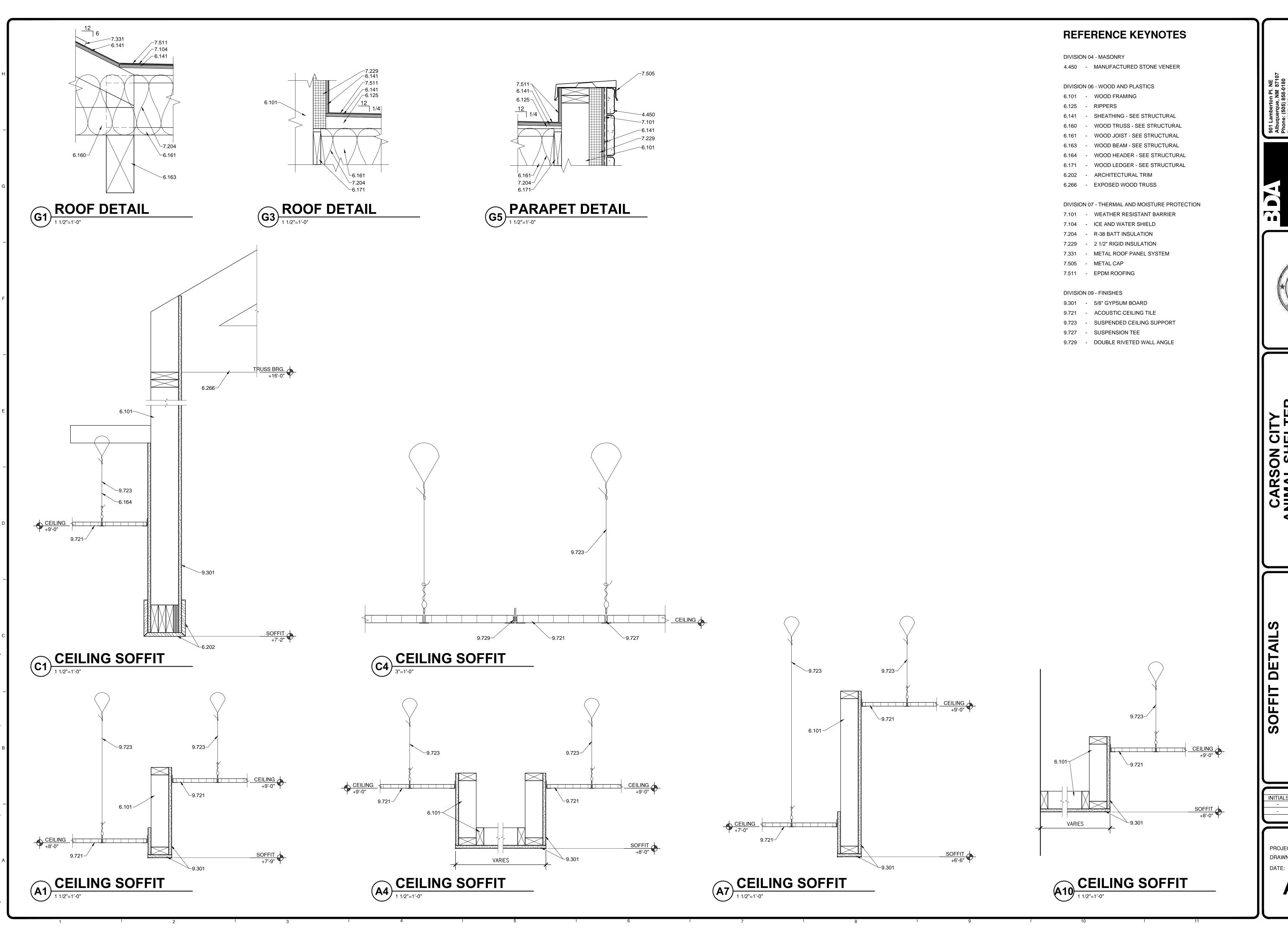




DETAILS MISC

BDA DSGN. REV. BDA TECH REV.

PROJECT NO.: 1107 DRAWN: 2/27/2015



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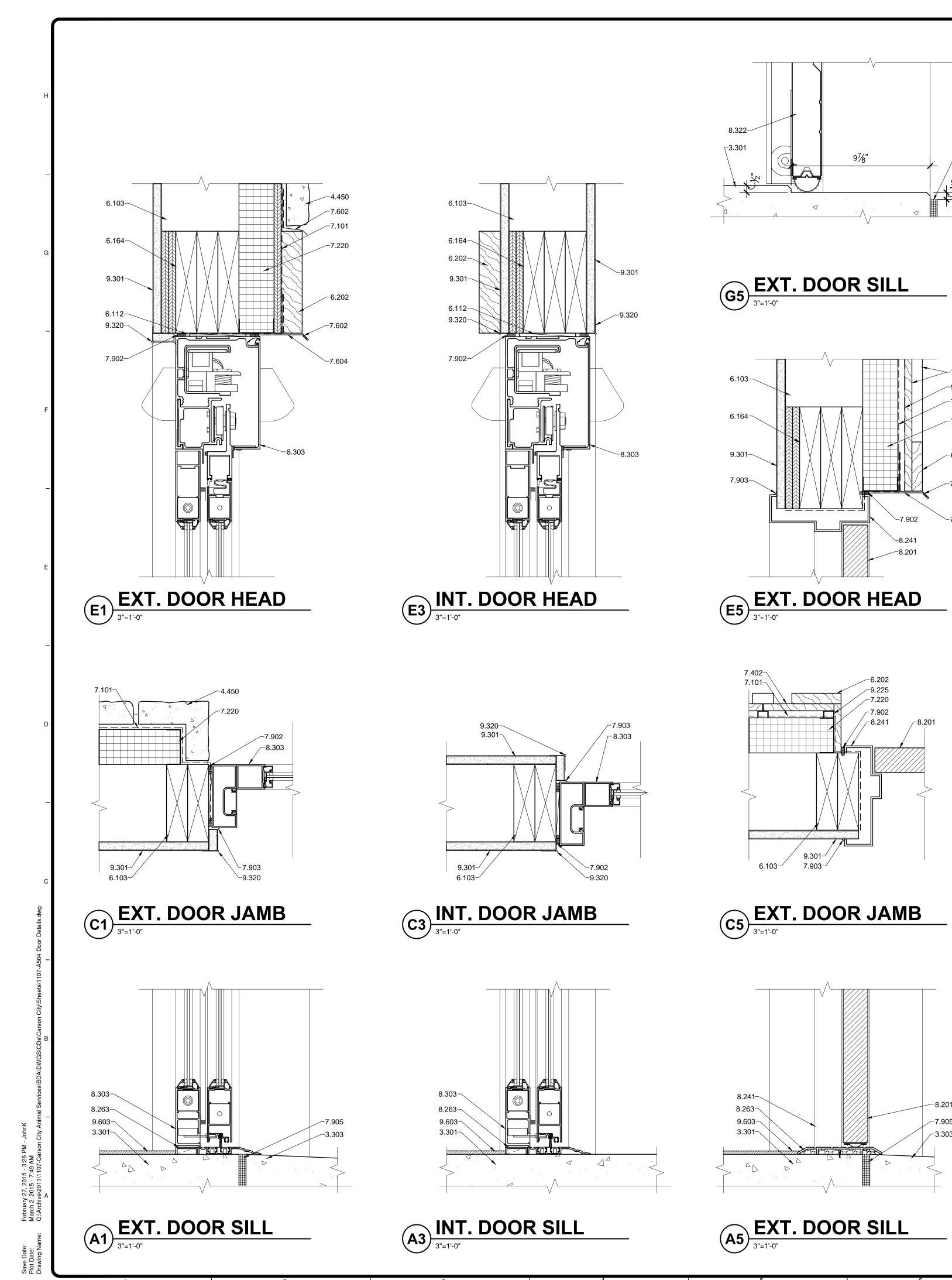
REVIEWS
TIALS
- BDA DSGN. REV.
- BDA TECH REV.

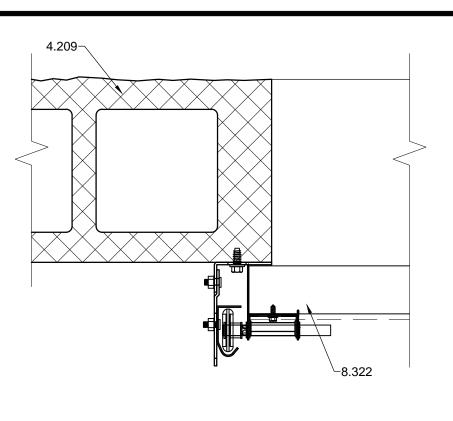
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PROJECT NO.: 1107

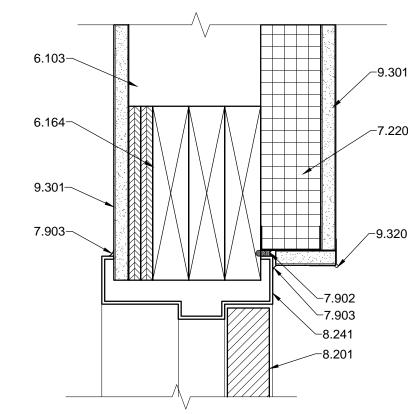
DRAWN: -

ATE: 2/27/2015
A503



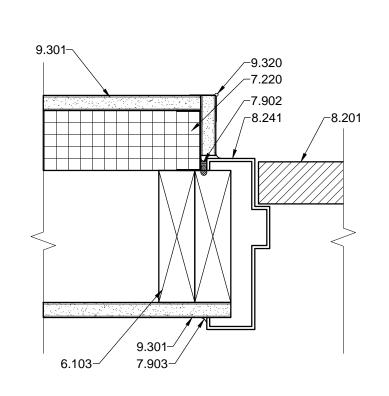


G7 EXT. DOOR JAMB

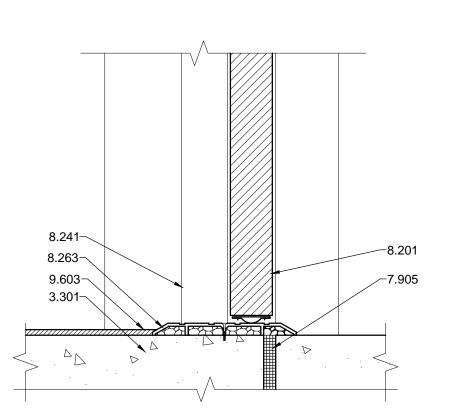


INT. DOOR HEAD

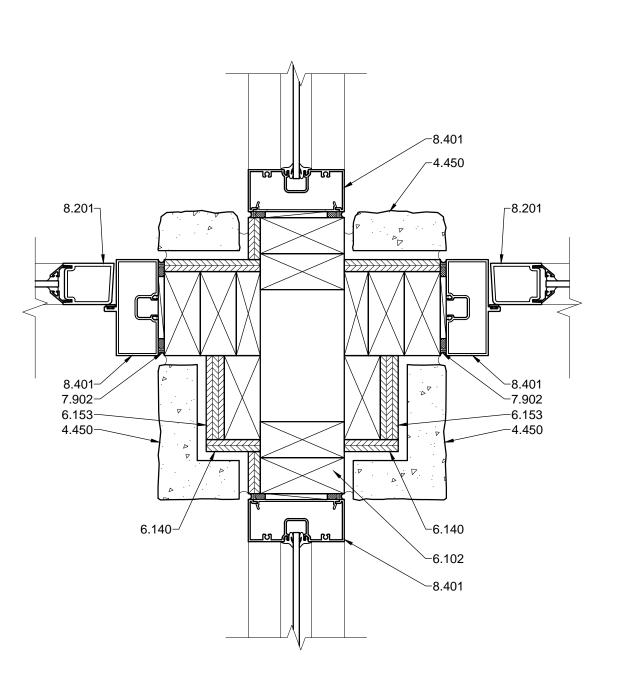
3"=1'-0"



C7 INT. DOOR JAMB



A7 INT. DOOR SILL



A9 INT. DOOR JAMB

REFERENCE KEYNOTES

DIVISION 03 - CONCRETE

3.301 - CONCRETE SLAB - SEE STRUCTURAL

3.303 - CONCRETE STOOP

DIVISION 04 - MASONRY

4.209 - SCORED SPLIT FACE CONCRETE MASONRY UNIT

4.450 - MANUFACTURED STONE VENEER

DIVISION 06 - WOOD AND PLASTICS

6.102 - 2X4 WOOD STUD 6.103 - 2X6 WOOD STUD

6.140 - SHEATHING 6.153 - 3/4" PLYWOOD

WOOD HEADER - SEE STRUCTURAL

ARCHITECTURAL TRIM

DIVISION 07 - THERMAL AND MOISTURE PROTECTION

WEATHER RESISTANT BARRIER

RIGID INSULATION **BOARD AND BATTEN SIDING**

METAL DRIP EDGE FLASHING

METAL FLASHING

BACKER ROD AND SEALANT

7.905 - 1/2" EXPANSION JOINT

DIVISION 08 - OPENINGS

8.201 - DOOR AS SCHEDULED

HOLLOW METAL FRAME AS SCHEDULED

ANCHORED METAL THRESHOLD SET IN BED OF SEALANT

SLIDING DOOR

SECTIONAL OVERHEAD DOOR

8.401 - STOREFRONT SYSTEM

DIVISION 09 - FINISHES

9.225 - METAL FURRING

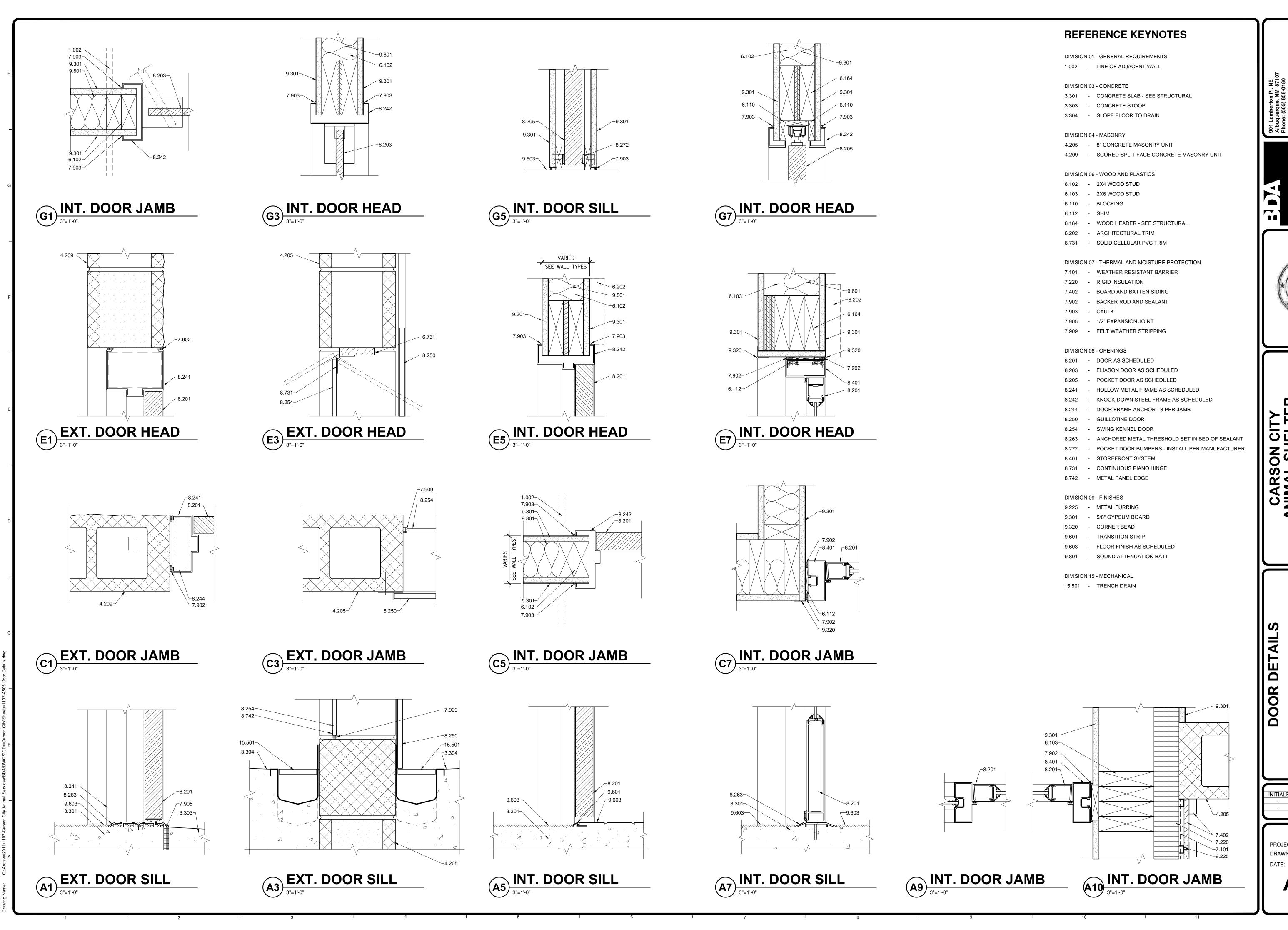
- 5/8" GYPSUM BOARD

9.320 - CORNER BEAD

9.603 - FLOOR FINISH AS SCHEDULED

DOOR DETAILS

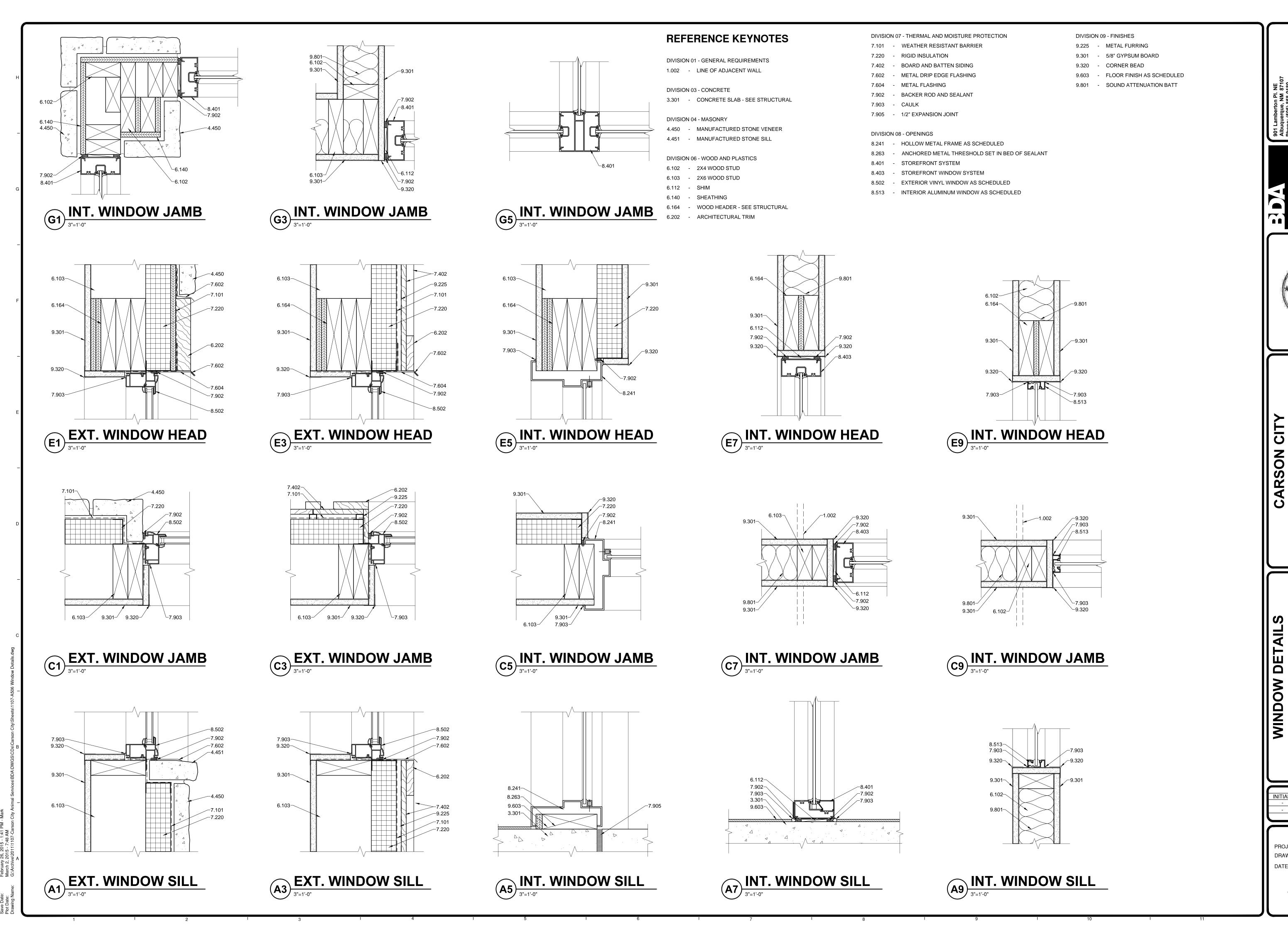
BDA DSGN. REV. BDA TECH REV.



BDA DSGN. REV. BDA TECH REV.

PROJECT NO.: 1107

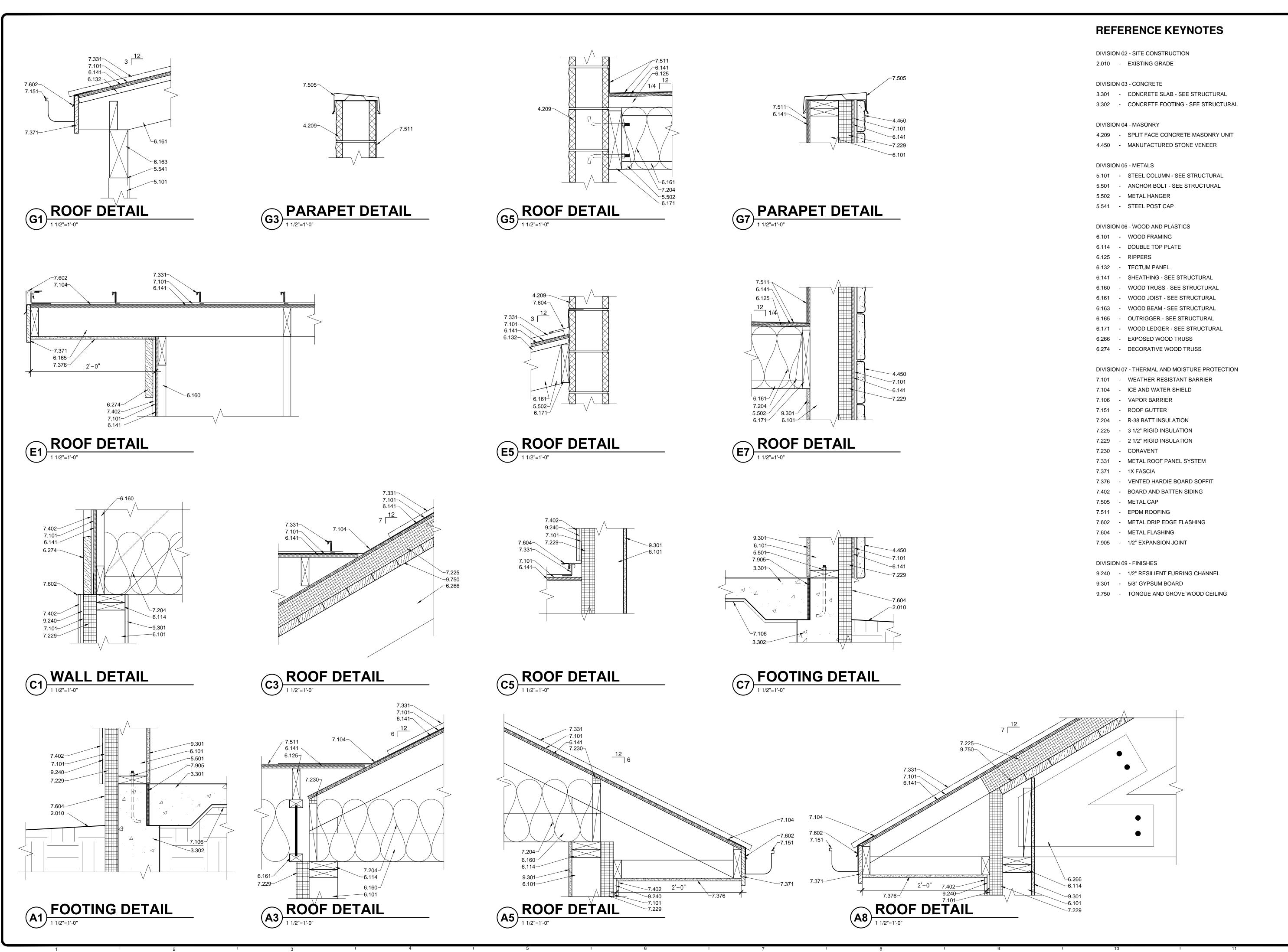
2/27/2015 **A505**



Date: February 26, 2015 - 1:41 PM - Mark ate: March 2, 2015 - 7:48 AM

A506

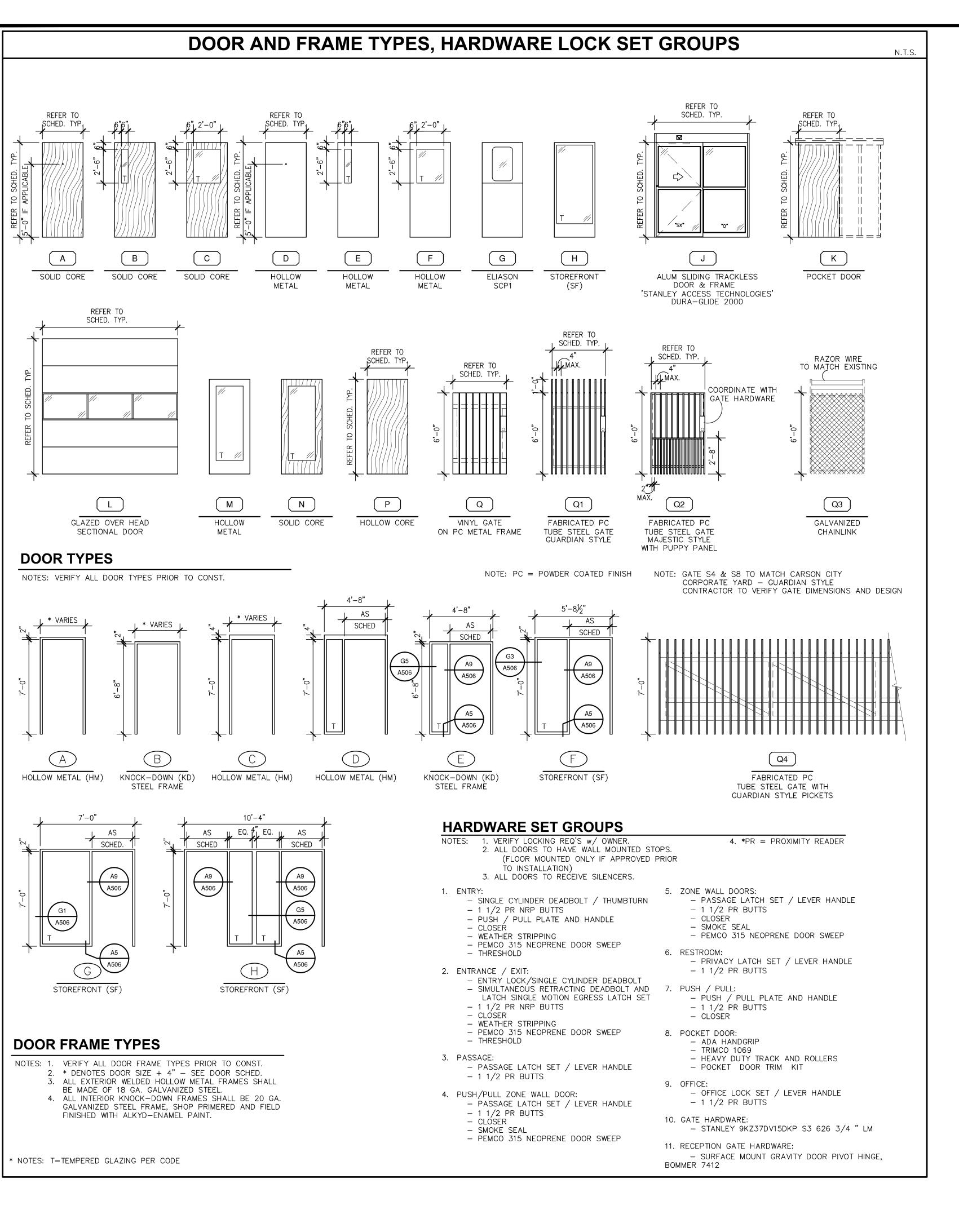
BDA DSGN. REV. BDA TECH REV.



DETAIL

BDA DSGN. REV. BDA TECH REV.

PROJECT NO.: 1107 2/27/2015



		DOOR	SC	H	ED	UL	_E						
	DOC		I = /s	.	Logu	T) (D	l		RAME	l p.=.	LBL	HW	REMARKS
NO.	NAME	DOOR DIMENSIONS	TYP	 	SGN -	-		HD.DTL	JMB.DTL	SILL.DTL	-	MEG	STANIEV DUDA CUDE
101A 101B	VESTIBULE VESTIBULE	6'-0" x 7'-0" x 1-3/4" 6'-0" x 7'-0" x 1-3/4"	J .I	AL AL	-	MFG MFG	AL AL	E1/A504 E3/A504	C1/A504 C3/A504	A1/A504 A3/A504	-	<u> </u>	STANLEY DURA-GLIDE STANLEY DURA-GLIDE
103	JAN	3'-0" x 6'-8" x 1-3/4"	A	SC	-	В	KD	E5/A504	C5/A504	A5/A504 A5/A505	-	3	*PR
104	FELINE ADOPTION	3'-0" x 6'-8" x 1-3/4"	N	SC	-	В	KD	E5/A505	C5/A505	/	-	4	
105A	FELINE F.P./LAUNDRY	3'-6" x 6'-8" x 1-3/4"	В	SC	-	В	KD	E5/A505	C5/A505	/	-	5	*PR
105B	FELINE F.P./LAUNDRY	3'-6" x 6'-8" x 1-3/4"	Α	sc	<u> </u>	В	KD	E5/A505	C5/A505	/	-	3	
106	FELINE ADOPTION	3'-0" x 6'-8" x 1-3/4"	N	sc	-	В	KD	E5/A505	C5/A505	/	-	4	
107A	ADOPTION LOBBY	3'-0" x 6'-8" x 1-3/4"	М	НМ	-	Е	KD	E5/A504	C5/A504	A5/A504	-	2	WOOD LINTEL - LOBBY SID
107B	ADOPTION LOBBY	3'-0" x 7'-0" x 1-3/4"	Н	AL	-	F	AL	E7/A505	A10/A505	A7/A505	-	4	WOOD LINTEL - LOBBY SID
109	FELINE PLAY-1	3'-0" x 7'-0" x 1-3/4"	Н	AL	-	Н	AL	E7/A505	C7/A505	A7/A505		4	WOOD LINTEL - LOBBY SID
110	FELINE PLAY-2	3'-0" x 7'-0" x 1-3/4"	Н	AL	-	Н	AL	E7/A505	C7/A505	A7/A505	-	4	WOOD LINTEL - LOBBY SID
112	CANINE F.P./LAUNDRY	3'-0" x 6'-8" x 1-3/4"	В	SC	-	В	KD	E5/A505	C5/A505	/	-	5	*PR
113A	SMALL DOG ADOPTION	3'-0" x 6'-8" x 1-3/4"	В	SC	<u> </u>	В	KD	E5/A505	C5/A505	/	-	4	ADD DEADBOLT
113B	SMALL DOG ADOPTION	3'-0" x 6'-8" x 1-3/4"	С	SC	<u> </u>	В	KD	E5/A505	C5/A505	/	-	4	
114	JAN	3'-0" x 6'-8" x 1-3/4"	A	SC	 -	В	KD	E5/A505	C5/A505	A5/A505	_	3	*PR
116	PUPPY ADOPTION	3'-0" x 6'-8" x 1-3/4"	C	SC	<u> </u>	В	KD	E5/A505	C5/A505	/		4	
117A	CANINE BUN #1	3'-0" x 6'-8" x 1-3/4"	N	HM	 _	E	KD	E7/A504	C7/A504	A7/A504	_	4	
117B	CANINE ACCUAINT	3'-0" x 7'-0" x 1-3/4"	M	HM	 	D	HM	E1/A505	C1/A505	A1/A505		2	
118	CANINE ACQUAINT	3'-0" x 7'-0" x 1-3/4"	Н	AL	+	G F	AL	E7/A505	A9/A504 A10/A505	A7/A505 A7/A505	-	4	
119A 119B	PUBLIC HALL PUBLIC HALL	3'-0" x 7'-0" x 1-3/4" 3'-0" x 6'-8" x 1-3/4"	H M	AL HM	+ -	E	AL KD	E7/A505 E5/A504	C5/A504	A5/A505	-	2	WOOD LINTEL - LOBBY SID
	CANINE RUNS #2	3'-0" x 6'-8" x 1-3/4"	N	НМ	-	E	KD	E7/A504	C5/A504	A3/A504 A7/A504	-	 	WOOD LINTEL - LOBBY SIL
120A 120B	CANINE RUNS #2	3'-0" x 7'-0" x 1-3/4"	M	HM	-	D	HM	E1/A504	C1/A504	A1/A504 A1/A505	-	2	
121	CANINE ACQUAINT	3'-0" x 7'-0" x 1-3/4"	H	AL	-	G	AL	E7/A505	A9/A504	A7/A505	-	4	
122	MENS R.R.	3'-0" x 6'-8" x 1-3/4"	A	SC	-	В	KD	E5/A505	C5/A505	A5/A505	-	6	
123	WOMENS R.R.	3'-0" x 6'-8" x 1-3/4"	A	SC	-	В	KD	E5/A505	C5/A505	A5/A505	-	6	
124	STAFF HALL	3'-0" x 6'-8" x 1-3/4"	В	SC	-	В	KD	E5/A505	C5/A505	/	-	3	*PR, WOOD LINTEL - LOBBY :
125A	STAFF OFFICE	3'-0" x 6'-8" x 1-3/4"	В	SC	-	В	KD	E5/A505	C5/A505	/	-	3	*PR
125B	STAFF OFFICE	3'-0" x 6'-8" x 1-3/4"	K	SC	-	В	KD	G7/A505	/	G5/A505	-	8	POCKET DOOR
126	RECLAIM/SURRENDER & LICENSE RECEPTION		В	SC	-	В	KD	E5/A505	C5/A505	/	-	5	WOOD LINTEL - LOBBY SIE
127	SERVER PHONE	4@2'-0" x 6'-8" x 1-3/4"	Р	НС	-	В	KD	E5/A505	C5/A505	/	-	3	BI-FOLD DOORS
128	RECLAIM/SURRENDER & LICENSE LOBBY	3'-0" x 6'-8" x 1-3/4"	С	sc	-	В	KD	E5/A505	C5/A505	/	-	3	ADD DEADBOLT
129	UNISEX R.R.	3'-0" x 6'-8" x 1-3/4"	Α	sc	-	В	KD	E5/A505	C5/A505	A5/A505	-	6	
30A	HALL	3'-0" x 7'-0" x 1-3/4"	М	НМ	-	Α	НМ	E5/A504	C5/A504	A5/A504	-	1	
30B	HALL	3'-0" x 6'-8" x 1-3/4"	В	sc	-	В	KD	E5/A505	C5/A505	/	-	5	*PR
31A	SURRENDER EXAM	3'-0" x 6'-8" x 1-3/4"	В	SC	-	В	KD	E5/A505	C5/A505	/	-	5	*PR
31B	SURRENDER EXAM	3'-0" x 6'-8" x 1-3/4"	В	SC	-	В	KD	E5/A505	C5/A505	/	-	3	
132	CONFERENCE / TRAINING	3'-0" x 6'-8" x 1-3/4"	С	sc	-	В	KD	E5/A505	C5/A505	/	-	9	
133	DVM CONSULT	3'-0" x 6'-8" x 1-3/4"	С	sc	-	В	KD	E5/A505	C5/A505	/	-	9	
134	HALL	3'-0" x 6'-8" x 1-3/4"	В	sc	-	В	KD	E5/A505	C5/A505	/	-	3	*PR
135	JAN	3'-0" x 6'-8" x 1-3/4"	Α	SC	-	В	KD	E5/A505	C5/A505	A5/A505	-	3	*PR
136A	BEHAVIORAL EXAM	3'-0" x 6'-8" x 1-3/4"	С	SC	<u> </u>	В	KD	E5/A505	C5/A505	/	-	4	
136B	BEHAVIORAL EXAM	3'-0" x 6'-8" x 1-3/4"	В	SC	ļ <u>-</u>	В	KD	E5/A505	C5/A505	/	_	5	*PR
137	EXOTIC HOLD	3'-0" x 6'-8" x 1-3/4"	С	SC	ļ <u>-</u>	В	KD	E5/A505	C5/A505	/	_	4	
138A	CANINE RUNS #3	3'-0" x 6'-8" x 1-3/4"	N	HM	\perp	В	KD	E7/A504	C7/A504	A7/A504	_	4	
138B	CANINE RUNS #3	3'-0" x 7'-0" x 1-3/4"	M	HM	<u> </u>	С	HM	E1/A505	C1/A505	A1/A505		2	
139	FERAL CAT	3'-0" x 6'-8" x 1-3/4"	C	SC	 	В	KD	E5/A505	C5/A505	/		4	DOOL/ET DOOD
140	EXAM / TREATMENT	3'-0" x 6'-8" x 1-3/4"	K	SC	 _	В	KD	G7/A505	/	G5/A505	_	8 MEC	POCKET DOOR
141	SURGERY	3'-0" x 6'-8" x 1-3/4"	G	AL	 	В	KD	G3/A505	G1/A505	/	_	MFG 9	ELIASON DOOR
142	ACO'S OFFICE	3'-0" x 6'-8" x 1-3/4"	С	SC	 _	В	KD KD	E5/A505	C5/A505	/		<u> </u>	ELIASON DOOR
143 144	WOMENS SHOWER	3'-0" x 6'-8" x 1-3/4" 3'-0" x 6'-8" x 1-3/4"	G A	AL SC	+	ВВ	KD	G3/A505 E5/A505	G1/A505 C5/A505	A5/A505	-	6 6	ELIASON DOOR
145	MANAGERS OFFICE	3'-0" x 6'-8" x 1-3/4"	В	SC	-	В	KD	E5/A505	C5/A505	/	-	9	
147	STAFF LOUNGE	3'-0" x 6'-8" x 1-3/4"	С	sc	+ -	В	KD	E5/A505	C5/A505	/	-	7	
148	FELINE HOLD/QUARANTINE	3'-0" x 6'-8" x 1-3/4"	С	SC	-	В	KD	E5/A505	C5/A505	/	-	4	
149	MENS SHOWER	3'-0" x 6'-8" x 1-3/4"	A	sc	-	В	KD	E5/A505	C5/A505	A5/A505	-	6	
150	JAN	3'-0" x 6'-8" x 1-3/4"	A	SC	-	В	KD	E5/A505	C5/A505	A5/A505	 	3	
151	CENTRAL STORAGE	3'-0" x 6'-8" x 1-3/4"	C	sc	-	В	KD	E5/A505	C5/A505	/	-	3	
152	INTAKE EXAM	3'-0" x 6'-8" x 1-3/4"	С	SC	-	В	KD	E5/A505	C5/A505	/	-	7	
153	INTAKE FP / LAUNDRY / BATHE	3'-0" x 6'-8" x 1-3/4"	K	SC	-	В	KD	G7/A505	/	G5/A505	-	8	POCKET DOOR
55A	CANINE HOLD / QUARANTINE	3'-0" x 6'-8" x 1-3/4"	F	НМ	-	В	KD	E7/A504	C7/A504	A7/A504	-	4	
55B	CANINE HOLD / QUARANTINE	3'-0" x 7'-0" x 1-3/4"	F	НМ	-	С	НМ	E1/A505	C1/A505	A1/A505	-	2	*PR
56A	CANINE ISOLATION	3'-0" x 6'-8" x 1-3/4"	F	НМ	-	В	KD	E5/A505	C5/A505	/	-	4	<u> </u>
56B	CANINE ISOLATION	3'-0" x 6'-8" x 1-3/4"	F	НМ	-	Α	НМ	E5/A504	C5/A504	A5/A504	-	2	
157	FELINE ISOLATION	3'-0" x 6'-8" x 1-3/4"	F	НМ	-	В	KD	E5/A505	C5/A505	/		4	
58A	INTAKE GARAGE	10'-0" x 10'-0" x 1-3/4"	L	НМ	<u> </u>	MFG	MFG	/	G7/A504	G5/A504	L	MFG	OVERHEAD SECTIONA
58B	INTAKE GARAGE	3'-0" x 7'-0" x 1-3/4"	D	НМ	-	С	НМ	E1/A505	C1/A505	A1/A505		2	*PR
58C	INTAKE GARAGE	3'-0" x 7'-0" x 1-3/4"	F	НМ	-	С	НМ	E1/A505 SIM	C1/A505 SIM	/		4	*PR
58D	INTAKE GARAGE	10'-0" x 10'-0" x 1-3/4"	L	НМ	-	MFG	MFG	/	G7/A504	G5/A504	<u> </u>	MFG	OVERHEAD SECTIONA
	INTAKE GARAGE	3'-0" x 7'-0" x 1-3/4"	F	НМ	-	С	НМ	E1/A505	C1/A505	A1/A505	<u> </u>	2	*PR
58E	EUTHANASIA	3'-0" x 7'-0" x 1-3/4"	Е	НМ	-	С			C1/A505 SIM		<u> </u>	4	*PR
159		3'-0" x 7'-0" x 1-3/4"	E	НМ		С	НМ	E1/A505 SIM	C1/A505 SIM	/	<u> </u>	4	
159	MECHANICAL STORAGE			VYL	-		-	/	/	/	<u> </u>	10	
159 160 S1		4'-0" x 6'-0"	Q								4 -	1	ľ
159 160 S1	MECHANICAL STORAGE		Q	VYL	-	-	-	/	/	/		10	
159 160 S1 S2	MECHANICAL STORAGE EXTERIOR YARD	4'-0" x 6'-0"	+ -		-	-	-	/	/	/	-	10	
159 160 S1 S2 S3	MECHANICAL STORAGE EXTERIOR YARD EXTERIOR YARD EXTERIOR YARD EXTERIOR YARD	4'-0" x 6'-0" 4'-0" x 6'-0" 4'-0" x 6'-0" 4'-0" x 6'-0"	Q Q Q1	VYL VYL STL	-	-	-	•	<u>'</u>	,	-	 	
159 160 S1 S2 S3 S4 S5	MECHANICAL STORAGE EXTERIOR YARD EXTERIOR YARD EXTERIOR YARD	4'-0" x 6'-0" 4'-0" x 6'-0" 4'-0" x 6'-0"	Q Q Q1 Q2	VYL VYL STL STL		-	-	/	/	/	-	10	GATE LATCH WITH PAD LO
159 160 S1 S2 S3 S4 S5	MECHANICAL STORAGE EXTERIOR YARD	4'-0" x 6'-0" 4'-0" x 6'-0" 4'-0" x 6'-0" 4'-0" x 6'-0" 4'-0" x 6'-0" PR 3'-0" x 6'-0"	Q Q Q1 Q2 Q2	VYL VYL STL STL	-	- - -	-	/	/	/	- - -	10	
S1 S2 S3 S4 S5 S6 S7	MECHANICAL STORAGE EXTERIOR YARD	4'-0" x 6'-0" 4'-0" x 6'-0" 4'-0" x 6'-0" 4'-0" x 6'-0" 4'-0" x 6'-0"	Q Q Q1 Q2 Q2 Q3	VYL VYL STL STL STL GLV		- - - -	-	/ /	/	/	-	10	GATE LATCH WITH PAD LO
159 160 S1 S2 S3 S4 S5 S6 S7	MECHANICAL STORAGE EXTERIOR YARD EXTERIOR YARD	4'-0" x 6'-0" 4'-0" x 6'-0" 4'-0" x 6'-0" 4'-0" x 6'-0" PR 3'-0" x 6'-0" 4'-0" x 6'-0" 26'-0" x 7'-0"	Q Q Q1 Q1 Q2 Q2 Q3 Q4	VYL VYL STL STL GLV STL	-	-	-	////	/ /	///	-	10	GATE LATCH WITH PAD LO
159 160 S1 S2 S3 S4 S5 S6 S7	MECHANICAL STORAGE EXTERIOR YARD	4'-0" x 6'-0" 4'-0" x 6'-0" 4'-0" x 6'-0" 4'-0" x 6'-0" 4'-0" x 6'-0" PR 3'-0" x 6'-0" 4'-0" x 6'-0"	Q Q Q1 Q1 Q2 Q2 Q3 Q4	VYL VYL STL STL STL GLV	-	-	-	/ / /	/ / /	/ / /	-	10	GATE LATCH WITH PAD LO

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REV.# DATE COMMENTS
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REVIEWS

ALS

BDA DSGN. REV.

BDA TECH REV.

CCAS
PROJECT NO.: 1107
DRAWN: LLK

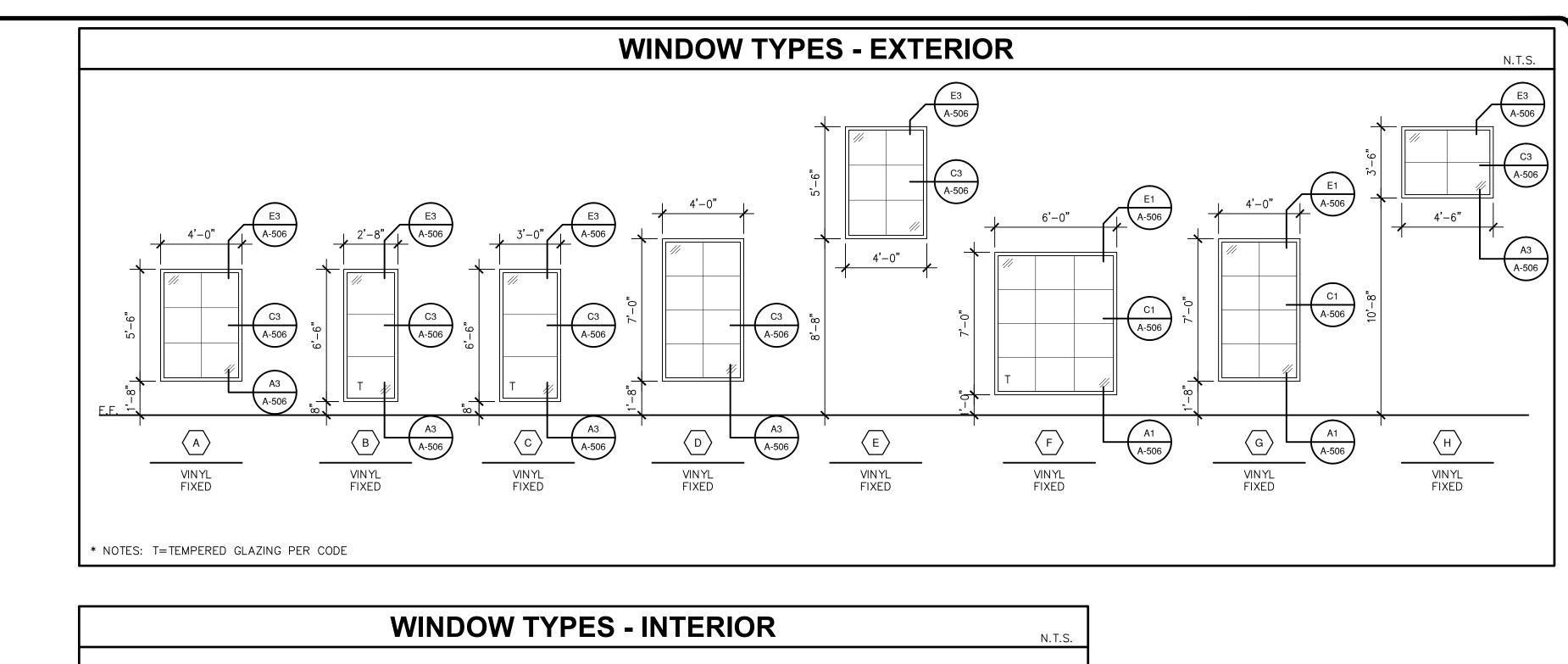
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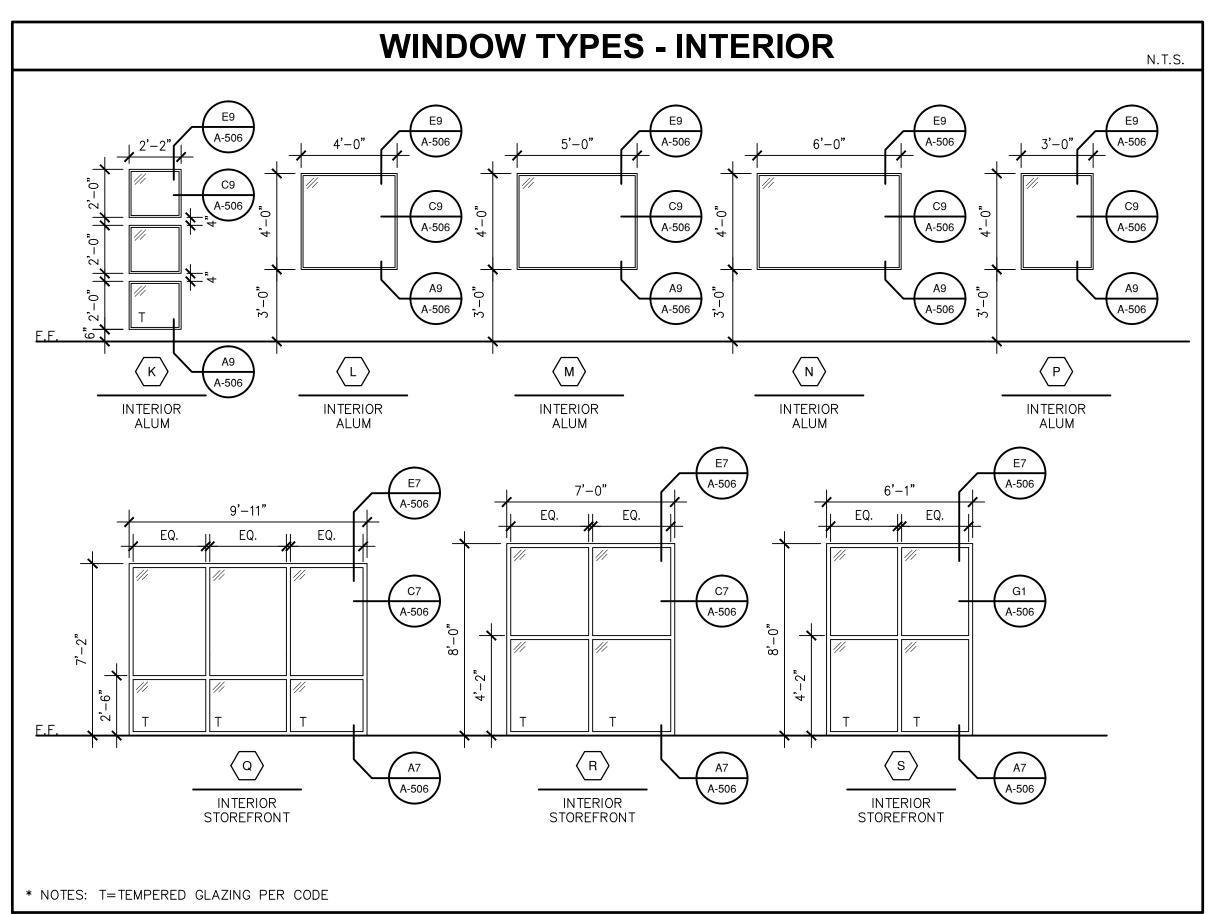
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2/27/2015





WINDOW

BDA DSGN. REV. BDA TECH REV.

PROJECT NO.: 1107

	ROOM FINISH	SCHED	DULE		ROOM FINISH SCHEDULE													
ROOM	FLOOR WALL	COVER	CASEWORK FABRIC			ROOM		FLOOR		WAI	LL COVE	.R		CASEWO	JRK	F	ABRIC	
. NAME	MAT'L GROUT BASE GROUT MAT'L C	GROUT HT. E	BASE WALL CNTR SHELVES SEAT BACK	REMARKS	NO.	. NAME	MAT'L G	ROUT BAS	SE GROUT	MAT'L	GROUT	HT.	BASE	WALL CI	NTR SHELV	/ES SE/	AT BACK	K REMARKS
VECTIBLILE	PT GRT PT GRT -		<u> </u>		156	CANINE ICOLATION	EPF	- FC	,	CT*	GRT	FULL		-				*SEE INT. ELEVATION C6/ I-105
VESTIBULE -	F1 GR1 B1 GR2 -				100 ,	CANINE ISOLATION	F2	- F2	2 -	W6,W4	GR2	'						Ţ
BURLIO HALL	PT GRT PT GRT -				457	FELINE IOOLATION	EPF	- FC	,	CT*	GRT	FULL	-	-			<u> </u>	*SEE INT. ELEVATION C6/ I-105
PUBLIC HALL	F1 GR1 B1 GR2 -				157	FELINE ISOLATION	F2	- F2	2 -	W6,W4	GR2	-	-					Ţ
WITTER OF COST	PT GRT PT - CT*	GRT +54"	PL** *C·	CT AT NORTH, SOUTH, WEST WALLS. SEE PAINT REFERNCE PLAN I-102	450	THE CARACE	EPF		-	EP*	-	+72"	-	-		<u> </u>	-	*EP AT RUNS. SEE PAINT REFERENCE PLAN I-102
3 JANITOR CLOSET	F1 GR1 B1 - W6	GR2 -	PL3 **r	*PL AT WALL MOUNTED CABINET	158 '	B INTAKE GARAGE	F2		-	P7	-	- '	-	-		-		T
FELINE ADOPTION	PT GRT PT GRT -				150		EPF	- FC	, -	EP	-		-				<u> </u>	*SEE INT. ELEVATION A8/ I-105
FELINE ADOPTION	F1 GR1 B1 GR2 -				159) EUTHANASIA	F2	- F2	<u> - </u>	P7	-	-	-	-				Ţ
ESTAIS S.D. / LAUNDDA/	PT GRT PT* GRT -		PL** - PL** PL** *B4	B4 AT CASEWORK TOEKICK. SEE INT. ELEVATION C7/ I-105	166	NECHANICAL CTORACE	SC	- RB	- د	CT*	GRT	+48"	-				<u> </u>	*CT AT MOP SINK
FELINE F.P. / LAUNDRY	F1 GR1 B1,B4 GR2 -		PL2 - PL4 PL3 **f	*SEE INT. ELEVATION A1/ I-104	160 ,	MECHANICAL STORAGE	-	- B2	2 -	W6	GR2	-	-	-			-	Ţ
ESTINE ADOPTION	PT GRT PT GRT -																	
FELINE ADOPTION	F1 GR1 B1 GR2 -						ROOM	/I HIP	11SH	LEG	END	/						
A DODTION LODDY	PT GRT PT* GRT MS,CT**	GRT -	PL** - PL,SS** *B4	B4 AT RECEPTION DESK TOEKICK. SEE INT. ELEVATION C7/ I-105														
ADOPTION LOBBY	F1 GR1 B1,B4 GR2 W1,W2,W3	GR2 -	PL2 - PL1.G1 **C	*CT AT FACE OF RECEPTION DESK, SEE INT. ELEV. A1, A5, E1, E5/ I-103	F	PT - PORCELAIN TILE			FC	- FLASH	HCOVE							

*B4 AT RECEPTION DESK TOEKICK. SEE INT. ELEVATION C7/ I-105

*B4 AT CASEWORK TOEKICK. SEE INT. ELEVATION C7/I-103

*CT AT NORTH, SOUTH, WEST WALLS. SEE PAINT REFERNCE PLAN I-102

*CT AT SOUTH WALL, SEE TILE PATTERN C3/ I-105 & PAINT REFERENCE PLAN I-102

*CT AT SOUTH WALL, SEE TILE PATTERN C3/ I-105 & PAINT REFERENCE PLAN I-102

CT WALL TILE AT EAST, SOUTH, WEST WALLS. STOP AT DOOR JAMB. SEE INT. ELEV. C1/ I-105

T BASE AT NORTH WALL AND EAST WALL. STOP AT DOOR JAMB

CT WALL TILE AT EAST AND SOUTH WALLS, SEE INT. ELEVATION C1/ I-105

*B4 AT RECEPTION DESK TOE KICK. SEE INT. ELEVATION C7/I-105

**CT AT FACE OF RECEPTION DESK, SEE INT. ELEVATION A8, C1, C4/ I-104

**SEE INT. ELEVATIONS A3 AND A6/ I-104

*FULL POLYASPARTIC EPF AT OUTDOOR RUNS

*FULL POLYASPARTIC EPF AT OUTDOOR RUNS

*CT BASE AT NORTH AND WEST WALLS

**PL AT WALL MOUNTED CABINET

*CT AT FACE OF RECEPTION DESK, SEE INT. ELEVATIONS E1, E5/ I-103

GRT - GROUT

RB - RUBBER BASE

CT - CERAMIC TILE

WS - WOOD STAIN

EPF - EPOXY FLOORING

			G. 1.1	51,51	OI L	112,110	GITE				1 21,011				
127	SERVER / PHONE	EPF	-	FC	-	-	-	-	-	-	-	-	-	•	-
	SERVERY I FIGHE	F2	-	F2	-	-	-	-	-	-	-	-	-	-	-
		PT	GRT	PT*	GRT	CT**	GRT	-	PL**	-	PL,SS**	-	-	-	
128	RECLAIM/SURRENDER & LICENSE LOBBY	F1	GR1	B1,B4	GR2	W2,W3	GR2	-	PL2	-	PL1,G1	-	-	-	-
		PT	GRT	CT*	GRT	CT**	GRT	FULL	_	_	_	_	_	_	*CT BASE AT NORTH AND EAST WALLS
129	UNISEX RESTROOM	F1	GR1	B3	GR2	W3,W4,W5	GR2				_		_	_	**CT WALL TILE AT SOUTH AND WEST WALLS. SEE INT. ELEVATION C1/ I-105
			_								_	_	_		OF WALE THE AT GOOTT AND WEST WALES, SEE INT. ELEVATION OF PIOS
130	HALL	PT	GRT	PT	GRT	-	-	-	-	-	-	-	-	-	-
		F1	GR1	B1	GR2	-	-	-	-	-	-	-	-	-	•
131	SURRENDER EXAM	EPF	-	FC	-	-	-	-	PL*	-	PL*	PL*	-	-	*SEE INT. ELEVATION C6/ I-104
131	CONNENDER EXAM	F2	-	F2	-	-	-	-	PL5	-	PL4	PL3	-	-	-
		PT	GRT	PT	GRT	-	-	-	PL*	-	PL*	PL*	-	-	*SEE INT. ELEVATION C8/ I-103
132	CONFERENCE / TRAINING	F1	GR1	B1	GR2	-	-	-	PL2	_	PL4	PL3	-	-	-
		EPF		FC	_	-		_	_	_	_	_	_	-	
133	DVM CONSULT				_	-					_				
		F2	-	F2	-	-			-	_	-	-	_	-	-
134	HALL	EPF	-	FC	-	-	-	-	-	-	-	-	-	-	-
		F2	-	F2	-	-	-	-	-	-	-	-	-	-	-
405	IANUTOR OLOGET	EPF	-	FC	-	CT*	GRT	+54"	-	-	-	PL**	-	-	*CT AT EAST, SOUTH, WEST WALLS. SEE PAINT REFERNCE PLAN I-102
135	JANITOR CLOSET	F2	-	F2	-	W6	GR2	-	-	-	-	PL3	-	-	**PL AT WALL MOUNTED CABINET
		EPF	-	FC	-	-	_	-	_	_	_	_	-	-	
136	BEHAVIORAL EXAM	F2		F2	_	_		_	_		_	_		-	
			-		-	-			-	-		-	-	-	
137	EXOTIC HOLD	EPF	-	FC	-	-	-	-	-	-	PL*	-	-	-	*PL AT ALL COUNTERS, SEE INT. ELEVATIONS C10, E1/ I-104
.0.	27.6 1.6 1.6 2.5	F2	-	F2	-	-	-	-	-	-	PL4	-	-	-	-
400	OANUNE DUNIO "O	EPF*	-	-	-	EP,CT**	GRT	+78"	-	-	-	-	-	-	*FULL POLYASPARTIC EPF AT OUTDOOR RUNS
138	CANINE RUNS #3	F2	-	-	-	P5,P7,P8,W6,W4	GR2	-	-	-	-	-	-	-	**CT AT SOUTH WALL, SEE TILE PATTERN C3/ I-105 & PAINT REFERENCE PLAN I-102
		EPF	-	FC	_	_	_	_	_	_	_	_	_	_	
139	FERAL CAT	F2		F2	-	_		_	_	_	_	_	_	-	
			<u> </u>		_	-								-	
140	EXAM / TREATMENT	EPF	-	FC	-	-	-	-	PL*	-	PL*	PL*	-	-	*SEE INT. ELEVATIONS E3, E5, E7, G1/ I-104
	·	F2	-	F2	-	-	-	-	PL5	-	PL4	PL3	-	-	**SEE A9/ I-105 FOR PASS THRU SECTION
444	CLIDOEDV	EPF	-	FC	-	-	-	-	PL*	-	PL*	PL*	-	-	*SEE INT. ELEVATION G4/ I-104
141	SURGERY	F2	-	F2	-	-	-	-	PL5	-	PL4	PL3	-	-	**SEE A9/ I-105 FOR PASS THRU SECTION
		EPF	-	FC	-	-	-	_	_	-	_	-	_	-	
142	ACO'S OFFICE	F2	<u> </u>	F2	_	_		_	_	_	_	_	-	_	
143	ICU	EPF	-	FC	-	-		-	-	-	-	-	-	-	•
		F2	-	F2	-	-	-	-	-	-	-	-	-	-	•
144	WOMENS SHOWER	EPF	-	-	-	CT*	GRT	+54"	-	-	-	-	-	-	*SEE INT. ELEVATION C2/ I-105
144	WOWLING SHOWLIN	F2	-	-	-	W6,W4	GR2	-	-	-	-	-	-	-	-
		EPF	-	FC	-	-	-	-	-	-	-	-	-	-	
145	MANAGER'S OFFICE	F2	-	F2	-	-	-	-	-	-	-	-	-	-	-
		EPF	 	FC	_	-		_	_	_	_	_	_	-	
146	HALL				_	-					_				
		F2	-	F2	-	-	-	-	-	-	-	-	-	-	-
147	STAFF LOUNGE	EPF	-	FC	-	-	-	-	PL*	-	PL*	PL*	-	-	*SEE INT. ELEVATION G7/ I-104
	0.7.11. 200.102	F2	-	F2	-	-	-	-	PL5	-	PL4	PL3	-	-	-
440	FELINE LIQUE / QUADANTINE	EPF	-	FC	-	-	-	-	-	-	-	-	-	-	-
148	FELINE HOLD / QUARANTINE	F2	-	F2	-	-	-	-	-	-	-	-	-	-	-
		EPF	-	-	-	CT*	GRT	+54"	-	_	_	_	_	-	*SEE INT. ELEVATION C2/ I-105
149	MENS SHOWER	F2	<u> </u>	_	_	W6,W4	GR2		_		_	_	_	_	
				F0			GRT								TOTAT COUTE WEST MODELLWALLS OFF DAINT REFERNOE BLANLAGS
150	JANITOR CLOSET	EPF	-	FC	-	CT*		+54"	-	-	-	-	-	-	*CT AT SOUTH, WEST, NORTH WALLS. SEE PAINT REFERNCE PLAN I-102
		F2	-	F2	-	W6	GR2	-	-	-	-	-	-	-	-
151	CENTRAL STORAGE	EPF	-	FC	-	-	-	-	-	-	-	-	-	ı	-
151	CENTRAL STORAGE	F2	-	F2	-	-	-	-	-	-	-	-	-	-	
		EPF	-	FC	-	EP	-	-	PL*	-	PL*	PL*	-	-	*SEE INT. ELEVATION A1/ I-105
152	INTAKE EXAM	F2	-	F2	_	P6	-	_	PL5	_	PL4	PL3	-	-	
		EPF	_	FC					PL*	_	PL*	PL*		-	*SEE INT. ELEVATION A3, A5/ I-105
153	INTAKE F.P. / LAUNDRY / BATHE		-		-	-		-		-			-		OLL INT. ELEVATION AS, AS/ 1-103
		F2	<u> </u>	F2	-	-	-	-	PL5	-	PL4	PL3	-	-	•
154	HALL	EPF	<u> </u>	FC	-	-	-	-	-	-	-	-	-	-	-
.∪-т	· · · ·—	F2	-	F2	-	-	-	-	-	-	-	-	-	-	·
455	CANINE LIGID / OLIABANTINE	EPF*	-	-	-	EP,CT**	GRT	+78"	-	-	-	-	-	-	*FULL POLYASPARTIC EPF AT OUTDOOR RUNS
155	CANINE HOLD / QUARANTINE	F2	_	-	-	P5,P7,P8,W6,W4	GR2	-	-	-	-	-	-	-	**CT AT SOUTH WALL, SEE TILE PATTERN C3/ I-105 & PAINT REFERENCE PLAN I-102
ı		. –													

MS,CT**

W1,W2,W3

W6

- EP,CT** GRT +78"

P5,P7,P8,W6,W4 GR2

CT**

CT**

W2,W3

W3,W4,W5

EP,CT** GRT +78"

GRT FULL

GRT FULL

GR2

GRT

GR2

P5,P7,P8,W6,W4 GR2

B1,B4 GR2

PT GRT

B1 GR2 RB

B2

PT

B1 -

PT GRT B1 GR2

B1 GR2

PT GRT B1 GR2

PT GRT

B1 GR2

CT* GRT B3 GR2

FC

F2 FC

F2

 PT
 GRT
 PT*
 GRT

 F1
 GR1
 B1,B4
 GR2

 PT
 GRT
 CT*
 GRT
 CT**
 GRT

 F1
 GR1
 B3
 GR2
 W3,W4,W5
 GR2

 PT
 GRT
 PT
 GRT

 F1
 GR1
 B1
 GR2

PT GRT PT* GRT

 F1
 GR1
 B1,B4
 GR2

 PT
 GRT
 PT
 GRT

 F1
 GR1
 B1
 GR2

PT GRT PT GRT

 PT
 GRT
 PT
 GRT

 F1
 GR1
 B1
 GR2

PT GRT F1 GR1

PT GRT

F1 GR1

F1 GR1

PT GRT F1 GR1

PT GRT F1 GR1

PT GRT F1 GR1

EPF F2

108 ADOPTION RECEPTION

112 CANINE F.P. / LAUNDRY

113 SMALL DOG ADOPTION

114 JANITOR CLOSET

116 PUPPY ADOPTION

117 CANINE RUN #1

119 PUBLIC HALL

120 CANINE RUNS #2

121 CANINE ACQUAINT

122 MENS RESTROOM

124 STAFF HALL

125 STAFF OFFICE

123 WOMENS RESTROOM

126 RECLAIM/SURRENDER & LICENSE RECEPTION

118 CANINE ACQUAINT

115 HALL

109 FELINE PLAY - 1

110 FELINE PLAY - 2

111 MECHANICAL

PL** - PL,SS**

PL1,G1

PL** PL**

PL4 PL3

- PL3

PL2

PL** -

PL2 -

PL** -

PL2 - PL1,G1

PL,SS**

GRT +54"

GR2

NC	D. MANUFACTURER	MATERIAL	SERIES	COLOR	REMARKS	CONTACT
F1	CACTUS TILE	PORCELAIN TILE	DOCK	SANTA MONICA	6"X36"	1(800) 528-9445
F2	SIKAFLOOR	EOPXY FLOORING	DECOFLAKE SYSTEM	SAHARA	HYBRID SMALL AND LARGE FLAKES WITH SIKA 315 COATING	CHRIS MORALES (925) 437-1727
D1	DALTILE	GLAZED PORCELAIN	CONCRETE CONNECTION	BOULEVARD BEIGE	3"X13" BULLNOSE	DALTH F OAL FO ONTD/775) 050 070
B1 B2		RUBBER BASE	700 SERIES	114 LUNAR DUST	3 X 13 BULLNUSE	DALTILE SALES CNTR(775) 358-872
_		CERAMIC BASE	UNGLAZED CERAMIC MOSAICS	BUILT UP BASE MT6A, A24 ALMOND	-	PACIFIC FLOORING (775) 359-3784
B3				· · · · · · · · · · · · · · · · · · ·		JS FLOORING (775) 267-4123
B4	DALTILE	GLAZED PORCELAIN	CONCRETE CONNECTION	BOULEVARD BEIGE	20"X20" TILE CUT DOWN TO 4"X20"	DALTILE SALES CNTR(775) 358-872
GF	R1 MAPEI	GROUT	FLEXCOLOR CQ	11 SAHARA BEIGE	-	WWW.MAPEI.COM
GF	R2 MAPEI	GROUT	FLEXCOLOR CQ	49 LIGHT ALMOND	-	WWW.MAPEI.COM
P1	SHERWIN WILLIAMS	PAINT	DURATION SATIN	SW6091 RELIABLE WHITE		CLIEDWIN WILLIAMS (775) 000 0014
P2		PAINT	DURATION SATIN	SW2804 RENWICK ROSE BEIGE	 	SHERWIN WILLIAMS (775) 883-0911
P3		PAINT	DURATION SATIN	SW6106 KILIM BIEGE	<u> </u>	SHERWIN WILLIAMS (775) 883-0911
P3 P4		PAINT			<u> </u>	SHERWIN WILLIAMS (775) 883-0911
			DURATION SATIN	SW6004 BELIARIE WULTE		SHERWIN WILLIAMS (775) 883-0911
P5		EPOXY PAINT	ļ-	SW6091 RELIABLE WHITE		SHERWIN WILLIAMS (775) 883-0911
P6		EPOXY PAINT	 -	SW2804 RENWICK ROSE BEIGE		SHERWIN WILLIAMS (775) 883-0911
P7		EPOXY PAINT	<u> </u> -	SW6106 KILIM BIEGE	-	SHERWIN WILLIAMS (775) 883-0911
P8	SHERWIN WILLIAMS	EPOXY PAINT	-	SW6004 MINK		SHERWIN WILLIAMS (775) 883-0911
W	1 CORONADO STONE	MANUFACTURED STONE VENEER	EASTERN MOUNTAIN LEDGE	MADISON COUNTY	-	CARSON MASONRY (775) 882-3832
W	2 DALTILE	GLAZED PORCELAIN	CONCRETE CONNECTION	BOULEVARD BEIGE	20"X20"	DALTILE SALES CNTR(775) 358-872
W:	3 AMERICAN OLEAN	CERAMIC TILE	UNGLAZED CERAMIC MOSAICS	ALMOND A24	2"X2"	JS FLOORING (775) 267-4123
W	4 AMERICAN OLEAN	CEREAMIC TILE	-	MATTE SMOKEY QUARTZ 0028	2"X4"	JS FLOORING (775) 267-4123
W	5 DALTILE	GLASS TILE	GLASS MOSAICS	IS16 BRICK DUST	1"X1"	DALTILE SALES CNTR(775) 358-872
W	6 DALTILE	CERAMIC TILE	-	MATTE ALMOND X735	6"X6"	DALTILE SALES CNTR(775) 358-872
W	7 DALTILE	CERAMIC TILE	-	MATTE ALMOND X735	6"X6" BULLNOSE	DALTILE SALES CNTR(775) 358-872
PL		PLASTIC LAMINATE	-	FLAME SOAPSTONE 4884-38	-	CABINET & LIGHTING (775) 851-400
	2 FORMICA	PLASTIC LAMINATE	-	FLAX GAUZE 7708-58	-	DAVE GOLLINGS (916) 426-3292
	3 FORMICA	PLASTIC LAMINATE	-	SAIL WHITE 463-58	-	DAVE GOLLINGS (916) 426-3292
PL		PLASTIC LAMINATE	-	AUTUMN INDIAN SLATE 3687-77	-	DAVE GOLLINGS (916) 426-3292
PL	5 FORMICA	PLASTIC LAMINATE	-	COGNAC MAPLE 7738-58		DAVE GOLLINGS (916) 426-3292
E.	1 ESI	EDGE BANDING	.018"	FORMICA, SAIL WHITE 463	MATCH WITH ALL PL3 UPPER CASEWORK	EDGEBANDING-SERVICES.COM
Εź	² ESI	EDGE BANDING	змм	FORMICA, MOJAVE 8751	MATCH WITH ALL PL1, PL4, PL5 COUNTERS AND LOWER CASEWORK	EDGEBANDING-SERVICES.COM
E	3 ESI	EDGE BANDING	змм	FORMICA, SAIL WHITE 463	MATCH WITH ALL PL2 LOWER CASEWORK	EDGEBANDING-SERVICES.COM
WS	SHERWIN WILLIAMS	WOOD STAIN	WOOD CLASSICS	SW3109-P BRIGHT CHERRY	-	SHERWIN WILLIAMS (775) 883-0911
G1	SAMSUNG	SOLID SURFACE	STARON TEMPEST	FB154 BRONZESTAR	-	1 (800) 795-7177
G2		METAL ROOFING	STANDARD COLORS	CHESTNUT	-	1 (800) 646-3826
G3		FIBER CEMENT SIDING	HARDIE PANEL VERTICAL SIDING	MONTEREY TAUPE	-	1 (888) 542-7343
G4		SPLIT FACE CMU, SCORE AT 8"	-	MATCH WITH 910, WHITESANDS	WHITESANDS, 910 COLOR BY UTILITY BLOCK COMPANY, INC.	-
G5		SMOOTH FACE CMU BLOCK	+	MATCH WITH 910, WHITESANDS	WHITESANDS, 910 COLOR BY UTILITY BLOCK COMPANY, INC.	+

SC - SEALED CONCRETE

SS - SOLID SURFACE

EP - EPOXY PAINT

PL - PLASTIC LAMINATE

MS - MANUFACTURED STONE

300



INITIALS

BV BDA DSGN. REV.

BDA TECH REV.

PROJECT NO.: 1107

1. ALL WALLS TO BE PGB FINISH UNLESS OTHERWISE NOTED

2. SEE SHEET I101 FOR INTERIOR SPECIFICATIONS SCHEDULE & ROOM FINISH SCHEDULE

3. ALL WALLS TO BE PAINTED P1 (SW 6091 RELIABLE WHITE) UNLESS OTHERWISE NOTED.

4. ALL PGB CEILINGS AND SOFFITS TO BE PAINTED P1 (SW 6091 RELIABLE WHITE) UNLESS OTHERWISE NOTED.

INTERIOR FINISHES \diamondsuit

PAINT: P2 - SW2804 RENWICK ROSE BEIGE

P3 - SW6106 KILIM BEIGE

P4 - SW6004 MINK

P6 - EPOXY, SW2804 RENWICK ROSE BEIGE P7 - EPOXY, SW6106 KILIM BEIGE

WALLCOVERING: W1 - MANUFACTURED STONE, SIERRA SUNRISE MS86
W2 - GLAZED PORCELAIN, BOULEVARD BEIGE, 20"X20"

B - CERAMIC TILE, ALMOND A24, 2"X2"

- CERAMIC TILE, MATTE SMOKEY QUARTZ 0028, 2"X4"

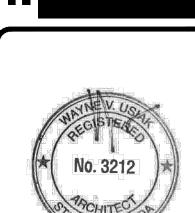
5 - GLASS TILE, IS16 BRICK DUST, 1"X1"

W6 - CERAMIC TILE, MATTE ALMOND X735, 6"X6"
 W7 - CERAMIC TILE, MATTE ALMOND X735, 6"X6" BULLNOSE



PAINT REFERENCE PLAN

1/8"=1'-0"



FR 1076

ANIMAL SHELTER 549 Airport Road

REFERENCE FLOOR PLAN

REV.# DATE COMMENTS
SION: - COMMENTS
SION: - COMMENTS

REVIEWS
ALS

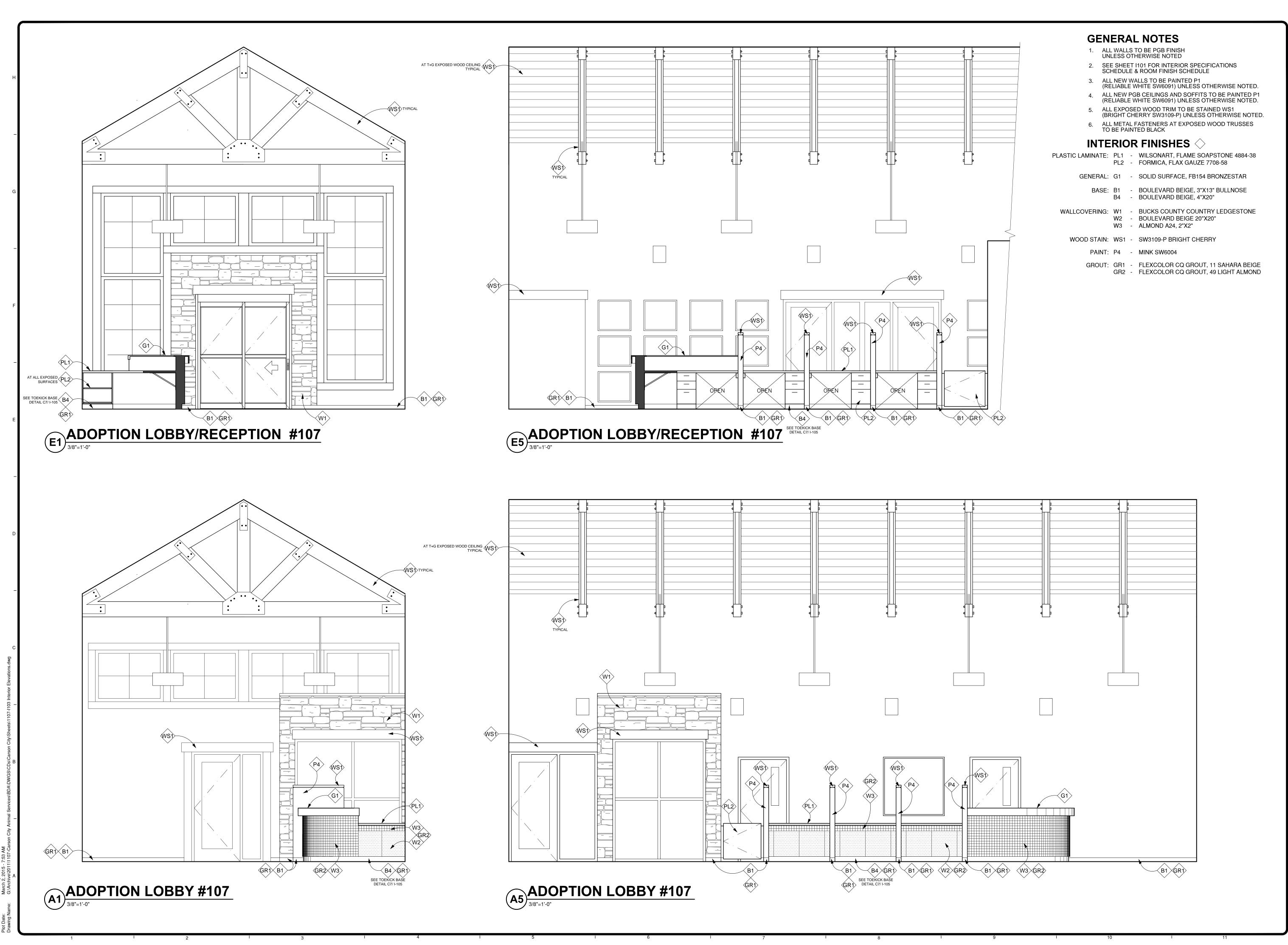
BDA DSGN. REV.

REVIEWS
INITIALS
BV BDA DSGN. REV.
- BDA TECH REV.

CCAS
PROJECT NO.: 1107
DRAWN: NF

I102

2 OF



No. 3212

No. 3212

CARSON CITY
ANIMAL SHELTER
549 Airport Road

549 Airpo Carson City, N City of Car

INTERIOR ELEVATIONS

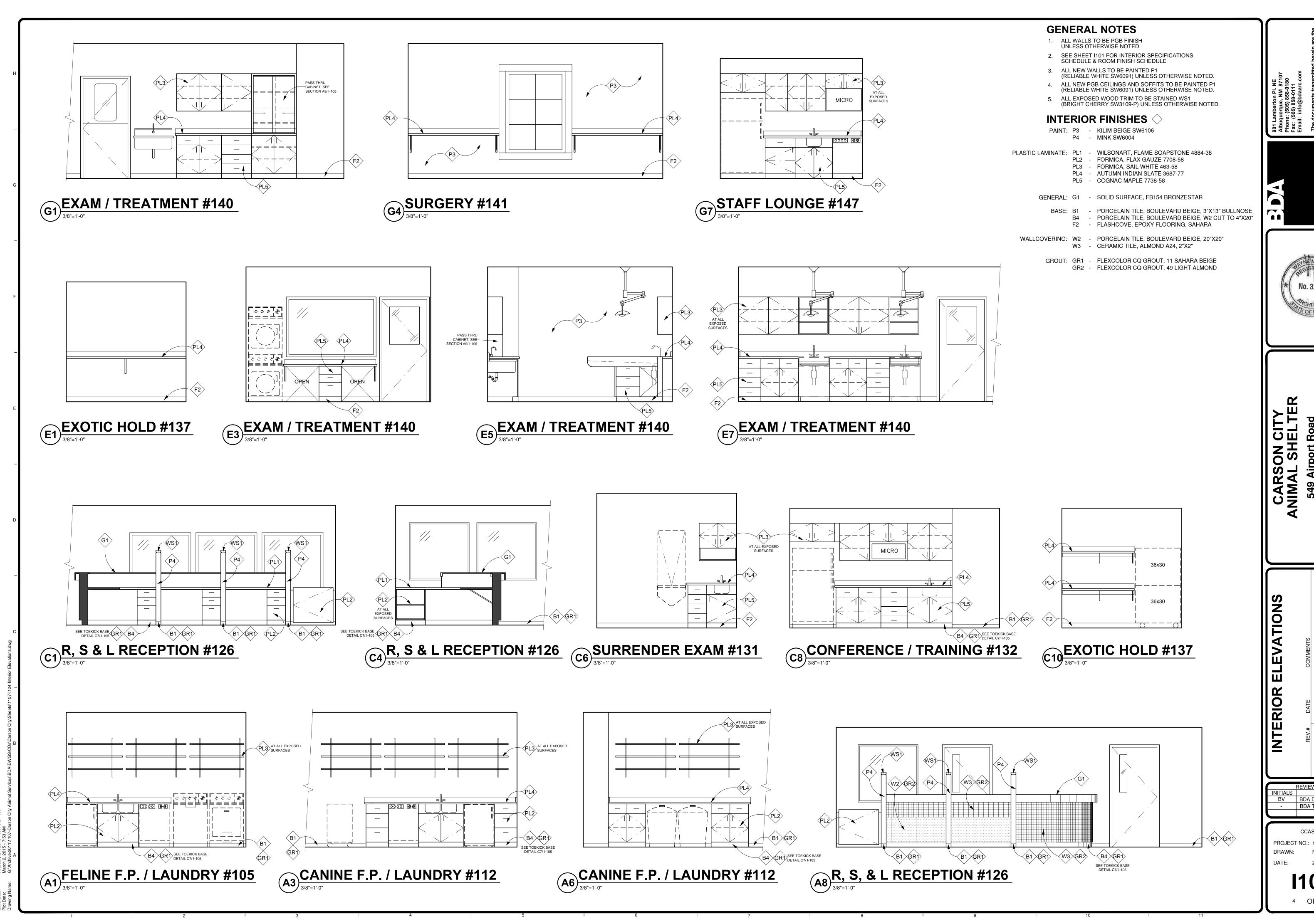
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REVIEWS
INITIALS
BV BDA DSGN. REV.
- BDA TECH REV.

CCAS
PROJECT NO.: 1107
DRAWN: NF

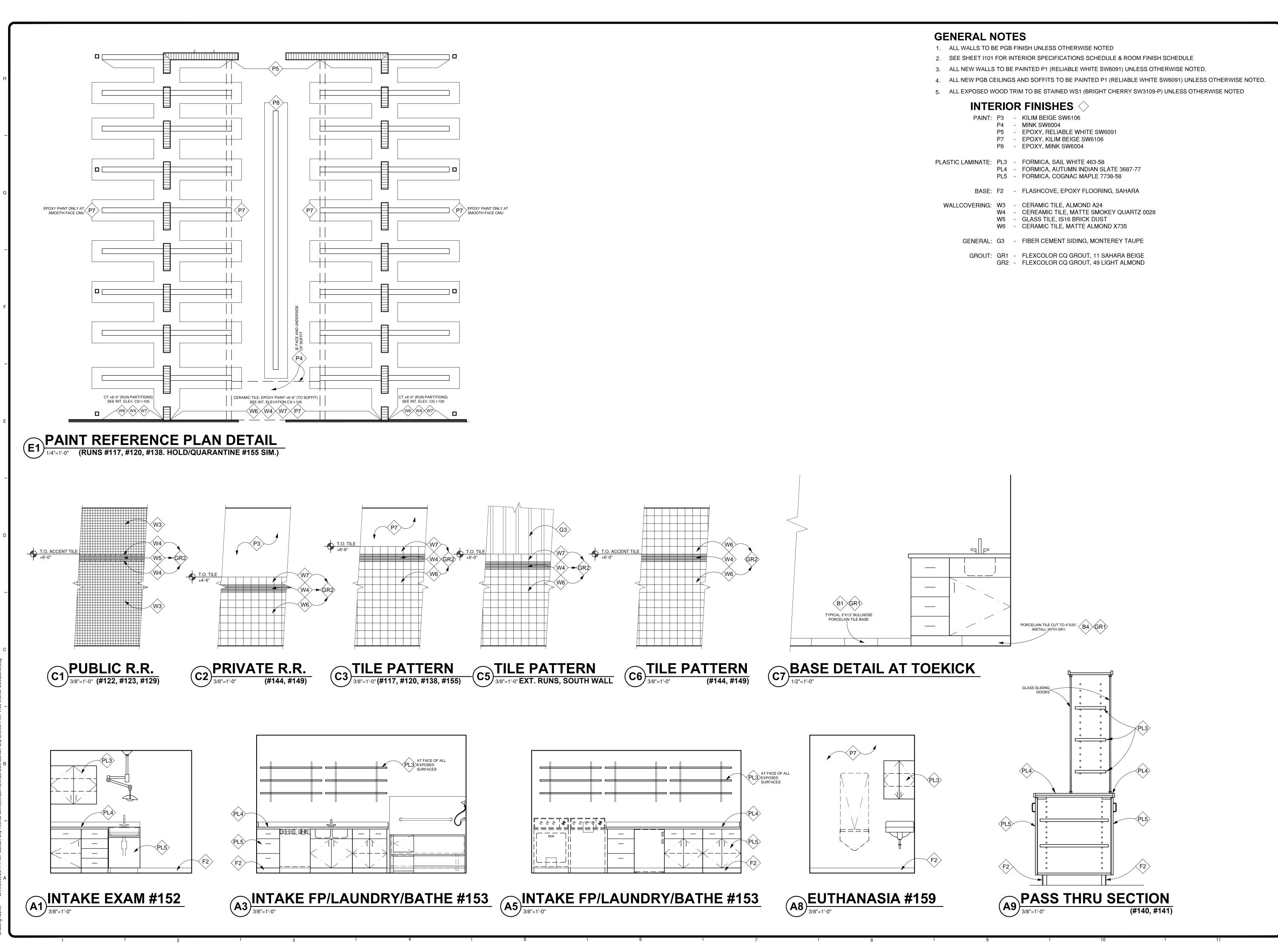
.WN: NF E: 2/27/201

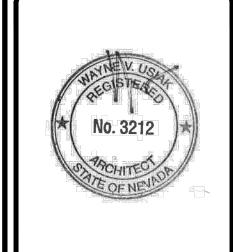
3 OF



BDA DSGN. REV. BDA TECH REV.

I104





ELEVATIONS INTERIOR

BV BDA DSGN. REV.
BDA TECH REV.

PROJECT NO.: 1107 2/27/2015

> **I105** 5 OF 5

	DESIGN CRITERIA			<u>GENERAL</u>	NOTES			SUPPLEMENTARY NOT	<u>ES</u>
1	NING CODES AND MANUALS: INTERNATIONAL BUILDING CODE (2012) WITH AMENDMENTS / AS AISC MANUAL OF STEEL CONSTRUCTION — (9TH EDITION) ACI BUILDING CODE REQUIREMENTS FOR	CCE 7-10	PROVIDE ALL MEASURES INCLUDE, BUT ARE NOT STRESSES AND TO HOL	S NECESSARY TO PROTECT THE STRUCTURE, W LIMITED TO, MEANS AND METHODS, BRACING, S	ORKERS AND OTHER PERSONS SHORING, FORMS, SCAFFOLDING MOLITION/CONSTRUCTION. OBSE	METHOD OF CONSTRUCTION. THE CONTRACTOR SHALL S DURING CONSTRUCTION. SUCH MEASURES SHALL G, GUYING OR OTHER MEANS TO AVOID EXCESSIVE SERVATION VISITS TO THE SITE BY THE STRUCTURAL	FOLLOWING OR APPROVED EQUIVALENT		ED ON THE CONSTRUCTION DOCUMENTS SHALL BE ONE OF THE
	STRUCTURAL REINFORCED CONCRETE. ACI 318 - 08. NATIONAL DESIGN SPECIFICATION FOR WOOD (2005). LATEST CODE AND SPECIFICATIONS FOR REINFORCED MASONRY		APPLY UNLESS SPECIFICFULLY SHOWN OR NOTE	CALLY SHOWN OR NOTED TYPICAL DETAILS ANI D SHALL BE SIMILAR TO DETAILS SHOWN FOR S	D NOTES ON THESE SHEETS SI SIMILAR CONDITIONS. ALL WORK	SHALL OTHERWISE. CONSTRUCTION DETAILS NOT K, MATERIALS AND CONSTRUCTION SHALL	ESR #1967 ER #5330 ESR #2508	POWERS/RAWL POWER-FAST SIMPSON EPOXY-TIE SET (XP)	
	BUILDING CATEGORY II (IBC, TBL 1604.5). IMPORTANCE FACTORS: Ie (SEISMIC) = 1.0, Is (SNOW) = 1.0, Iw (WIND) = 1.0. LOAD COMBINATIONS:	.0	 WRITING DURING THE B DRAWINGS OR IN THE S 	LICABLE BUILDING CODES, REGULATIONS AND SA IDDING PERIOD OF ANY THE CONTRACTOR SHA SPECIFICATIONS OR OF ANY VARIATIONS REGULA RULES AND CONCERNED. ANY SUCH DISCREPANC	ALL INFORM THE ENGINEER IN C ATIONS. UPON RECEIPT OF SUC	CH INFORMATION, NEEDED IN ORDER TO			UFACTURER AND THE APPLICABLE ER REPORT. WHERE SPALLING IS DRILLED HOLE. CONTRACTOR SHALL HAVE ER REPORT ON—SITE DURING
•	BASIC STRENGTH DESIGN (IBC, 1605.2.1). BASIC ALLOWABLE STRESS DESIGN (IBC, 1605.3.1). DEFLECTIONS (IBC, TBL 1604.3).		VARIATION NOT REPOR	TED SHALL BE THE RESPONSIBILITY OF THE CON	NTRACTOR.		EXPANSION BOLTS DRILLED AND INSTA THE FOLLOWING OR APPROVED EQUIVA		ONSTRUCTION DOCUMENTS SHALL BE ONE OF
ROOF.	DESIGN LOADS CAL LOADINGS :		PENETRATIONS SHALL B		M DRAWINGS MADE BY THEM. (ANICAL, ELECTRICAL AND PLUMBING PLANS. ALL OPENINGS AND CONTRACTOR SHALL NOT PROCEED WITH ANY WORK SHOWN PENINGS, SEE TYPICAL STRUCTURAL DETAILS.	ESR #1917 - ESR #2251- ESR #3260- ESR #1396-	HILTI KWIK-BOLT TZ ITW RAMSET / REDHEAD TRUBOLT POWERS/RAWL POWER-BOLT WEDGE-ALL	
TYPE 1 = SHINGLES/LINER/METAL TYPE 2 = CLAY TILE TYPE 3 = CONCRETE	DESCRIPTION DEAD LOAD (PSF) LIVE LOAD (PSF) ROOF		ARCHITECT PRIOR TO S		SHALL BE VERIFIED WITH ARCH	HITECTURAL DRAWINGS. RESOLVE ALL DISCREPANCIES WITH	INSTALLATION OF EXPANSION BOLTS S HAVE ER REPORT ON-SITE DURING AL		NUFACTURER AND THE APPLICABLE ICBO-ER REPORT. CONTRACTOR SHALL
ARAPET)	ROOF TYPE 1 20 20 ROOF TYPE 2 N/A N/A ROOF TYPE 3 N/A N/A N/A 30	IBC, TABLE 1607.1 IBC, TABLE 1607.1 IBC, TABLE 1607.1 (Pq) GROUND / NON-REDUCIBLE	THE CONTRACTOR SHAL	L VERIFY ALL DIMENSIONS, CONDITIONS AND ELE ATTENTION OF THE ENGINEER BEFORE PROCEED			• DEFFERED SUBMITTALS :		
#	N/A 21 N/A 14	(Pf) FLAT ROOF (Ps) SLOPED ROOF	OBTAINED PRIOR TO MA	SE FROM THE APPROVED PLANS AND SPECIFICATION OF BEDIAGES WITHOUT THE WITH THE WITHOUT THE WITH THE WITHOUT THE WIT			THE ARCHITECT AND ENGINEER OF RE	ELEMENTS SHALL BE SUBMITED AND REVIEWED BY THE C CORD SHALL REVIEW THESE SUBMITTALS PRIOR TO SUBMI OD TRUSSES — MUST BE SUBMITTED FOR REVIEW W/ MAI	ITAL TO THE CITY BUILDING OFFICIAL :
	N/A 35	PARAPETS/WALLS WIDTH OF PARAPET DRIFT W= 4.73	• THE MECHANICAL CONT	BE REPAIRED OR REPLACED AS DIRECTED. RACTOR SHALL COORDINATE INSTALLATION OF TABLES FOR SUPPORT STRUCTURES AND INSERTS.	THE REQUIRED INSERTS WITH T	THE GENERAL CONTRACTOR. REFER			
* FLOORS	FLOOR FLOOR TYPE 1 N/A FLOOR TYPE 2 N/A FLOOR TYPE 3 N/A	IBC, TABLE 1607.1 IBC, TABLE 1607.1 IBC, TABLE 1607.1		RACTOR SHALL FURNISH ALL NECESSARY STRUC	CTURES FOR MECHANICAL EQUI	IPMENT, HANGING DEVICES AND INSERTS			
TYPE 1 = WOOD SHEATHING TYPE 2 = METAL DECK/CONCRETE TYPE 3 = CONCRETE	TYP. FLOOR N/A EXIT FACILITIES N/A	IBC, TABLE 1607.1 INCLUDES CORRIDOR AND STAIRS		CONCEPTS, INC DRAFT			THE STRUCTURAL SHOP DRAWING REVIEW I	SHOP DRAWINGS S INTENDED TO HELP THE ENGINEER VERIFY HIS DESIGN OF	CONCEDT. THE DEVIEW IS ONLY FOR CENERAL
* INCLUDES COVERINGS	OFFICE FLOOR BALCONIES DECKS CONCENTRATED N/A N/A	IBC, TABLE 1607.1 IBC, TABLE 1607.1 IBC, TABLE 1607.1	JAN 9, 20 SOIL PRESSURES (ALL P	RESSURE TAKEN FROM SOILS REPORT AS MOST FOOTINGS (SPREAD)	CONSERVATIVE VALUES)	2500 PSF	CONFORMANCE WITH THE DESIGN CONCEPT SPECIFICATIONS, WHICH HAVE PRIORITY OVI FABRICATION PROCESSES, MEANS, METHO	AND DOES NOT RELIEVE THE CONTRACTOR FROM COMI ER SHOP DRAWINGS. THE CONTRACTOR IS RESPONSIBLE F DDS, TECHNIQUES, SAFETY AND COORDINATION OF THE WO	PLIANCE WITH THE DESIGN DRAWINGS AND OR CONFIRMED AND CORRELATED DIMENSIONS,
	STORAGE LIGHT = N/A HEAVY = N/A	IBC, TABLE 1607.1	DESCRIPTION OF ALLOWABLE SOIL PRESSURES (PER SOILS REPORT)	GROUND SLABS CONTINUOUS FOOTINGS		2500 PSF 2500 PSF		DRAWINGS AND THOSE OF HIS SUBCONTACTORS.	
** <u>WALLS</u> TYPE 1 = WOOD/METAL	WALL TYPE 1 EXT. = 16 INT. = 6						IF A CURSORY REVIEW SHOWS MAJOR ERROPLAN LAYOUTS SHOWING LOCATIONS OF I	ORS WHICH SHOULD HAVE BEEN FOUND BY THE CONTRACTEMS DETAILED ON THE SHOP DRAWINGS. ANY CHANGES	
TYPE 2 = MASONRY TYPE 3 = CONCRETE	WALL TYPE 2 EXT. = 70 INT. = 58 EXT. = 120 WALL TYPE 3						BE CONSIDERED REVIEWED AFTER ENGINEER RECORD IN A TIMELY MANNER NOT TO EXC		SHOP DRAWINGS WILL BE REVIEWED BY THE ENGINEER OF
** FINISHES (I.E. BRICK, STUCCO, GYP.) INCLUDE	IN1. = 90								
	ND LOADS: 1. BASIC WIND SPEED (3-SECOND GUST)	= 126 MPH	SEISMIC LOADS : STRUCTURAL IRREGULAR	RITIES N/A			REQUIRED SHOP DRAWING SUBMITTATE CONCRETE REINFORCING (LAYOUT) CONCRETE MIX DESIGN	ALS	
	 WIND EXPOSURE PRESSURE COEFFICIENTS INTERNAL (GCPi) 	C (PER BUILDING OFFICIAL).18	PLAN STRUCTURAL	·	VERTICAL IRREGULARITIES 1a - STIFFNESS IRRE	EGULARITY - SOFT STORY	PRE-ENGINEERED WOOD TRUSSES (LAYOU STRUCTURAL STEEL LAYOUT MASONRY WALL LAYOUT/REINFORCING	JT)	
	4. MAIN LATERAL FORCE RESISTING SYSTEM WINDWARD (WORST CASE)	= 27.6 PSF		ME TORSIONAL IRREGULARITY NTRANT CORNERS		REGULARITY - EXTREME SOFT STORY	MASONKI WALL LATOUT/REINFORGING		
ZONE A ZONE S ZONE A A A A A A A A A A A A A A A A A A A	LEEWARD (WORST CASE)	= 7.3 PSF		IRAGM DISCONTINUITY	2 - WEIGHT (MASS) 3 - VERTICAL GEON	OMETRY IRREGULARITY			
LE SE				OF-PLAN OFFSETS PARALLEL SYSTEMS	RESISTING EL				
TYPICAL EYTERIOR WALL ZONEO			ANALYSIS PROCEDURE:	EQUIVALENT STATIC METHOD	DYNAMIC ANALYSIS	Y IN CAPACITY — WEAK STORY			
TYPICAL EXTERIOR WALL ZONES	5		1. MAPPED SPE	CTRAL RESPONSE ACCELERATIONS Ss	= 2.35g (.2 SEC.	•			
ZONE 3 ZONE 2 ZONE 3	5. COMPONENTS & CLADDING (WALLS) AT CORNERS FOR DISTANCE OF THE LESSER OF 10% OF HEIGHT BUT NOT LESS THAN 4% OF HORIZ. BLDG. DIM. N			(BASED ON GEOTECHNICAL DATA) SPONSE COEFFICIENTS	= .808g (1.0 SEC. = D	i. RESPONSE)			
ZONE 3 ZONE 2 ZONE 3	ZONE 4 (MAX)	= -33 PSF - AWAY FROM CORNERS = -37 PSF - NEAR CORNERS	J. SPECIKAL RE	SDs SD1					
ZONE 2 ZONE 2 ZONE 2	ZONE 5 (MAX) 6. ROOF WIND LOAD UPLIFTS ZONE 1 (MAX)	(WITHIN A DISTANCE) = -29 PSF (UPLIFT) - GROSS	4. SEISMIC DESI5. RESPONSE MO	GN CATEGORY ODIFICATION FACTOR (R)	= E = 5 (SPECIAL MAS	SONRY SHEARWALLS)			
ZONE 3 ZONE 2 ZONE 3	ZONE 2 (MAX) ZONE 3 (MAX)	= -41 PSF (UPLIFT) - GROSS = -65 PSF (UPLIFT) - GROSS	6. SEISMIC RESP	PONSE COEFFICIENT [CS/1.4]	= .3 (ASD)				
TYPICAL PLAN OF ROOF ZONES									
ABBREVIATIONS								YMBOLS	
(THIS IS A COMPREHENSIVE LIST OF ABBREVIATION TO THE STATE OF T	NS, SOME OF WHICH MAY NOT APPEAR ON THESE DRAWI CLR CLEAR CMU CONCRETE MASONRY UNIT	NGS.) EL ELEVATION ELEV ELEVATOR	KLF KIPS PER LINI KSI KIPS PER SQL				UNLESS OTHERWISE NOTED	SECTION / ELEVATION	\$ STEPPED FOOTING
APA AMERICAN PLYWOOD ASSOCIATION A.S.T.M AMERICAN SOCIETY OF TESTING MATERIALS ARCH ARCHITECTURAL	COL COLUMN CONC CONCRETE CONN CONNECTION	E.N EDGE NAILING E.S EDGE SCREW EQ EQUAL	LL LIVE LOAD LLH LONG LEG HO LLV LONG LEG VEF	O.H. — OVERH RIZONTAL OPNG. — OPENI	HEAD ING	S.J STEEL JOIST WD SL SNOW LOAD WL		✓	© CENTER LINE
ASD ALLOWABLE STRESS DESIGN BLDG BUILDING BLK BLOCK	CONT CONTINUOUS CORR CORRIDOR DBL DOUBLE DTL DETAIL	EW EACH WAY EXIST EXISTING EXP EXPANSION	LONG. LONGITUDINAL LWC LIGHT WEIGHT MANUF MANUFACTURE		D PER LINEAR FOOT	SPECS SPECIFICATIONS W/O S.S STAINLESS STEEL WT STD STANDARD WWF	WITH OUT	FIELD WELD	P PLATE L ANGLE
BM BEAM B.N BOUNDARY NAILING B.O BOTTOM OF BTM BOTTOM	DIA DIAMETER DIM DIMENSION DL DEAD LOAD	F.D FLOOR DRAIN FDN FOUNDATION FTG FOOTING FT FEET	MAS MASONRY MAX MAXIMUM MIN MINIMUM MISC MISCELLANEOU	PSI POUNI	DS PER SQUARE FOOT DS PER SQUARE INCH	STL — STEEL TEMP — TEMPORARY T & B — TOP & BOTTOM T & G — TONGUE AND GROOVE	•	ELEVATION / TOP OF _ / BRG.	[] SQUARE Ø DIAMETER
BRG BEARING BRK BRICK C CHANNEL	D.T DOUBLE TEE / DRAIN TILE DWL DOWEL EA EACH	GALV GALVANIZE GA GAUGE HK HOOK	MTL METAL M.O MECHANICAL (N NORTH	OPENING REINF. — REINFO REQ'D REQUII	ORCING RED	THK THICK/ THICKENED T.O TOP OF TRANS TRANSVERSE	>	MOMENT CONNECTION	
CJ CONTROL JOINT CCJ CONSTRUCTION CONTROL JOINT	EF EACH FACE EJ EXPANSION JOINT	HORIZ. — HORIZONTAL HSS — HOLLOW STRUCTURAL SECTION INT — INTERIOR JST — JOIST	NTS NOT TO SCAL O.C ON CENTER	E S.A STUD SCHED SCHED		TS TUBE STEEL TYP TYPICAL	7///	SLAB DEPRESSION	JJK Group, Inc. Consulting Structural Engineers
		uuisi							3240 Juan Tabo Bldg. C Albuquerque, New Mexico 87111 tel. 505 296 5706 fax 505 296 1672 www.jjkgroup.com The following professional engineering submittal and its contents, including, without limitation, general notes, conditions, details, sections, calculations
									In terilowing protessional engineering submitted and its contents, including, without limitation, general notes, conditions, details, sections, colculations and acode item explanations (collectively, the "Submittal"), is intended for an extricted to the exclusive use and reference of the client to whom this Submittal is addressed ("Client") solely with reference to and in cornection with the project hereinafter described ("Project"), and is not to be reproduced, copied, disseminated, entered into a computer database or used part of or in cometion with any other document or submittal, or otherwise used in any form or manner or by any person or entity other than Client without the express, prior written permission of JMK Group, Inc. JMK Group, inc. hereby discions any and all responsibility, obligation, and liability of every kind and nature whatsoever, and howsever orising, to any person or entity in any vay relating to or arising out of any unauthorized use of the Submittal.

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PROJECT NO.: 1107 DRAWN: DATE: 2/10/15

> **S0.1** OF

SPECIAL INSPECTION NOTES AND TABLES

1704 SPECIAL INSPECTIONS:

1704.1 GENERAL: Where application is made for construction as described in this section, the owner or contractor shall employ one or more approved agencies to perform inspections during construction on the types of work listed under Section 1704. The special inspector shall be a qualified person who shall demonstrate competence, to the satisfaction of the building official, for the inspection of the particluar type of construction or operation requring special inspection.

1704.1.1 STATEMENT OF SPECIAL INSPECTIONS: The applicant shall submit a statement of special inspections prepared by the registered design professional in responsible charge in accordance with Section 107.1 as a doondition for issuance. This statement shall be in accordance with Section 1705.

1704.1.2 REPORT REQUIREMENT: Special inspectors shall keep records of inspections. The special inspector shall furnish inspection reports to the building official, and to the registered design professional in responsible charge. Reports shall indicate that work inspected was done in conformance to approved construction documents. Discrepancies shall be brought to the immediate attention of the contractor for correction. If the discrepancies are not corrected, the discrepancies shall be brought to the attention of the building official and to the registered design professional in responsible charge prior to the completion of that phase of the work. A final report documenting required special inspections and correction of any discrepancies noted in the inspections shall be submitted at a point in time agreed upon prior to the start of work by the applicant and the building official.

1704.2 INSPECTION OF FABRICATORS: Where fabrication of structural load-bearing members and assemblies is being performed on the premises of a fabricator's shop, special inspection of the fabricated items shall be required by this Section and as required elsewhere in this code.

1704.2.1 FABRICATION AND IMPLEMENTATION PROCEDURES: The special inspector shall verify that the fabricator maintains detailed fabrication and quality control procedures that provide a basis for inspection control of the workmanship and the fabricator's ability to conform to approved construction documents and referenced standards. The special inspector shall review the procedures for completeness and adequacy relative to the code requirements for the fabricator's scope of work.

1704.3 REQUIRED VERIFICATION AND I	NSPECTION OF	STEEL CONSTRUCTION	
VERIFICATION AND INSPECTION	CONTINUOUS/ PERIODIC	REFERENCED STANDARD*	IBC REFERENCE
1. MATERIAL VERIFICATION OF HIGH-S	TRENGTH BOLTS	S, NUTS AND WASHERS:	
A. Identification markings to conform to ASTM standards specified in the approved Construction Documents.	Р	AISC 360, Section A3.3 and applicable ASTM material standards	
B. Manufacturer's certificate of compliance required.	Р		
2. INSPECTION OF HIGH-STRENGTH BO	OLTING:		
A. Snug—tight joints	Р		
B. Pretensioned and slip—critical joints using turn—of—nut with matchmarking, twist off bolt or direct tension indicator methods of installation.	Р	AISC 360, Section M2.5	1704.3.3
C. pretensioned and slip—critical joints using turn—of—nut without matchmarking or calibrated wrench methods of installation.	С		
3. MATERIAL VERIFICATION OF STRUC	TURAL STEEL A	ND COLD-FORMED STEEL DECK:	1
A. For structural steel, identification markings to conform to AISC 360.	Р	AISC 360, Section M2.5	
B. For other steel, identification markings to conform to ASTM standards specified in the approved construction documents.	Р	Applicable ASTM material standards	
C. Manufacturer's certified test reports.	Р	Applicable ASTM material standards	
4. MATERIAL VERIFICATION OF WELD F	FILLER MATERIAL	_S:	
A. Identification markings to conform to AWS specification in the approved construction documents.	Р	AISC 350, Section A3.5 and applicable AWS A5 documents	
B. Manufacturer's certificate of compliance required.	Р	AISC 360, Section M2.5	
5. INSPECTION OF WELDING:			
A. Structural steel and cold—formed steel deck:			
Complete and partial joint penetration groove welds.	С		
2. Multipass fillet welds.	С	AUG 24.4	
3. Single-pass fillet welds > 5/16"	С	AWS D1.1	1704.3.1
4. Plug and slot welds.	С		
5. Single−pass fillet welds ≤ 5/16".	P		
6. Floor and roof deck welds.	Р	AWS D1.3	
B. Reinforcing steel:			
Verification of weldability of reinforcing steel other than ASTM A 706.	Р		
Reinforcing steel resisting flexural and axial forces in intermediate and special moment frames, and boundary	С		
elements of special structural walls and shear reinforcement.		AWS D1.4 ACI 318: Section 3.5.2	
3. Shear reinforcement.	С		
4. Other reinforcing steel.	Р		
6. INSPECTION OF STEEL FRAME JOIN	T DETAILS FOR	COMPLANCE	1
A. Details such as bracing and stiffening.	Р		
B. Member locations.	Р		1704.3.2
C. Application of joint details at each connection.	Р		
A. Where applicable, see also Sec	tion 1707.1, Sp	ecial inspection for seismic resistar	ice.

A. Where applicable, see also Section 1707.1, Special inspection for seismic resistance.	
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VERIFICATION AND INSPECTION	CONTINUOUS/ PERIODIC	REFERENCED STANDARD*	IBC REFEREN
 Inspection of reinforcing steel, including prestressing tendons, and placement. 	Р	ACI 318: 3.5, 7.1-7.7	1913.4
Inspection of reinforcing steel welding in accordance with Table 1704.3, Item 5b.		AWS D1.4 & ACI 318: 3.5.2	
3. Inspect bolts to be installed in concrete prior to and during placement of concrete where allowable loads have been increased or where strength design is used.	С	ACI 318: 8.1.3, 21.2.8	1911.5 , 1912.1
4. Inspection of anchors installed in hardened concrete.	Р	ACI 318: 3.8.6, 8.1.3, 21.2.8	1912.1
5. Verifying use of required design mix.		ACI 318: Ch. 4, 5.2-5.4	1904.2.2, 1913.2, 1913.
 At the time fresh concrete is sampled to fabricate specimens for strength tests, perform slump and air content tests, and determine the temperature of the concrete. 	С	ASTM C 172, ASTM C 31, & ACI 318: 5.6, 5.8	1913.10
7. Inspection of concrete and shotcrete placement for proper application techniques.	С	ACI 318: 5.9, 5.10	1913.6, 1913. 1913.8
8. Inspection for maintenance of specified curing temperature and techniques.	Р	ACI 318: 5.11-5.13	1913.9
Inspection of prestressed concrete: A. Application of prestressing forces.			
B. Grouting of bonded prestressing tendons in the seismic-force resisting system.	C C	ACI 318: 18.20 ACI 318: 18.18.4	
10. Erection of precast concrete members.	Р	ACI 318: Ch. 16	
 Verification of in—situ concrete strength, prior to stressing of tendons in posttensioned concrete and prior to removal of shores and forms from beams and structural slabs. 	P	ACI 318: 6.2	
 Inspect formwork for shape, location and dimensions of the concrete member being formed. 	Р	ACI 318: 6.1.1	

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INSPECTION TASK	CONTINUOUS/		REFERENCE FOR CRI	
	PERIODIC	IBC REFERENCE	ACI 530/ASCE 5/ TMS 402*	ACI 530/ASCE 6/ TMS 602*
Compliance with required inspectin provisions of the construction documents and the approved submittals shall be verified.	Р			ART. 1.5
Verification of I'm and I'AAC prior to construction except where specifically exempted by this code.	Р			ART. 1.4B
Verification of slump flow and VSI as delivered to the site for self—consolodating grout.	С			ART. 1.5B.1b.3
As masonry construction begins, the following shall be verified to ensure compliance:	Р			ART. 2.6A
A. Proportions of site—prepared mortar.	P			ART. 3.3B
B. Construction of mortar joints. C. Location of reinforcement, connectors, prestressing tendons and anchorages.	Р			ART. 3.4,3.6A
D. Prestressing technique.	Р			ART. 3.6B
E. Grade and size of prestressing tendons and anchorages.	Р			ART. 2.4B,2.4H
During construction the inspection program shall verify: A. Size and location of structural elements.	P P			ART. 3.3F
B. Type, size and location of anchors, including other details of anchorage of masonry to structural members, frames or other construction.	P		SEC. 1.2.2(e), 1.16.1	
C. Specified size, grade and type of reinforcement, anchor bolts, prestressing tendons and anchorages.	P		SEC. 1.15	ART. 2.4,3.4
D. Welding of reinforcing bars. E. Preparation, construction and protection of masonry during cold weather (temp.	С	SEC. 2104.3,	SEC. 2.1.9.7.2, 3.3.3.4(b)	
below 40 deg. F) or hot weather (temp. above 90 deg. F). F. Application and measurement of	Р	2104.4		ART. 1.8C,1.8D
prestressing force.	С			ART. 3.6B
6. Prior to grouting, the following shall be verified to ensure compliance: A. Grout space is clean.				
B. Placement of reinforcement and	Р			ART. 3.2D
connectors and prestressing tendons and anchorages. C. Proportions of site—prepared grout and	Р			ART. 3.4
prestressing grout for bonded tendons.	Р			ART. 2.6B
D. Construction of mortar joints.	P		+	ART. 3.3B
7. Grout placement shall be verified to ensure compliance with code and construction document provisions.	C			ART 3.5
A. Grouting of prestressing bonded tendons.	С			ART. 3.6C
8. Preparation of any required grout specimens, mortar specimens and/or prisms shall be observed.		SEC. 2105.2.2, 2105.3		ART. 1.5

Δ	The	specific	standards	referenced	are	those	listed	l in	Chanter	35	

	1704.6 WOOD CONSTRUCTION: Special inspections of the fabrication and assemblies shall be in accordance with Section 1704.2. Special	n process of prefabricated	wood structural elements assemblies shall be in
Ε	accordance with this section.	inpocutoria of arce built	additioned strain be in
	1704.6.1 HIGH-LOAD DIAPHRAGMS: High-load diaphragms design special inspections as indicated in Section 1704.1. The special insascertain whether it is of the grade and thickness shown on the verify the nominal size of framing members at adjoining panel edgratener lines and that the spacing between fasteners in each line	spector shall inspect the vapproved building plans. Ages, the nail or staple dia	vood structural panel sheathing to Additionally, the special inspector must meter and length, the number of
	1704.6.2 METAL-PLATE-CONNECTED WOOD TRUSSES SPANNING 60 greater, the special inspector shall verify that the temporary in member restraint/bracing are installed in accordance with the	nstallation restraint/bracin	g and the permanent individual truss
	1704.7 REQUIRED VERIFICATION AND INSPECTION OF	F SOILS	
	 VERIFICATION AND INSPECTION TASK	CONTINUOUS/ PERIODIC	
;	Verify materials below shallow foundations are adequate to achieve the design bearing capacity.	Р	
	Verify excavations are extended to proper depth and have reached proper material.	Р	
,	Perform classification and testing of compacted fill materials.	Р	
,	Verify use of proper materials, densities and lift thicknesses during placement and compaction of compacted fill.	С	
	5. Prior to placement of controlled fill, observe subgrade and verify that site has been prepared properly.	Р	
	1704.8 REQUIRED VERIFICATION AND INSPECTION OF		
	VERIFICATION AND INSPECTION TASK	CONTINUOUS/ PERIODIC	
	Verify element materials, sizes and lengths comply with the requirements.	С	
	Determine capacities of test elements and conduct additional load tests, as required.	С	
	3. Observe driving operations and maintain complete accurate	С	

records for each element.

volumes.

Section 1704.4.

4. Verify placement locations and plumbness, confirm type

design capacity, record tip and butt elevations and

and size of hammer, record number of blows per foot of penetration, determine required penetrations to achieve

document any damage to foundation element.	
5. For steel elements, perform additional inspections in accordance with Section 1704.3.	
6. For concrete elements and concrete—filled elements, perform additional inspections in accordance with Section 1704.4.	
7. For specialty elements, perform additional inspections as determined by the registered design professional in responsible charge.	
1704.9 REQUIRED VERIFICATION AND INSPECTION OF CAST-IN-	-PLACE DEEP FOUNDATION ELEMENT
VERIFICATION AND INSPECTION TASK	CONTINUOUS/ PERIODIC
VERIFICATION AND INSPECTION TASK 1. Observe drilling operations and maintain complete and accurate records for each element.	1

1704.12 SPRAYED FIRE RESISTANT MATERIALS Special inspections for sprayed fire-resistant materals applied ─ to structural elements and decks shall be in accordance with Sections 1704.12.1 through 1704.12.6. Special inspections shall be based on the fire-resistant design as designated in the approved construction documents.

and adequate end bearing strata capacity. Record concrete or grout

3. For concrete elements, perform additional inspections in accordance with

• 1705 SPECIAL INSPECTIONS FOR WIND RESISTANCE:

1705.4 WIND RESISTANCE: The statement of special inspections shall include wind requirements for structures constructed in the following areas: 1. In wind Exposure Category B, where the 3-second-gust basic wind speed is 120 miles per hour

(mph) (52.8 m/s) or greater. 2. In wind Exposure Category C, where the 3-second-gust basic wind speed is 110 mph (49 m/s) or greater.

1705.4.1 WIND REQUIREMENTS IN THE STATEMENT OF SPECIAL INSPECTIONS: When Section 1705.4 specifies that wind requirements be included, the statement of special inspections shall identify the main windforce—resisting systems and wind—resisting components subject to special inspections as specified in

1705.4.2 DETAILED REQUIREMENTS: The statement of special inspections shall include at least the following systems and components:

1. Roof cladding and roof framing connections.

2. Wall connections to roof and floor diaphragms and framing. 3. Roof and floor diaphragm systems, including collectors, drag struts and boundary elements. 4. Vertical windforce—resisting systems, including braced frames, moment frames and shear walls. 5. Windforce—resisting system connections to the foundation.

process of prefabricated wood structural elements psections of site-built assemblies shall be in

• 1707 SPECIAL INSPECTIONS FOR SEISMIC RESISTANCE: 1707.1 SPECIAL INSPECTIONS FOR SEISMIC RESISTANCE: Special inspections itemized in Sections 1707.2 through 1707.9, unless exempted by the exceptions of Section 1704.1, 1705.3, or 1705.3.1, are required for

> the following: 1. The seismic—force—resisting systems in structures assigned to Seismic Design Category C, D, E or F, as determined in Section 1613. 2. Designated seismic systems in structures assigned to Seismic Design Category D, E or F. 3. Architectural, mechanical and electrical components in structures assigned to Seismic Design Category C, D, E or F that are required in Sections 1707.6 and 1707.7.

1707.2 STRUCTURAL STEEL: Continuous special inspection is required for structural welding in accordance with AISC 341. Exceptions: 1. Special inspections of structural steel in structures assigned to Seismic Design Category C that are not specifically detailed for seismic resistance, with a response modification coefficient, R, of 3 or less, excluding cantilver column systems.

2. For ordinary moment frames, ultrasonic and magnetic particle testing of complete joint penetration groove welds are only required for demand critical welds.

1707.3 STRUCTURAL WOOD: Continuous special inspection is required during field gluing operations of elements of the seismic-force-resisting system. Periodic special inspection is required for nailing, bolting, anchoring and other fastening of components within the system, including wood shear walls, wood diaphragms, drag struts, braces, shear panels and hold-downs.

Exception: Special inspection is not required for wood shear walls, shear panels and diaphragms, including nailing, bolting, anchoring and other fastening to other components of the seismic-force-resisting system, where the fastener spacing of the sheathing is more than 4 inches (102 mm) on center (o.c.). 1707.4. COLD-FORMED STEEL LIGHT-FRAME CONSTRUCTION: Periodic special inspection is required during

welding operations of elements of the seismic-force-resisting system. Periodic special inspection is required for screw attachment, bolting, anchoring and other fastening of components within the seismic-force resisting system, including shear walls, braces, diaphragms, collectors (drag struts) and hold-downs. Exception: Special inpsection is not required for cold-formed steel light-frame shear walls, braces, diaphragms, collectors (drag strust) and hold-downs where either of the following apply: 1. The sheathing is gypsum board or fiberboard.

2. The sheathing is wood structural panel or steel sheets on only one side of the shear wall, shear panel or diaphragm assembly and the fastener spacing of the sheathing is more than 4 inches o.c. 1707.5. STORAGE RACKS AND ACCESS FLOORS: Periodic special inspection is required during the anchorage

of access floors and storage racks 8 feet (2438mm) or greater in height in structures assigned to Seismic Design Category D, E or F. 1707.6 ARCHITECTURAL COMPONENTS: Periodic special inspection is required during the erection and fastening

of exterior cladding, interior and exterior nonbearing walls and interior and exterior veneer in structures assigned to Seismic Design Category D, E or F. Exceptions: 1. Special inspection is not required for architectural components in structures 30 feet (9144mm)

or less in height. 2. Special inspection is not required for cladding and veneer weighing 5psf (24.5 n/m²) or less. 3. Special inspection is not required for interior nonbearing walls weighing 15psf (73.5 n/m²) or

1707.8. MECHANICAL AND ELECTRICAL COMPONENTS Special inspection for mechanical and electrical equipment shall be as follows: 1. Periodic special inspection is required during the anchorage of ectrical equipement for emergency or standby power systems in structures assigned to Seismic Design Category C, D, E or F. 2. Periodic special inspection is required during the installation of chorage of other electrical equipement in structures assigned to Seismic Design Category E or F.

3. Periodic special inspection is required during installation of piping systems intended to carry flammable, combustible or highly toxic contents and their associated mechanical units in structures assigned to Seismic Design Category C, D, E or F. 4. Periodic special inspection is required during the installation of HVAC ductwork that will contain hazardous materials in structures assigned to Seismic Design Category C, D, E OR F.

5. Periodic special inspection is required during the installation of vibration isolation systems in structures assigned to Seismic Design Category C, D, E OR F where the construction documents require a nominal clearance of 0.25 inches (6.4mm) or less between the equipment support frame

1707.8. DESIGNATED SEISMIC SYSTEM VERIFICATIONS: The special inspector shall examine designated seismic systems requiring seismic qualification in accordance with Section 1708.4 and verify that the label, anchorage or mounting conforms to the certificate of compliance.

1707.9. SEISMIC ISOLATION SYSTEM: Periodic special inspection is required during the fabrication and installation of isolator units and energy dissipation devices that are part of the seismic isolation system.

 NON-DESTRUCTIVE TESTING ALL COMPLETE PENETRATION GROOVE WELDS CONTAINED IN JOINTS AND SPLICES SHALL BE TESTED 100%

EITHER BY ULTRASONIC TESTING OR BY RADIOGRAPHY.

RECOMMENDED MINIMUM (U.O.N.) SAMPLING AND TESTING FREQUENCIES

IT IS RECOMMENDED THAT THE SITE PREPARATION, FOUNDATION CONSTRUCTION AND FLOOR SLAB CONSTRUCTION BE MONITORED BY THE GEOTECHNICAL ENGINEER OR HIS REPRESENTATIVE. THE FOLLOWING ARE RECOMMENDED MINIMUM SAMPLING AND TESTING FREQUENCIES.

EARTHWORK - AT LEAST ONE MOISTURE-DENSITY (PROCTOR) TEST, ATTERBERG LIMITS TEST AND PERCENT FINER THAN #200 SIEVE TEST SHOULD BE PERFORMED PER EACH SOIL TYPE SUCH AS SUBGRADE AND SELECT FILL. - IN STRUCTURE AREAS, AT LEAST 1 DENSITY AND MOISTURE CONTENT TEST PER 2000 SQUARE FEET OF SURFACE AREA SHOULD BE PERFORMED ON THE SUBGRADE SOILS, AND AT LEAST 1 DENSITY AND MOISTURE CONTENT TEST PER 2000 SQUARE FEET OF SURFACE AREA SHOULD BE PERFORMED FOR EACH COMPACTED 6-INCH THICK LAYER OF FILL. TESTING BACKFILL TRENCHES SHOULD BE AT LEAST 1 DENSITY AND MOISTURE CONTENT TEST PER 100 LINEAR FEET OF TRENCH PER 6-INCH COMPACTED FILL

- A MINIMUM OF AT LEAST FIVE (5) DENSITY AND MOISTURE CONTENT TESTS SHOULD BE PERFORMED IN THE BUILDING AREA ON THE SUBGRADE SOILS, AND A MINIMUM OF AT LEAST FIVE (5) DENSITY AND MOISTURE CONTENT TESTS SHOULD BE PERFORMED PER 6-INCH COMPACTED THICKNESS OF FILL IN THE BUILDING AREA. TESTING OF BACKFILLED TRENCHES SHOULD BE AT LEAST 1 DENSITY AND MOISTURE CONTENT TEST PER 100 LINEAR FEET OF TRENCH PER 6-INCH COMPACTED FILL THICKNESS. - It is imperative that a qualified field technician be on-site during all soil processing and PLACEMENT.

- AT LEAST 1 SLUMP, AIR CONTENT AND TEMPERATURE TEST SHOULD BE PERFORMED PER 50 CUBIC YARDS OF EACH TYPE OF CONCRETE PLACED EACH DAY INCLUDING WHEN CONCRETE TEST CYLINDERS ARE MOLDED.

- AT LEAST 1 SET OF 4 CONCRETE TEST CYLINDERS SHOULD BE MOLDED FOR EACH TYPE OF CONCRETE PER 50 CUBIC YARDS OR FRACTION THEREOF PLACED IN A DAY. - EACH SET OF CYLINDERS SHOULD BE TESTED FOR COMPRESSIVE STRENGTH WITH 2 OF THE CYLINDERS TESTED AT 7 DAYS AND 2 OF THE CYLINDERS TESTED AT 28 DAYS. - REINFORCING STEEL SHOULD BE CHECKED FOR SIZE OF PLACEMENT PRIOR TO CONCRETE PLACEMENT.

- THE DIMENSIONS OF EACH FOUNDATION INCLUDING REINFORCING STEEL SIZE AND PLACEMENT SHOULD BE CHECKED. - THE BEARING MATERIAL AT EACH FOUNDATION SHOULD BE CHECKED TO VERIFY THAT THE MATERIALS ARE SUITABLE FOR FOUNDATION SUPPORT.

THE ABOVE TYPICAL TESTING FREQUENCIES MAY BE MODIFIED AS

REQUIRED BY THE GEOTECHNICAL ENGINEER OF RECORD.

NOTE:

INDICATES SPECIAL INSPECTION IS REQUIRED.

SPECIAL INSPECTOR MUST SUBMIT REPORTS TO ENGINEER OF RECORD FOR APPROVAL PRIOR TO ISSUANCE OF CERTIFICATE OF OCCUPANCY.

CONTINUOUS SPECIAL INSPECTION (C):

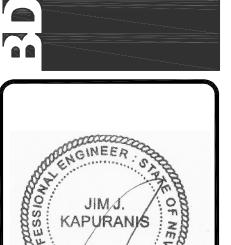
THE SPECIAL INSPECTOR IS ON SITE AT ALL TIMES WHILE WORK IS IN PROGRESS OBSERVING THE TASK REQUIRING SPECIAL INSPECTION.

PERIODIC SPECIAL INSPECTION (P):

THE SPECIAL INSPECTOR SHALL INSPECT THE WORK ACCORDING TO THE SCHEDULE OUTLINED IN THE PLANS AND SPECIFICATIONS, AND MAKE SURE THAT THE PERIODIC INSPECTION IS ADEQUATE TO SATISFY THE PURPOSE OF A CONTINUOUS INSPECTION ON THE PARTICULAR WORK AND APPROVED BY THE BUILDING OFFICIAL.

Consulting Structural Engineers

3240 Juan Tabo Blda, C Albuquerque, New Mexico 87111 tel. 505 296 5706 fax 505 296 1672 www.jikgroup.com The following professional engineering submittal and its contents, including, without limitation, general notes, conditions, details, sections, calculation and code item explanations [collectively, the "Submittal"), is intended for and restricted to the exclusive use and reference of the client to whom



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CCAS PROJECT NO.: 1107 DRAWN: DATE 2/10/15

CONCRETE

- PROTECT FRESHLY POURED CONCRETE FROM PREMATURE DRYING AND EXCESSIVE COLD AND HOT TEMPERATURES. START CURING AS SOON AS FREE WATER HAS DISAPPEARED FROM THE CONCRETE SURFACE AFTER PLACING AND FINISHING. ALL CURING PROCEDURES TO FOLLOW ACI 308.
- PROTECT CONCRETE FROM DAMAGE AND REDUCED STRENGTH CAUSED BY FROST, FREEZING ACTIONS AND LOW TEMPERATURES IN COMPLIANCE WITH ACI 306.
- PROTECT CONCRETE FROM DAMAGE AND REDUCED STRENGTH CAUSED BY HIGH TEMPERATURES IN COMPLIANCE WITH ACI 305. UNIFORMLY COOL WATER AND AGGREGATES BEFORE MIXING TO OBTAIN A CONCRETE MIXTURE TEMPERATURE OF NOT GREATER THAN 90 DEGREES FAHRENHEIT AT POINT OF PLACEMENT.

• REMOVE ALL DEBRIS FROM FORMS BEFORE POURING.

- CONCRETE SHALL NOT BE DROPPED THROUGH REINFORCING STEEL SO AS TO CAUSE SEGREGATION OF AGGREGATES. USE HOPPERS, CHUTES OR TRUNKS OF VARIOUS LENGTHS SO THAT THE FREE UNCONFINED FALL OF CONCRETE SHALL NOT EXCEED (5) FEET, AND A SUFFICIENT NUMBER SHALL BE USED TO ENSURE THE CONCRETE IS KEPT LEVEL AT ALL TIMES.
- ALL ITEMS TO BE CAST IN CONCRETE SUCH AS REINFORCING, DOWELS, BOLTS, ANCHORS, PIPES, SLEEVES ETC. SHALL BE SECURELY POSITIONED IN THE FORMS BEFORE PLACING THE
- REFER TO SHEET SO.2A FOR CONTROL/CONSTRUCTION AND ISOLATION
- ALL HARDROCK CONCRETE SHALL BE OF REGULAR WEIGHT OF
- 145 POUNDS PER CUBIC FOOT. AGGREGATE SIZE SHALL CONFORM TO ASTM C33.
- CONCRETE GROUT SHALL BE NON-SHRINKING WITH SUFFICIENT WATER TO ALLOW POURING. ULTIMATE COMPRESSIVE STRENGTH
- (F'c) AT (28) DAYS SHALL BE EQUAL TO 4000 PSI (MIN). ● NO MORE THAN 90 MINUTES SHALL ELAPSE BETWEEN CONCRETE BATCHING AND CONCRETE PLACEMENT UNLESS

APPROVED BY TESTING AGENCY. CONCRETE SHALL BE PLACED

• ONE GRADE OF CONCRETE SHALL BE POURED AT THE JOB SITE AT ANY ONE TIME.

WITHIN 15 MINUTES AFTER DISCHARGE.

- CONTRACTOR SHALL COORDINATE PLACEMENT OF ALL OPENINGS, CURBS, DOWELS, SLEEVES, CONDUITS, BOLTS AND EMBEDS REQUIRED WITH MECH., ELC., AND EQUIPMENT MANUFACTURER'S PRIOR TO PLACEMENT.
- NO ALUMINUM CONDUIT OR PRODUCTS CONTAINING ALUMINUM OR ANY OTHER MATERIAL INJURIOUS TO THE CONCRETE SHALL BE EMBEDDED IN
- CURING: PROVIDE 7—DAY MINIMUM CONTINUOUS CURE ON ALL CONCRETE SURFACES AS SPECIFIED.
- ALL CONCRETE TESTS INCLUDING AIR CONTENT, SLUMP, AND TEST CYLINDERS SHALL BE TAKEN AT THE POINT OF DISCHARGE AND FROM THE DISCHARGE END OF PUMP HOSE WHEN CONCRETE IS PUMPED.

ADMIXTURES

TYPE OF ADMIXTURE	<u>DESCRIPTION</u>
Accelerators (ASTM C494)	Accelerating setting and early—strength development.
Air detrainers	Decrease air content.
Air—entraining admixtures (ASTM C260)	Improve durability in freeze—thaw, deicer, sulfate and alkali—reactive environments; Improve workability by introducing air bubbles in concrete.
Alkali—aggregate reactivity inhibitors	Reduce alkali—aggregate reactivity expansion.
Coloring admixtures (ASTM C979)	Colored concrete.
Corrosion inhibitors (ASTM C1582)	Reduce steel corrosion activity in a chloride—laden environment.
Hydration control admixtures	Suspend and reactivate cement hydration with stabilizer and activator.
Retarding admixtures (ASTM C494 and AASHTO M 194, Type B)	Retard setting time.
Shrinkage reducers	Reduce drying shrinkage.
Superplasticizers (ASTM C1017, Type 1)	Increase flowability of concrete; Reduce water—cement ratio.
Water reducer (ASTM C494 and AASHTO M 194, Type A)	Reduce water content at least 59
Synthetic Micro Fibers	Reduce plastic and crazing cracks

CONCRETE QUALITY (ACI 318-08)

● CEMENT TYPE (ASTM C150)

TYPE OF PORTLAND CEMENT	<u>DESCRIPTION</u>
Туре І	General—purpose for pavements, floors, reinf. conc. buildings, bridges, tanks, reservoirs, pipe, masonry units and pre—cast conc. products.
Type IA (Exterior)	Type I cement w/ air entraining.
Type II	Used for moderate sulfate attack.
Type IIA	Type II cement w/ air entraining.
Type III	High early strength (one week or less).
Type IIIA	Type III w/ air entraining.
Type IV	Used where rate/amount of heat generated from hydration must be minimized.
Type V	Used for severe sulfate attack.

• COMPRESSIVE STRENGTH AND WATER-CEMENT RATIO (BY MASS)

(ACI 211.1 & 211.3)			
COMPRESSIVE STRENGTH	* EXPOSURE	WATER-CEME	NT RATIOS
@ 28 DAYS (PSI)	<u>CATEGORY</u>	NON-AIR ENTRAINED	AIR ENTRAINING
7000	F0, S0,	0.33	ı
6000	P0, C0	0.41	0.32
5000		0.48	0.40
4000		0.57	0.48
3000		0.68	0.59
2000		0.82	0.74
4000	P1, S1	0.50	ı
4500	F1, F2, F3, S2	0.45	_
5000	C2, S3	0.40	-

♠ AIR CONTENT (ACI 211.1) (%)

* EXPOSURE	AGGREGATE SIZE (IN)							
	3/8	1/2	3/4	1	1½	2	3	6
MILD	4.5	4.0	3.5	3.0	2.5	2.0	1.5	1.0
MODERATE (F1)	6.0	5.5	5.0	4.5	4.5	3.5	3.5	3.0
SEVERE (F2 & F3)	7.5	7.0	6.0	6.0	5.5	5.0	4.5	4.0

* EXPOSURE CATEGORY	EXPOSURE CONDITION
F0, S0, P0, C0	Concrete protected from exposure to freezing and thawing, application of deicing chemicals, or aggressive substances.
P1	Concrete intended to have low permeability when exposed to water.
F1, F2, F3	Concrete exposed to freezing and thawing in a moist condition or
C2	deicers. For corrosion protection for reinforced concrete exposed to chlorides from deicing salts, salt water, brackish water, seawater, or spray from these sources.

* SULFATE EXPOSURE CLASS		SULFATE (SO4) IN SOIL, % BY MASS	SULFATE (SO4) IN WATER, PPM
S0	Negligible	Less than 0.10	Less than 150
S1	Moderate**	0.10 to 0.20	150 to 1500
S2	Severe	0.20 to 2.00	1500 to 10,000
S3	Very Severe	Over 2.00	Over 10,000

● <u>SLUMP (ACI 211.1)</u>

TYPE OF CONSTRUCTION	<u>Slum</u> f	<u> (IN)</u>
	<u>MAXIMUM</u>	MINIMUM
Reinforced foundation walls and footings	6	3
Unreinforced footings, caissons, and sub-structure walls	4	1
Reinforced slabs, beams, and walls	6	3
Building columns	6	4
Pavements	3	1
Heavy mass construction	3	1
Bridge decks	4	3
Sidewalk, driveway, and slabs on ground	5	3

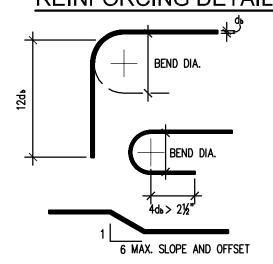
CONCRETE REINFORCING

- FABRICATION AND PLACEMENT OF REINFORCING STEEL SHALL BE IN ACCORDANCE WITH CRSI MSP-1 "MANUAL OF STANDARD PRACTICE" AND ACI 301 "SPECIFICATIONS FOR STRUCTURAL CONCRETE."
- ALL REINFORCING STEEL SHALL BE NEW STOCK DEFORMED BARS CONFORMING TO ASTM A615 AS FOLLOWS:
 - #3 BARS..... GRADE 40 #4 & LARGER BARS..... GRADE 60
- WELDED WIRE FABRIC SHALL CONFORM TO ASTM A185.
- ALL BENDS SHALL BE MADE COLD.
- ALL WALLS AND COLUMNS SHALL BE DOWELED INTO FOOTING WITH BARS OF THE SAME SIZE AND SPACING AS THE BARS ABOVE (U.O.N. ON
- ALL REINFORCING STEEL SHALL BE SECURELY WIRED AND PROPERLY SUPPORTED ABOVE THE GROUND AND AWAY FROM
- PROVIDE CORNER BARS THE SAME SIZE AND SPACING AS THE HORIZ. REINF. AT THE CORNERS AND INTERSECTION OF ALL WALLS, BEAMS AND FOOTINGS (U.O.N. ON PLANS).
- CONTINUOUS FOOTING REINFORCEMENT SHALL HAVE A MINIMUM LAP PER SCHEDULE AND THE SPLICES IN ADJACENT BARS SHALL NOT BE LESS THAN (3) FEET APART.
- ALL DIMENSIONS SHOWING THE LOCATION OF REINFORCING STEEL NOT NOTED AS "CLEAR" ARE TO CENTER OF STEEL. MINIMUM REBAR COVER FOR CONCRETE SHALL BE AS FOLLOWS:

	MIN COVER	TOLERANCES + OR -		
CAST AGAINST PERM. EXPOSED TO EARTH	3"	3/8"		
EXPOSED TO EARTH OR WEA	ATHER			
NO. 5 AND SMALLER BARS	1-1/2"	3/8"		
NO. 6 AND LARGER BARS	2"	3/8"		
NOT EXPOSED TO EARTH OR SLABS, WALLS, JOISTS:	POSED TO EARTH OR WEATHER WALLS, JOISTS:			
NO. 11 AND SMALLER BARS	3/4"	3/8"		
NO. 14 AND NO. 18 BARS	1-1/2"	3/8"		

- TOLERANCES FOR LONGITUDINAL LOCATION OF BENDS AND ENDS OF REINFORCEMENT SHALL BE PLUS OR MINUS (2) INCHES EXCEPT AT DIS- CONTINUOUS ENDS OF MEMBERS WHERE TOLERANCES SHALL BE PLUS OR MINUS 1/2 INCH.
- REINFORCING FOR CONCRETE POURED ON GRADE SHALL BE SUPPORTED BY STEEL CHAIRS.

REINFORCING DETAILS



STANDARD HOOKS FOR	R PRIMARY REINFORCEMENT
BAR SIZE NO.	MINIMUM FINISHED BEND DIAMETER*
3 through 8	6dь
9, 10, 11	8dь
14 and 18	10d _b
* measured on inside of b	ar
	d _b

←6d_b for No. 3 through No. 5 12d_b for No. 6 through No. 8

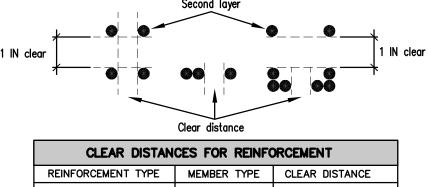
REQUIRED

INDICATES CONCRETE QUALITIES

NOTE:

CONCRETE REINFORCING (CONTINUED)

STANDARD HOOK	S FO	R STIRRU	PS & TIE REINFORCEMENT				
BAR SIZE NO).	MINIMUM	FINISHED BEND DIAMETER*				
3 through 5		4dь					
6 through 8			6dь				
* measured on inside of bar							
STIRRUP SIZE/SPACING - TYPICAL (U.O.N.)							
LATERAL TIES	STIR	RUP SIZE	SPACING				
MAIN REINF. (db)							
≤ #10		#3	Lesser of: a. 16db				
> #10		#4	b. 48 x Stirrup Dia. c. Least Col./Bm. Dim.				
SPIRAL TIES			_				
≤ #10		# 3	A4				
			Max spacina = 3"				



* If db < 1 inch, use 1 inch. Otherwise, use db. ** If 1.5db < 1½ inches, use 1½ inches. Otherwise, use 1.5db.

REINFORCING DEVELOPMENT LENGTHS / SPLICES

CONCRETE:

• TENSION SPLICE

L _s VALUES (IN), CLASS A										
BAR	f _c '(psi)									
SIZE	2000	3000	4000	5000	6000	7000	8000	9000	10000	
#3	27	22	21	21	21	21	21	21	21	
#4	36	29	26	23	21	21	21	21	21	
# 5	45	37	32	29	26	24	23	21	21	
#6	54	44	38	34	31	29	27	26	24	
# 7	63	52	45	40	36	34	32	30	28	
#8	72	59	51	46	42	39	36	34	32	
#9	81	67	58	52	47	44	41	38	36	
#10	92	75	65	58	53	49	46	43	41	

			L	, VALUES (IN), CLASS	S B			
BAR				f _c '	(psi)				
SIZE	2000	3000	4000	5000	6000	7000	8000	9000	10000
#3	35	29	27	27	27	27	27	27	27
#4	47	38	33	30	27	27	27	27	27
# 5	59	48	42	37	34	31	29	28	27
#6	70	58	50	45	41	38	35	33	32
# 7	82	67	58	52	47	44	41	39	37
#8	94	77	66	59	54	50	47	44	42
#9	106	86	75	67	61	57	53	50	47

DEVELOPMENT LENGTHS

				L _a VALI	JES (IN)					
BAR				f _c '	(psi)					
SIZE	2000	3000	4000	5000	6000	7000	8000	9000	10000	
# 3	27	22	21	21	21	21	21	21	21	
#4	36	29	26	23	21	21	21	21	21	
# 5	45	37	32	29	26	24	23	21	21] /
#6	54	44	38	34	31	29	27	26	24] /
# 7	63	52	45	40	36	34	32	30	28] \\
#8	72	59	51	46	42	39	36	34	32] \
#9	81	67	58	52	47	44	41	38	36] \
#10	92	75	65	58	53	49	46	43	41	SEE REINF. STANDARD HOOK DETAILS (S0.2
										1

ALL CONCRETE MIX DESIGN SUBMITTALS TO BE

STAMPED BY A REGISTERED PROFESSIONAL

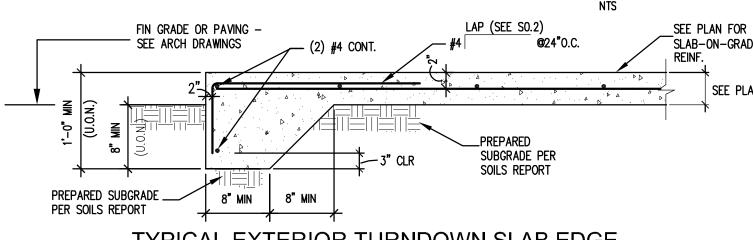
PROVIDER) PRIOR TO REVIEW AND WILL BE

ENGINEER (CONTRACTED BY MATERIAL

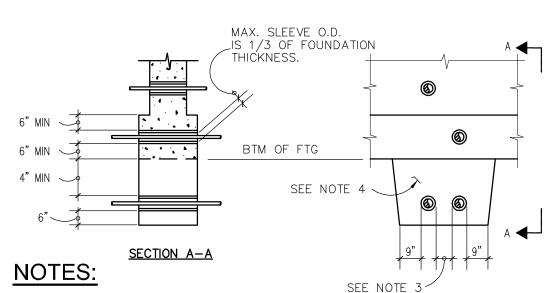
REVIEWED W/ THE ABOVE GUIDELINES.

Clear distance								
CLEAR DISTANCES FOR REINFORCEMENT								
REINFORCEMENT TYPE	MEMBER TYPE	CLEAR DISTANCE						
Deformed	Beams/Walls	* dь ≥ 1 IN						
bars	Columns	** 1.5dь > 1½						

NONSTAGGERED



MISC. CONCRETE DETAILS (CONTINUED)

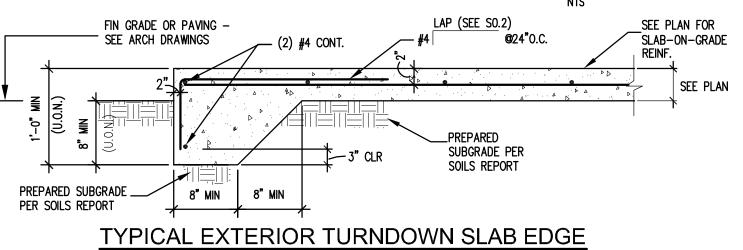


1. DO NOT PASS PIPES UNDER OR THROUGH ISOLATED FOUNDATIONS.

(SEE PLAN WHERE OCCURS)

- 2. PROVIDE SLEEVES FOR ALL PIPES PASSING THROUGH CONCRETE WITH I.D. 2" LARGER THAN 3. CLEARANCE BETWEEN ADJACENT SLEEVES SHALL BE LESS THAN DIA. OF LARGER SLEEVE OR
- 4. FOR PIPES BELOW FOUNDATIONS, EXCAVATE AS SHOWN AND FILL WITH CONCRETE PRIOR TO

TYPICAL PIPE PENETRATIONS THROUGH FOOTINGS



PLACING FOUNDATIONS.

KAPURANK

GENERAL DETAILS ONCRE Ü

BDA DSGN. REV.

CCAS PROJECT NO.: 1107 DRAWN: DATE: 2/10/15

OF

JJK Group, Inc. Consulting Structural Engineers

3240 Juan Tabo Bldg. C Albuquerque, New Mexico 87111 tel. 505 296 5706 fax 505 296 1672 www.jjkgroup.com

GENERAL

1. SAW-CUT JOINTS AS SOON AS THE SLAB WILL SUPPORT THE WEIGHT OF THE SAW AND OPERATOR WITHOUT DISTURBING THE FINAL FINISH.

2. THE DEPTH OF SAW-CUT WHEN USING A WET CUT SAW SHALL BE 1/4 THE SLAB THICKNESS. 3. PROVIDE CONSTRUCTION JOINT AT THE END OF CONCRETE PLACEMENT FOR THE DAY. SEE DETAIL A (THIS SHEET).

4. SEE FOUNDATION AND/OR CONTROL JOINT PLAN FOR ADDITIONAL INFORMATION.

5. SEE GENERAL NOTE SHEET SO.2 FOR ADDITIONAL CONCRETE REQUIREMENTS. 6. CONTRACTOR TO FAMILIARIZE HIMSELF WITH SOILS REPORT FOR SUBGRADE PREPARATION.

DESIGN

THE FOLLOWING MINIMUM ALLOWABLE REINFORCING RATIO USED IN THE DESIGN IS AS FOLLOWS:

.1 % FOR WELDED WIRE FABRIC .2 % FOR REINFORCING BARS

THIS MINIMUM REINFORCING RATIO IS FOR SHRINKAGE AND KEEPING RANDOM CRACKING TIGHT. IT ALSO ALLOWS FOR LONGER JOINT SPANS.

CONTROL /CONTRACTION JOINT SPACING WITH MINIMUM REINFORCING IS BASED ON THE FOLLOWING SUBGRADE DRAG FORMULA:

As = FLW / 2 Fs WHERE:

- CROSS-SECTION AREA OF STEEL, IN SQUARE INCHES PER LINEAL FOOT OF SLAB WIDTH
 - COEFFICIENT OF SUBGRADE FRICTION. (DESIGNERS USE 1.5 OR 2.0 FOR PAVEMENTS; 1.5 IS
- RECOMMENDED FOR CONCRETE FLOORS ON GROUND.) SLAB LENGTH (OR WIDTH IF APPROPRIATE) BETWEEN FREE ENDS, IN FEET.
- (A FREE END IS ANY JOINT FREE TO MOVE IN A HORIZONTAL PLANE.)
- WEIGHT OF SLAB, IN POUNDS PER SQUARE FOOT. (FOR NORMAL-WEIGHT CONCRETE, DESIGNERS USE 12.5 POUNDS PER INCH OF FLOOR THICKNESS.)
 - ALLOWABLE WORKING STRESS OF REINFORCEMENT, IN POUNDS PER SQUARE INCH.

(THE WORKING STRESS OF STEEL IS USUALLY 0.67 TO 0.75 TIMES THE YIELD STRENGTH OF THE STEEL IN POUNDS PER SQUARE INCH.)

DEFINITIONS

■ ISOLATION JOINTS

ISOLATION JOINTS ARE PLACED WHEREVER COMPLETE SEPARATION BETWEEN THE FLOOR AND ADJOINING CONCRETE IS NEEDED TO ALLOW THEM TO MOVE INDEPENDENTLY WITHOUT DAMAGE. ISOLATION JOINTS PERMIT HORIZONTAL AND VERTICAL MOVEMENT BETWEEN THE ABUTTING FACES OF THE FLOOR SLAB AND OTHER PARTS OF THE BUILDING BECAUSE THERE IS NO KEYWAY, BOND OR MECHANICAL CONSTRUCTION ACROSS THE JOINT.

CONTROL / CONTRACTION JOINTS

CONTROL JOINTS (ALSO CALLED CONTRACTION JOINTS) ACT TO RELIEVE STRESS AND WITH PROPER SPACING (SEE CONTROL JOINT PLAN) THEY ELIMINATE THE CAUSE OF UNCONTROLLED RANDOM CRACKING. THEY ALLOW HORIZONTAL MOVEMENT OF THE SLAB. THE OBJECTIVE IS TO FORM A PLANE OF WEAKNESS IN THE SLAB SO THAT THE CRACK WILL OCCUR ALONG THAT LINE AND NOWHERE ELSE. AS SHOWN ON SECTIONS ON THIS SHEET ALL SLAB REINFORCING MUST BE DISCONTINUOUS THROUGH JOINT. LOAD TRANSFER ACROSS THE CONSTRUCTION JOINT IS PROVIDED BY USE OF DOWELS (A BOND BREAKER IS USED ON ONE END TO ALLOW HORIZONTAL MOVEMENT).

CONSTRUCTION JOINT

CONSTRUCTION JOINTS ARE STOPPING PLACES AND FORM THE EDGE OF EACH DAY'S WORK. THEY FREQUENTLY ALIGN WITH CONTROL / CONTRACTION JOINTS OR ISOLATION JOINTS. WHENEVER CONTINUOUS CONCRETE PLACEMENT WILL BE INTERRUPTED FOR 30 MINUTES OR MORE, A BONDED OR TIED CONSTRUCTION JOINT SHOULD BE FORMED AND DEFORMED REINFORCING BARS ADDED. IF THE CONSTRUCTION JOINT OCCURS WITHIN THE PANEL (I.E. BETWEEN SPECIFIED CONTROL / CONTRACTION JOINTS) ALL REINFORCING MUST CONTINUE THROUGH THE CONSTRUCTION JOINT.

VISIBLE CONDITIONS THAT MAY OCCUR DURING CONSTRUCTION

RANDOM CRACKING

WHEN RANDOM CRACKING OCCURS ON A NEWLY PLACED SLAB, IT IS USUALLY RELATED TO IMPROPER TIMING OF JOINT SAWING. THE PURPOSE OF CUTTING THE SLAB IS TO INDUCE A CRACK BENEATH THE CUT.

CONCRETE NEEDS TO GAIN ADEQUATE STRENGTH BEFORE HAVING JOINTS CUT INTO IT. IDEALLY, THE TENSILE STRENGTH HOLDS THE SLAB TOGETHER. THE SAWCUT NOTCH CREATES A REDUCED SLAB SECTION, WHICH INCREASES TENSILE STRESSING IN THE CONCRETE BELOW THE NOTCH. IN THE REDUCED SECTION, THE TENSILE STRESS IS OF GREATER MAGNITUDE THAN THE CONCRETE TENSILE STRENGTH. THUS A CRACK OCCURS BELOW THE NOTCH. THE CRACK AND SAWCUT COMBINE TO RELIEVE THE STRESSES AND THUS PREVENT UNWANTED RANDOM CRACKING. BUT NEW CONCRETE IS ALWAYS TRYING TO SHRINK. AS THE SAWBLADE CUTS A JOINT IN THE CONCRETE, THE SAWCUT WEAKENS THE CONCRETE SLAB. IF SAWCUTTING IS STARTED WHEN CONTRACTION STRESS (AS A RESULT OF CONCRETE SHRINKAGE) IS GREAT AND TENSILE STRENGTH IS NOT YET ADEQUATE TO RESIST IT, CRACKS CAN JUMP AHEAD OF THE BLADE DURING JOINT CUTTING.

IF COOLING WATER (USED WITH WET SAWING) HITS THE WARM SLAB, IT CAN BE A THERMAL SHOCK THAT ADDS TO THE POTENTIAL FOR RANDOM CRACKING AHEAD OF THE

TO AVERT RANDOM CRACKING, SAWCUT JOINTING MUST BE DONE BEFORE CONCRETE COOLING AND DRYING STARTS, BUT AFTER SOME (TENSILE) STRENGTH HAS DEVELOPED (7 HOURS MAXIMUM AFTER CONCRETE IS POURED). THE NOTCH INSTALLED BY SAWCUTTING SHOULD BE DEEP ENOUGH SO THAT THE CRACK OCCURS BELOW THE SAWCUT (1/4 OF THE SLAB THICKNESS IS SUFFICIENT).

BLEEDING AND SET RETARDING

EXCESSIVE BLEEDING THAT OCCURS AFTER CONCRETE PLACING, STRIKEOFF, AND BULLFLOATING CAN DELAY SUBSEQUENT FINISHING STEPS. IN MOST INSTANCES, THE CAUSE OF EXCESSIVE BLEEDING IS DUE TO ONE OF THE FOLLOWING:

- A WATER-CEMENT RATIO THAT IS TOO HIGH
- POOR AGGREGATE GRADATION
- SLOW SET TIMES AMBIENT CONDITIONS THAT HINDER SURFACE WATER EVAPORATION: LOW TEMPERATURES, HIGH HUMIDITY, OR LACK OF AIR MOVEMENT

BLISTERING IS THE CONVEX RAISING OF THE SURFACE MORTAR LAYER WHILE THE CONCRETE IS STILL PLASTIC. THE BLISTERS ARE ATTRIBUTED TO SEALING THE FLOOR

SURFACE BEFORE ALL THE BLEEDWATER AND AIR HAVE ESCAPED.

SIMILAR TO BLISTERING, DELAMINATION OF SURFACE MORTAR CAN OCCUR DUE TO ENTRAPMENT OF BLEEDWATER AND AIR BELOW THE PREMATURELY SEALED MORTAR SURFACE. DELAMINATIONS AFFECT LARGER SURFACE AREAS THAN BLISTERS, AND ARE VERY DIFFICULT TO DETECT DURING FINISHING. THEY BECOME APPARENT AFTER CONCRETE SURFACE DRYING WHEN THE DELAMINATED AREA IS CRUSHED UNDER TRAFFIC. THE THICKNESS OF DELAMINATED MORTAR RANGES FROM ABOUT 3 MM TO 9 MM (1/8 IN TO 3/8 IN). THE AFFECTED AREA CAN BE ANYWHERE FROM A FEW SQUARE CENTIMETERS (INCHES) TO A FEW SQUARE METERS (YARDS).

IF THE CONCRETE HAS STIFFENED FROM THE TOP DOWN, AS IT OFTEN DOES WHEN WIND SPEEDS ARE HIGHER, THERE IS A TENDENCY TO FINISH THE SLAB TOO SOON, BEFORE BLEEDING IS COMPLETE. FINISHING OPERATIONS PERFORMED WHILE THE UNDERLYING CONCRETE IS STILL SOFT (AND BLEEDING) WILL SEAL THE SLAB SURFACE, POTENTIALLY TRAPPING BLEEDWATER AND LEADING TO DELAMINATIONS.

PLASTIC SHRINKAGE CRACKING

PLASTIC SHRINKAGE CRACKING IS DUE TO CONCRETE AT THE SURFACE DRYING (AND SHRINKING) BEFORE INITIAL SET OF THE CONCRETE OCCURS. PLASTIC SHRINKAGE OCCURS DURING AND AFTER FINISHING, USUALLY WHEN THERE IS RAPID EVAPORATION OF BLEED WATER. THE CONDITIONS THAT LEAD TO RAPID WATER EVAPORATION ARE LOW RELATIVE HUMIDITY, HIGH AIR TEMPERATURES, RAPID AIR MOVEMENT (WIND) ACROSS THE CONCRETE SURFACE, AND ELEVATED CONCRETE TEMPERATURES. UNDER THESE CONDITIONS THE CONCRETE SURFACE CAN CRUST OVER WHILE THE UNDERLYING CONCRETE IS STILL PLASTIC. AS PLASTIC SHRINKAGE CRACKS FORM, THEY START AT THE SURFACE AND MAY EXTEND SOME DEPTH INTO THE UNHARDENED CONCRETE. FLOATING THE CONCRETE SLAB CAN REPAIR PLASTIC SHRINKAGE CRACKS, BUT ONLY IF DONE IMMEDIATELY AS THE CRACKS OCCUR.

CRAZE CRACKS ARE FINE RANDOM CRACKS OR FISSURES IN A CONCRETE SURFACE. ON CONCRETE FLATWORK, THEY USUALLY EXTEND LESS THAN 3 MM (1/4 IN) BELOW THE SURFACE. THE CRACKS OCCUR WITHIN THE PASTE-RICH SURFACE MORTAR AND GENERALLY PASS THROUGH THE PASTE AND NOT THROUGH AGGREGATE PÁRTICLES. IT IS TYPICAL FOR THE CRACKS TO FORM A MAP PATTERN. THE NARROW CRACKS ARE SO FINE THAT THEY ARE DIFFICULT TO SEE. IN MANY INSTANCES, THEY ARE ONLY VISIBLE DURING THE DRYING PHASE OF A WETTED SURFACE OR WHEN A TRANSLUCENT COATING IS INSTALLED. CRAZE CRACKS ARE ATTRIBUTED TO INADEQUATE CURING THAT LEADS TO CONCRETE SURFACE DRYING AND COOLING BEFORE THE MORTAR HAS GAINED SUFFICIENT STRENGTH. THESE ARE COSMETIC BLEMISHES THAT GENERALLY HAVE NO EFFECT ON THE SERVICEABILITY OR DURABILITY OF THE FLOOR.

WHEN THE EDGES AND CORNERS OF A FLOOR SLAB ON GROUND DISH UPWARD IN THE ABSENCE OF ANY LOADS OTHER THAN GRAVITY, THE SLAB IS SAID TO BE CURLING. IT IS USUALLY ATTRIBUTED TO DIFFERENCES IN MOISTURE CONTENT OR TEMPERATURE FROM TOP TO BOTTOM WITHIN THE SLAB. THESE TEMPERATURE AND MOISTURE GRADIENTS DEVELOP BETWEEN THE TOP AND BOTTOM SURFACES AS THE CONCRETE IN A FLOOR SLAB HARDENS. THE SLAB WILL CURL UP IF THE TOP IS TRYING AND COOLING (SHORTENING) WHILE THE BOTTOM REMAINS MOIST AND WARM. UNDER OPPOSITE CONDITIONS, THE SLAB SHOULD THEORETICALLY CURL DOWN. DOWNWARD CURL AS SUCH, HOWEVER, DOES NO OCCUR DUE TO SUBBASE RESTRAINT.

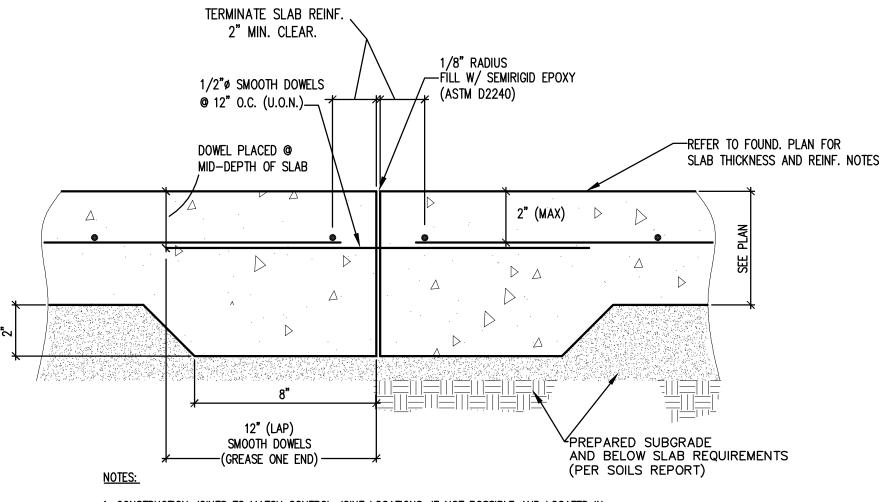
A POPOUT IS A CONICAL FRAGMENT THAT BREAKS OUT OF A CONCRETE SURFACE, LEAVING A HOLE. THE HOLE VARIES IN SIZE FROM 5 MM TO 50 MM (1/4 IN TO 2 IN), THOUGH LARGER POPOUTS ARE POSSIBLE. USUALLY, A FRACTURED AGGREGATE PARTICLE IS LOCATED AT THE BOTTOM OF THE HOLE. THE MATCHING PIECE OF THE FRACTURED PARTICLE ADHERES TO THE POINT OF THE POPOUT CONE. POPOUTS ARE CONSIDERED A COSMETIC DETRACTION AND GENERALLY DO NOT AFFECT THE SERVICE LIFE OF THE CONCRETE.

*CONTROL JOINTS - TYP. ISÓLATION JOINTS — TYP. - SEE 3/S0.2A 12'-0" (MAX) - U.O.N. * CONTROL JOINTS - TYP. \$0.2A TYPICAL BLOCK-OUT (WHERE OCCURS) TYPICAL PANEL SEE 4

* CONSTRUCTION JOINT LOCATION MAY ALSO OCCUR @ SAME LOCATION

√TYPICAL JOINT LAYOUT PLAN

(REFER TO CONTROL JOINT PLAN FOR ACTUAL LOCATIONS)



1. CONSTRUCTION JOINTS TO MATCH CONTROL JOINT LOCATIONS. IF NOT POSSIBLE AND LOCATED IN

2. REFER TO DEFINITIONS (THIS SHEET).

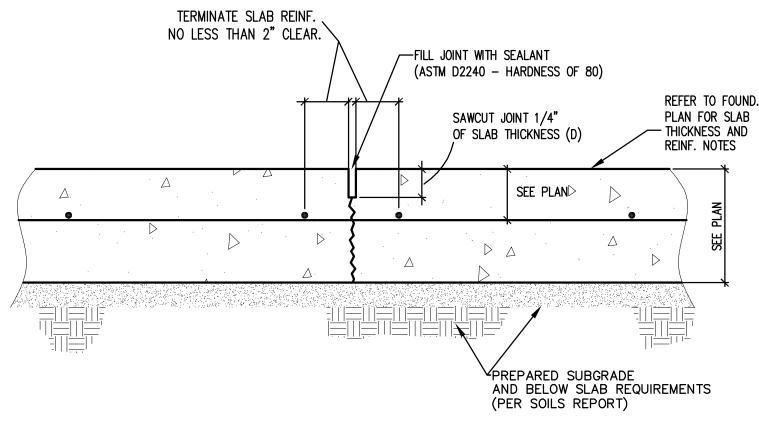
CONSTRUCTION JOINT SECTION

TYPICAL REINFORCING AT ROUND OPENINGS

PANEL AREA, CONTINUE ALL REINFORCING THRU JOINT.

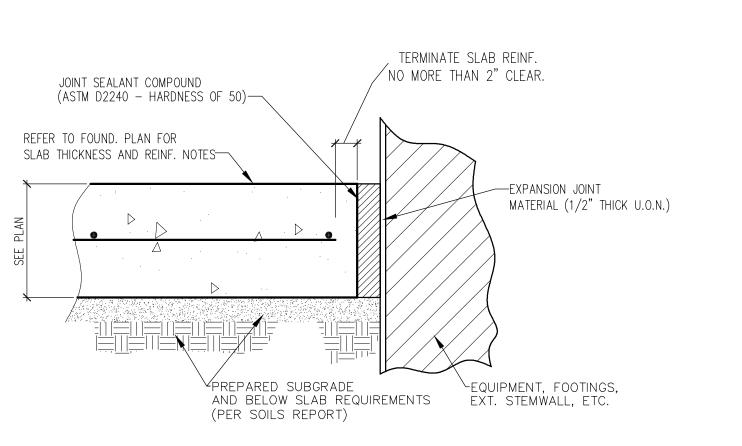
(TO OCCUR @ CONTROL JOINT LOCATION)

THE CONCRETE SLAB-ON-GRADE FOR THIS PROJECT IS A NON-LOAD BEARING ELEMENT AND THE ONLY DESIGN PARAMETERS USED IS FOR JOINT SPACING AND MINIMUM SHRINKAGE REINFORCEMENT. ALL CONCENTRATED OR UNIFORM LOADS IMPOSED ON THE SLAB-ON-GRADE HAVE DESIGNED FOOTINGS FOR THESE SITUATIONS.



- 1. SAWCUT JOINT MUST OCCUR WITHIN 7 HOURS OF CONCRETE POUR. 2. NOT USED.
- 3. SPACING OF JOINTS TO BE 12'-0" O.C. MAX EACH WAY (U.O.N.) ON PLAN. 4. REFER TO CONTROL JOINT PLAN OR FOUNDATION PLAN FOR JOINT LOCATIONS.
- 5. REFER TO DEFINITIONS (THIS SHEET)

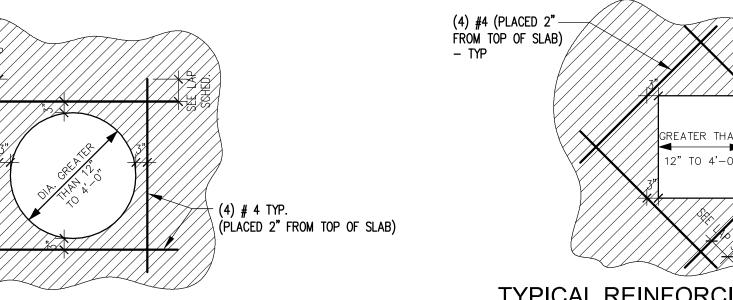
CONTROL / CONTRACTION JOINT SECTION



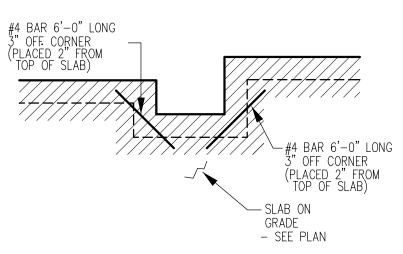
NOTES:

1. EXPANSION JOINT MATERIAL TO BE MINIMUM 1/2" THICK (U.O.N.). 2. REFER TO DEFINITIONS (THIS SHEET).

ISOLATION / EXPANSION JOINT SECTION



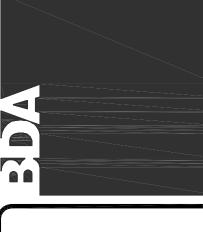
TYPICAL REINFORCING AT SQUARE 4A OR RECTANGULAR OPENINGS © 2 AT CONCRETE SLAB-ON - GRADE NTS

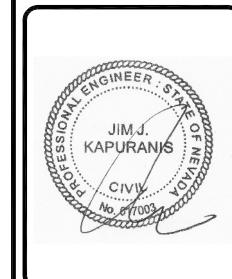


5 TYPICAL REINFORCING © RE-ENTRANT CORNERS NTS



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ZO 04 U W C

BDA DSGN. REV. BDA TECH REV.

2/10/15

CCAS PROJECT NO.: 1107 DRAWN:

DATE:

OF

TIMBER

- EACH PIECE OF STRUCTURAL LUMBER, SHEATHING AND TIMBER SHALL BE MARKED WITH THE GRADE BY SUCH COMPETENT AND RELIABLE ORGANIZATION WHOSE REGULAR BUSINESS IS TO ESTABLISH LUMBER GRADES.
- ALL LUMBER, EXCEPT WHERE SPECIFICALLY NOTED OTHERWISE, SHALL BE MILL SIZED AND SURFACED ON (4) SIDES. ALL SHALL BE STRAIGHT STOCK, FREE FROM WARP OR CUP, AND SINGLE LENGTH PIECES. SPLICES WILL NOT BE PERMITTED EXCEPT WHERE SPECIFICALLY SO DETAILED OR AS DIRECTED BY THE ENGINEER.
- ROUGH HARDWARE, JOIST HANGERS, STRAPS, HOLDOWNS, ETC. SHALL BE MANUFACTURED BY "SIMPSON" COMPANY OR APPROVED EQUAL. THE MAXIMUM SIZE AND NUMBER OF FASTENERS SPECIFIED BY THE MANUFACTURER SHALL BE USED UNLESS NOTED OTHERWISE.
- BLOCKING AND FIRESTOPPING TO BE INSTALLED AS REQUIRED TO SUPPORT ALL ITEMS OF FINISH SUCH AS BULKHEADS AND BUCKS. PROVIDE FIREBLOCKING TO CUT OFF ALL CONCEALED DRAFT OPENINGS, BOTH VERTICAL AND HORIZONTAL, BETWEEN CEILING AND FLOOR AREAS (AS REQUIRED BY BUILDING OFFICAL AND ARCHITECT).
- BOLTS (IF APPLICABLE) SHALL BE INSTALLED IN HOLES BORED WITH A BIT 1/16" LARGER THAN THE DIAMETER OF THE BOLT. BOLTS AND NUTS SEATING ON WOOD SHALL HAVE CUT STEEL WASHERS UNDER HEADS AND NUTS. NUTS SHALL BE PULLED TIGHT AND AGAIN CHECKED AND TIGHTENED JUST PRIOR TO ENCLOSING BOLTED MEMBERS. COUNTER BORE FOR BOLTED HEADS OR NUTS ONLY WHERE SO INDICATED ON THE DRAWINGS AND THEN TO SUFFICIENT DEPTH TO HOUSE THE BOLT HEAD OR NUT AND WASHER. CUT OFF EXCESSIVE BOLT PROJECTION WHERE NECESSARY. NICK THREADS TO PREVENT LOOSENING.
- LAG SCREWS (IF APPLICABLE) SHALL BE SCREWED AND NOT DRIVEN INTO PLACE. LAG SCREWS FASTÈNING ONE WOÓD MEMBER TO ANOTHER SHALL HAVE A PENETRATION INTO FAR MEMBER OF NOT LESS THAN (2/3) OF THE LENGTH OF THE LAG SCREW MEASURED UNDER THE HEAD U.O.N. IN PLACING LAG SCREWS IN WOOD, A HOLE SHALL FIRST BE BORED OF THE SAME DIAMETER AND DEPTH OF THE SHANK OF THE SCREW. AFTER WHICH THE HOLE SHALL BE CONTINUED TO A DEPTH EQUAL TO THE LENGTH OF THE LAG SCREW WITH A DIAMETER EQUAL TO THE DIAMETER OF THE SCREW AT THE ROOT OF THE THREAD.
- COMMON NAILS SHOULD BE USED WHEN NAILING IS SPECIFIED ON THESE PLANS (U.O.N.), SUCH AS AT SHEARWALLS AND DIAPHRAGMS. ALL OTHER NAILING MAY BE OF THE "BOX OR SINKER" TYPE.
- SHEATHING GRADE SHALL BE CD—X WITH EXTERIOR GLUE P.S. 1—83, U.O.N. ON PLANS:

DESCRIPTION	REQUIREMENTS
DESCRIPTION	REQUIREMENTS
ROOF SHEATHING	5/8" APA RATED T&G ROOF PLYWOOD SHEATHING. NAIL W/ 10d @ 6" O.C. BOUNDARY/EDGES AND 12" O.C. FIELD. (U.O.N. ON SHEARWALL SCHEDULE) SPAN INDEX = 48/24
WALL SHEATHING	1/2" APA RATED WALL PLYWOOD SHEATHING. NAIL W/ 10d @ 4" O.C. BOUNDARY/EDGES AND 12" O.C. FIELD. (U.O.N. ON SHEARWALL SCHEDULE)
	7/16" WAFERBOARD AND ORIENTED STRAND BOARD CONFORMING TO NER-108 AND PRODUCT STANDARD 2-92, AND WITH THE SAME EXPOSURE DURABILITY CLASSIFICATION, NOMINAL THICKNESS AND SPAN/INDEX RATIO MAY BE SUBSTITUTED FOR PLYWOOD %%JUONLY%%JU IF APPROVED BY THE STRUCTURAL ENGINEER.

NOTES:

-THE NAIL EDGE DISTANCE FOR 3" NOMINAL (2-1/2" ACTUAL) WIDE MEMBERS ON WHICH SHEETS ARE SPLICED SHALL BE 3/4" MIN.

-THE NAIL EDGE DISTANCE FOR 2" NOMINAL (1-1/2" ACTUAL) WIDE MEMBERS ON WHICH SHEETS ARE SPLICED SHALL BE 3/8" MIN. CARE SHALL BE MADE NOT TO SPLIT THE

-NAILS MAY BE SLANT DRIVEN TO MAINTAIN MINIMUM EDGE DISTANCE.

- ALL ROUGH CARPENTRY WILL PRODUCE JOINTS TRUE AND TIGHT AND WELL NAILED WITH MEMBERS ASSEMBLED IN ACCORDANCE WITH THE DRAWINGS AND ALL PERTINENT BUILDING CODES. THE SHIMMING OF SILLS, JOISTS SHORT STUDS, TRIMMERS, HEADERS OR OTHER FRAMING MEMBERS SHALL NOT BE PERMITTED. ALL WALLS AND PARTITIONS SHALL BE STRAIGHT, PLUMB AND ACCURATELY LOCATED. CAREFULLY SELECT ALL STRUCTURAL MEMBERS. INDIVIDUAL PIECES SHALL BE SELECTED SO THAT KNOTS AND OBVIOUS MINOR DEFECTS WILL NOT INTERFERE WITH THE PLACING OF BOLTS, OR PROPER NAILING OR THE MAKING OF SOUND CONNECTIONS. LUMBER MAY BE REJECTED BY THE ENGINEER FOR EXCESSIVE WARP, TWIST, BOW OR CROOK, MILDEW, FUNGUS OR MOLD AS WELL AS FOR IMPROPER GRADE MARKINGS, DEFECTS WHICH WILL RENDER A PIECE UNABLE TO SERVE ITS INTENDED FUNCTION SHALL BE DISCARDED.
- UNLESS OTHERWISE NOTED ON PLANS, LUMBER SHALL BE AT LEAST OF THE GRADES SHOWN IN THE TABLE BELOW. ALL LUMBER SHALL BE SURFACED AND FREE OF HEART CENTER. LUMBER SHALL MEET SPIECES AND COMMERCIAL GRADE AS INDICATED ON THE PLANS AND THE DESIGN VALUES FOR VISUALLY GRADED LUMBER IN ACCORDANCE WITH THE NATIONAL DESIGN SPECIFICATION BY THE NATIONAL FOREST PRODUCTS ASSOCIATION, WHICHEVER IS GREATER. BASED VALUES SHOWN MAY BE ADJUSTED IN ACCORDANCE WITH THE NATIONAL DESIGN SPECIFICATION. "DF" INDICATES DOUGLAS-FIR-LARCH, "HF" INDICATES HEM-FIR, "SPF" INDICATES SPRUCE-PINE-FIR.

TIMBER (CONT'D)

		MINIMUM LUMBEI				/DEI	O NIDO O	001 FD \		
7.05					S (PER NDS 2001 ED.) BASE VALUES					
TYPE	PRIMARY				BA					
	USE	(IN)	GRADE			(PSI)			
				Fb	Fv	*E	Fc//	Fc(perp)		
	STUDS	2x	DF # 2	700	180	1.4	850	625		
SAWN LUMBER	JOISTS	2" & WIDER	DF #2	900	180	1.6	1350	625		
	BEAMS	5"x5" & LARGER	DF #2	875	170	1.3	600	625		
	POSTS	5"x5" & LARGER	DF #2	750	170	1.3	700	625		
MICRO-LAMS	BEAMS	ANY	LAM	2400	275	1.8	2400	500		
PSL	BEAMS	ANY	LAM	2900	285	2.0	2900	750		

*1.0X10^6 (PSI)

- "GANG-NAIL" PRE-ENGINEERED TRUSSES ARE TO BE CONSTRUCTED WITH METAL PLATE CONNECTORS AND DESIGNED AND MANUFACTURED BY OTHERS. DESIGN, CONSTRUCTION, AND INSTALLATION SHALL MEET ALL APPLICABLE REQUIREMENTS OF THE NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION AND OF THE TRUSS PLATE INSTITUTE. PROVIDE ALL REQUIRED BLOCKING AND BRACING REQUIRED BY THE MANUFACTURER FOR CONSTRUCTION AND ERECTION IN ADDITION TO BLOCKING SHOWN ON THE STRUCTURAL DETAILS. MEMBERS OF A COMPLETED TRUSS ARE NEVER TO BE NOTCHED OR CUT. THE THE TRUSS MANUFACTURER SHALL PROVIDE DESIGN CALCULATIONS AND SHOP DRAWINGS SIGNED AND SEALED BY A STRUCTURAL ENGINEER (CONTRACTED BY TRUSS MANUFACTURER) FOR REVIEW PRIOR TO FABRICATION. THE DESIGN SHALL ACCOUNT FOR ALL UNIFORM LOADS AND EQUIPMENT LOADS. CONTACT THE STRUCTURAL ENGINEER FOR UNIFORM LOADING AND REQUIREMENTS IF REQUIRED.
- TRUSS SHOP DRAWINGS SHALL SHOW THE TRUSS DESIGN LOADS, SIZE AND GRADE OF THE CHORDS AND WEBS, LOCATIONS OF THE JOINTS AND CONNECTIONS, SIZE AND TYPE OF METAL PLATES AND ALL BRACING AND BLOCKING REQUIREMENTS.
- ROOF AND FLOOR TRUSSES SHALL BE DESIGNED FOR THE FOLLOWING CRITERIA:
 - ➤ SEE DESIGN LOADS SHEET SO.1 FOR VERTICAL LOADS ➤ STRESS INCREASE FOR DURATION OF LOAD - ROOF (15%)
- LOCATION OF TRUSS BRACING REQUIRED BY THE PLANS OR TRUSS MANUFACTURER'S DESIGN SHALL BE INDICATED ON EACH TRUSS BY PAINT MARKING.
- WHERE PARALLAM "PSL" MEMBERS ARE INDICATED ON THE PLANS AND SCHEDULES THEY SHALL BE MANUFACTURED BY TRUSS-JOIST MACMILLAN (NER-482 & ICBO ER-4979), OR BE AN APPROVED EQUAL PRODUCT. MEMBERS SHOWN ON THE PLANS AND SCHEDULES ARE DETERMINED FROM MANUFACTURER SUPPLIED INFORMATION AND SHOULD BE REVIEWED FOR COMPLIANCE BY THE MANUFACTURER'S CIVIL OR STRUCTURAL ENGINEER. LOADING INFORMATION MAY BE PROVIDED UPON REQUEST. NOTCHES, HOLES OR CUTS SHOWN IN THE TYPICAL DETAILS ARE ALLOWED WITHOUT ADDITIONAL APPROVAL; ALL OTHER MEMBER MODIFICATIONS ARE TO BE APPROVED BY THE STRUCTURAL ENGINEER.
- PLYWOOD WEB JOISTS NOTED "TJI" ARE TO BE MANUFACTURED BY TRUSS-JOIST MACMILLAN (NER-200 & ICBO ER-4354) OR APPROVED EQUAL. PROVIDE BLOCKING, WEB STIFFENERS, AND BRACING OVER THE SPAN PER THESE STRUCTURAL DRAWINGS AND ALL MANUFACTURER'S RECOMMENDATIONS. TOP AND BOTTOM FLANGES OF JOISTS ARE NEVER TO BE CUT AND ALL HOLES THROUGH THE JOIST WEB ARE TO BE SPECIFICALLY APPROVED BY STRUCURAL ENGINEER.

FASTENING	G SCHEDULE	
CONNECTION	FASTENING ^{a,m}	LOCATION
1. JOIST TO SILL GIRDER	3-8d common 3-3"x0.131" nails 3-3" 14 gage staples	toenail
2. BRIDGING TO JOIST	2-8d common 2-3"x0.131" nails 2-3" 14 gage staples	toenail each end
3. 1"X6" SUBFLOOR OR LESS TO EACH JOIST	2-8d common	face nail
4. WIDER THAN 1"X6" SUBFLOOR TO EACH JOIST	3-8d common	face nail
5. 2" SUBFLOOR TO JOIST OR GIRDER	2–16d common	blind and face na
6. SOLE PLATE TO JOIST OF BLOCKING	16d at 16" o.c. 3"x0.131" nails at 8" o.c. 3" 14 gage staples at 12" o.c.	typical face nail
SOLE PLATE TO JOIST OR BLOCKING AT BRACED WALL PANEL	3-16d at 16" 4-3"x 0.131"nails at 16" 4-3" 14 gage staples per 16"	braced wall panels
7. TOP PLATE TO STUD	2-16d common 3-3"x0.131"nails 3-3" 14 gage staples	end nail
8. STUD TO SOLE PLATE	4-8d common 4-3"x0.131" nails 3-3" 14 gage staples	toenail
	2—16d common 3—3"x0.131"nails 3—3" 14 gage staples	end nail
9. DOUBLE STUDS	16d at 24" o.c. 3"x0.131" nails at 8" o.c. 3" 14 gage staples at 8" o.c.	face nail
IO. DOUBLE TOP PLATES	16d at 16" o.c. 3"x0.131" nails at 12" o.c. 3" 14 gage staples at 12" o.c.	typical face nail
DOUBLE TOP PLATES	8-16d common 12-3"x0.131"nails 12-3" 14 gage staple typ. face nail	lap splice
11. BLOCKING BETWEEN JOISTS OR RAFTERS TO TOP PLATE	3-8d common 3-3"x0.131" nails 3-3" 14 gage staples	toenail
12. RIM JOIST TO TOP PLATE	8d at 6" (152mm) o.c. 3"x0.131" nail at 6" o.c. 3" 14 gage staple at 6" o.c.	toenail
13. TOP PLATES, LAPS AND INTERSECTIONS	2-16d common 3-3"x0.131"nails 3-3" 14 gage staples	face nail
14. CONTINUOUS HEADER, TWO PIECES	16d common	16" o.c. along ed
5. CEILING JOISTS TO PLATE	3-8d common 5-3"x0.131"nails 5-3" 14 gage staples	toenail
16. CONTINUOUS HEADER TO STUD	4-8d common 3-16d common minimum	toendii
17. CEILING JOISTS, LAPS OVER PARTITIONS (SEE SECTION 2308.10.4.1, TABLE 2308.10.4.1) 18. CEILING JOISTS TO PARALLEL RAFTERS	4-3" x0.131"nails 4-3" 14 gage staples 3-16d common minimum	face nail
(SEE SECTION 2308.10.4.1, TABLE 2308.10.4.1) 19. RAFTER TO PLATE	4-3" x0.131"nails 4-3" 14 gage staples 3-8d common	face nail
(SEE SECTION 2308.10.1, TABLE 2308.10.1) 20. 1" DIAGONAL BRACE TO EACH STUD AND PLATE	3-3"x0.131"nails 3-3" 14 gage staples 2-8d common	toenail
21. 1"X8" SHEATHING TO EACH BEARING WALL	2-3" x0.131"nails 2-3" 14 gage staples face nail 2-8d common	face nail
22. WIDER THAN 1"X8" SHEATHING TO EACH BEARING	3–8d common	face nail
23. BUILT-UP CORNER STUDS	16D common 3"x0.131"nails 3" 14 gage staples	24" o.c. 16" o.c. 16" o.c.
24. BUILT-UP GIRDER AND BEAMS	20d common 32" o.c. 3"x0.131" nails at 24" o.c. 3" 14 gage staples at 24" o.c.	face nail at top of bottom staggered opposite sides
	2-20d common 3-3"x0.131"nails 3-3" 14 gage staples	face nail at ends each splice
25. 2" PLANKS	16d common	at each bearing
26. COLLAR TIE TO RAFTER	3-10D common 4-3"x0.131"nails 4-3" 14 gage staples face nail	face nail
27. JACK RAFTER TO HIP	3-10d common 4-3"x0.131" nails 4-3" 14 gage staples	toenail
00 D00E D4FTFD TO 0 DV DV DT D7 D7	2-16d common 3-3"x0.131"nails 3-3" 14 gage staples	face nail
28. ROOF RAFTER TO 2-BY RIDGE BEAM	2-16d common 3-3"x0.131" nails 3-3" 14 gage staples	toenail
00 MOOT TO DAMP 12:27	2-16d common 3-3"x0.131"nails 3-3" 14 gage staples	face nail
29. JOIST TO BAND JOIST	3-16D common 5-3"x0.131"nails 5-3" 14 gage staples	face nail
30. LEDGER STRIP	3-16D common 4-3"x0.131"nails	face nail

4-3" 14 gage staples

FASTENING SCHEDULE								
CONNECTION	FAS	LOCATION						
31. WOOD STRUCTURAL PANELS AND PARTICLEBOARD, SUBFLOOR, ROOF AND WALL SHEATHING (TO FRAMING):	1/2" and less 19/32" to 3/4" 7/8" to 1"	6d ^{c,1} 2¾"x0.113" nail ⁿ 1¾"16 gage ^o 8d ^d or 6d ^e 2¾"x0.113" nail ^p 2" 16 gage ^p						
SINGLE FLOOR (COMBINATION SUBFLOOR-UNDERLAYMENT TO FRAMING):	1½" TO 1½" ¾" and less ½" to 1" 1½" to 1¼"	10d ^d or 8d ^e 6d ^e 8d ^e 10d ^d or 8d ^e						
32. PANEL SIDING (TO FRAMING)	1/2" or less 5%"	6d ^f 8d ^f						
33. FIBERBOARD SHEATHING: ⁹	½" ²⁵ ⁄ ₃₂ "	No. 11 gage roofing nail h 6d common nail No. 16 gage staple i No. 11 gage roofing nail h 8d common nail No. 16 gage staple i						
34. INTERIOR PANELING	1/4" 3/8"	4d ^j 6d ^k						

FOOT NOTES:

- a. COMMON OR BOX NAILS ARE PERMITTED TO BE USED EXCEPT WHERE OTHERWISE STATED. b. NAILS SPACED AT 6 INCHES ON CENTER AT EDGES, 12 INCHES AT INTERMEDIATE SUPPORTS EXCEPT 6 INCHES AT SUPPORTS WHERE SPANS ARE 48 INCHES OR MORE. FOR NAILING OF WOOD STRUCTURAL PANEL AND PARTICLEBOARD DIAPHRAGMS AND SHEARWALLS, REFER TO SECTION 2305. NAILS FOR WALL SHEATHING ARE PERMITTED TO BE
- COMMON, BOX OR CASING. c. COMMON OR DEFORMED SHANK.
- d. COMMON. e. DEFORMED SHANK.
- CORROSION-RESISTANT SIDING OR CASING NAIL.
- FASTENERS SPACES 3 INCHES ON CENTER AT EXTERIOR EDGES AND 6 INCHES O.C. AT INTERMEDIATE SUPPORTS. h. CORROSION-RESISTANT ROOFING NAILS WITH 7/16-INCH-DIAMETER HEAD AND 1 1/2-INCH LENGTH FOR 1/2-INCH SHEATHING AND 1 3/4-INCH LENGTH FOR 25/32-INCH SHEATHING.
- CORROSION-RESITANT STAPLES WITH NOMINAL 7/16-INCH CROWN AND 1 1/8-INCH LENGTH FOR 1/2-INCH SHEATHING AND 1 1/2-INCH LENGTH FOR 25/32-INCH SHEATHING. PANEL SUPPORTS AT 16 INCHES (20 INCHES IF STRENGTH AXIS IN THE LONG DIRECTION OF THE PANEL, UNLESS OTHERWISE MARKED).
- CASING OR FINISH NAILS SPACED 6 INCHES ON PANEL EDGES, 12 INCHES AT INTERMEDIATE SUPPORTS. PANEL SUPPORTS AT 24 INCHES. CASING OR FINISH NAILS SPACED AT 6 INCHES ON PANEL EDGES, 12 INCHES AT
- INTERMEDIATE SUPPORTS. FOR ROOF SHEATHING APPLICATIONS, 8d NAILS ARE THE MINIMUM REQUIRED FOR WOOD STRUCTURAL PANELS.
- m. STAPLES SHALL HAVE A MINIMUM CROWN WIDTH OF $\frac{7}{6}$ INCH n. FOR ROOF SHEATHING APPLICATIONS, FASTENERS SPACED 4 INCHES ON CENTER AT EDGES, 8 INCHES AT INTERMEDIATE
- o. FASTENERS SPACED 4 INCHES ON CENTER AT EDGES, 8 INCHES AT INTERMEDIATE SUPPORTS FOR SUBFLOOR AND WALL
- SHEATHING AND 3 INCHES ON CENTER AT EDGES, 6 INCHES AT INTERMEDIATE SUPPORTS FOR ROOF SHEATHING. p. FASTENERS SPACED 4 INCHES ON CENTER AT EDGES, 8 INCHES AT INTERMEDIATE SUPPORTS.

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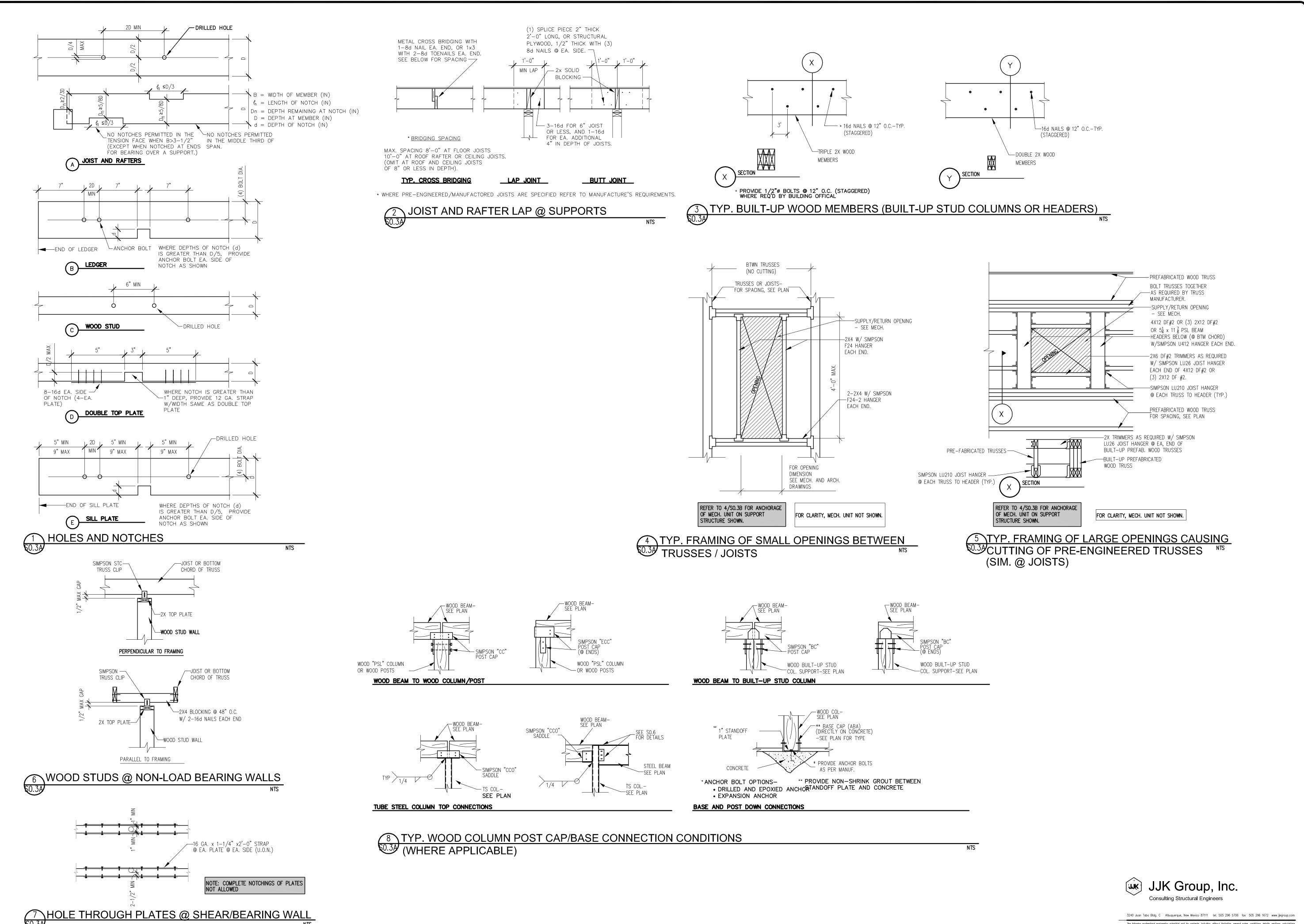
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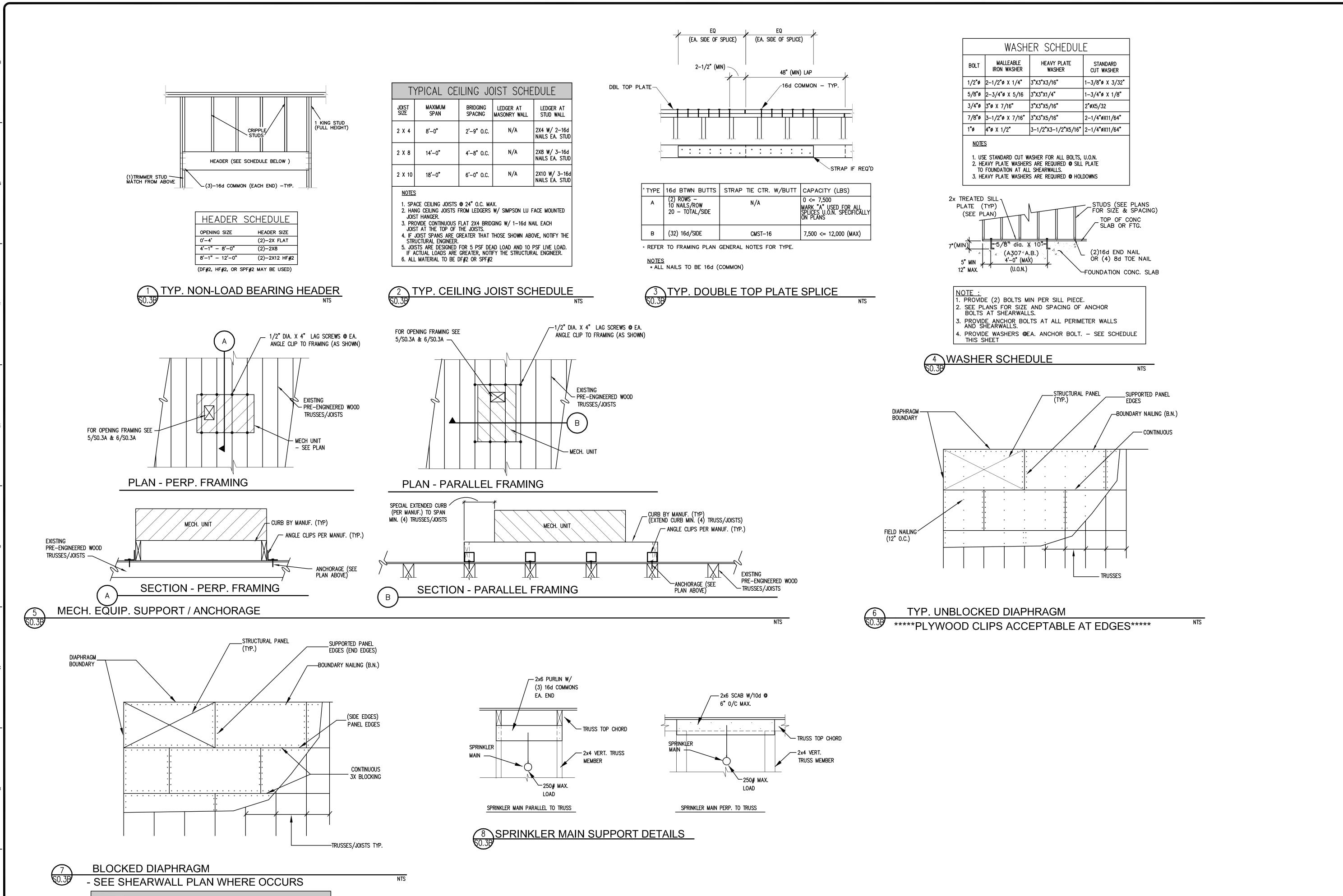
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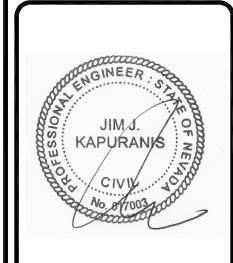
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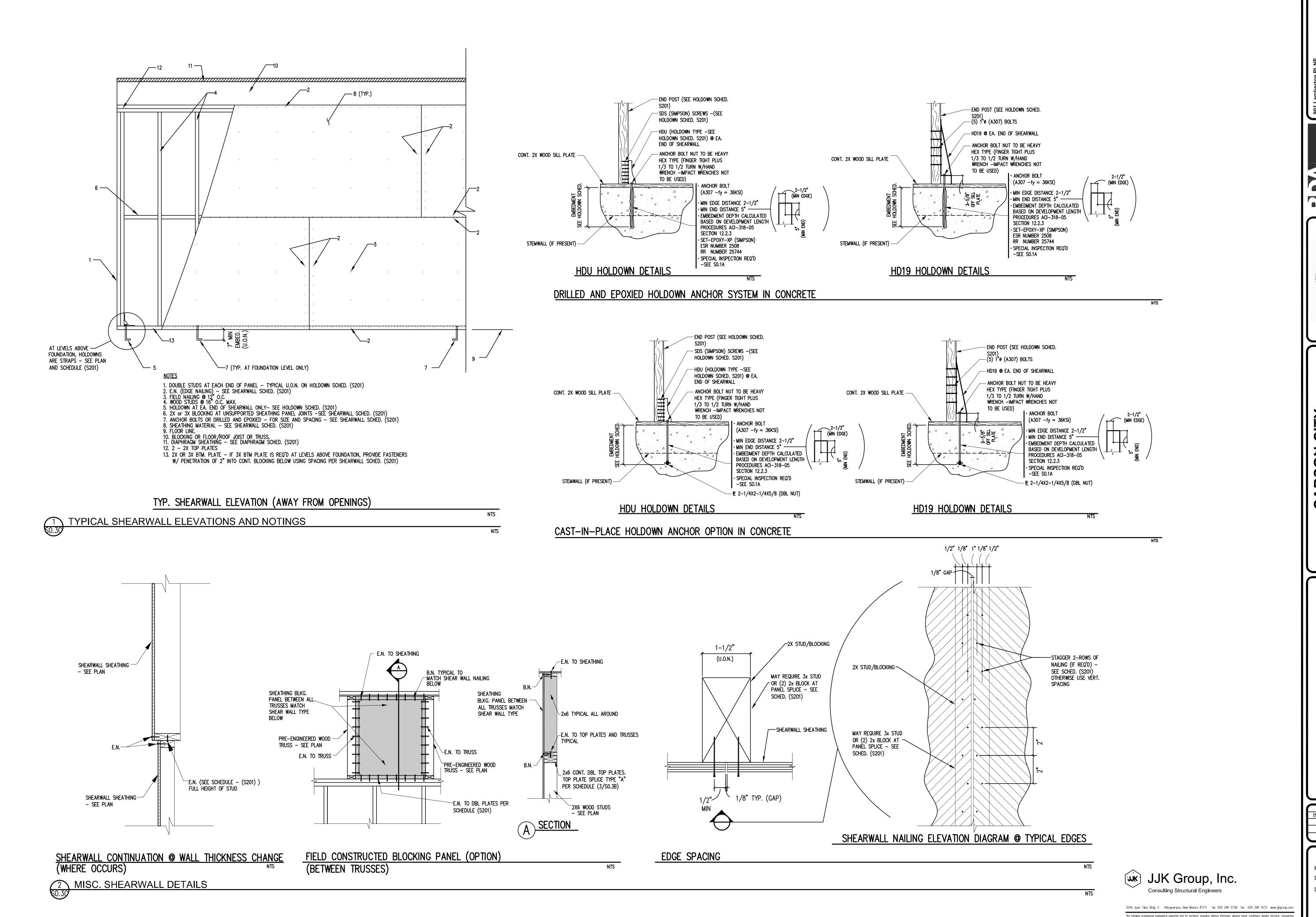
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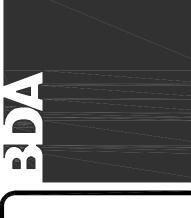
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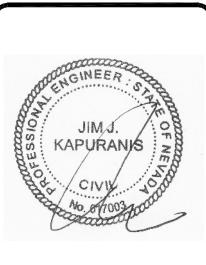
T&G OR PLYWOOD CLIPS ARE NOT A

SUBSTITUTION FOR BLOCKED DIAPHRAGM.



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WOOD SHEARWALL
GENERAL NOTES

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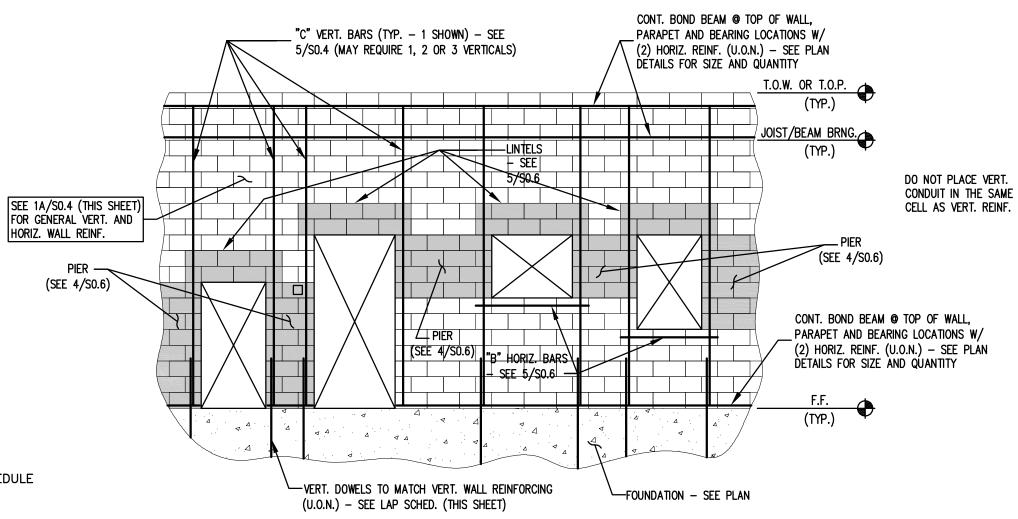
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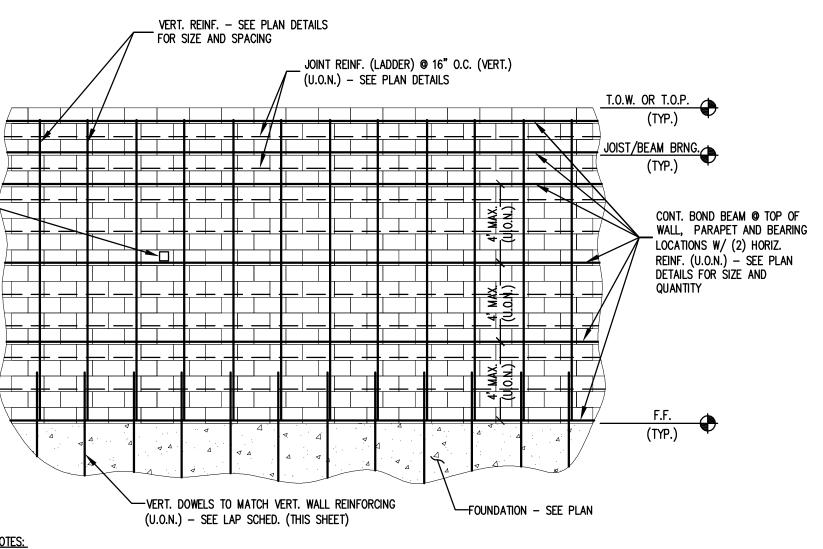
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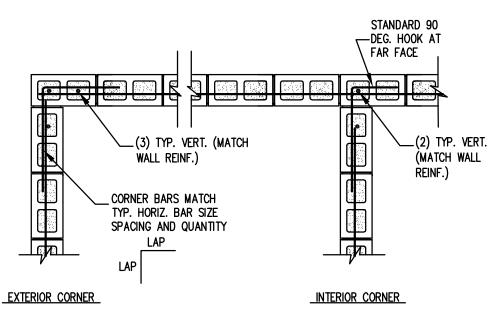
- CONCRETE MASONRY UNITS SHALL BE HOLLOW, SUITABLE FOR BEARING WALL CONSTRUCTION. ALL BLOCKS SHALL CONFORM TO GRADE "N" UNITS GIVEN IN ASTM C90 LATEST EDITION. AND IN ADDITION SHALL HAVE A LINEAR SHRINKAGE OF .065% MAXIMUM FROM SATURATED TO THE OVEN DRY CONDITION. MASONRY UNITS SHALL HAVE CURED FOR NOT LESS THAN (28) DAYS WHEN PLACED IN THE STRUCTURE. PROVIDE ALL BOND BEAM UNITS, LINTELS, ETC., AS NOTED ON PLANS.
- DO NOT USE CHIPPED OR CRACKED BLOCKS. IF ANY SUCH BLOCKS ARE DISCOVERED IN ANY FINISHING WALL, THEY SHALL BE PROMPTLY REMOVED AND REPLACED WITH NEW BLOCKS TO THE APPROVAL OF THE STRUCTURAL ENGINEER.
- THE USE OF ADMIXTURES SHALL NOT BE PERMITTED IN MORTAR OR GROUT UNLESS SUSTAINING DATA HAS BEEN SUBMITTED TO AND REVIEWED BY THE ENGINEER. THE USE OF ADMIXTURES IN MORTAR SHALL NOT BE PERMITTED WITHOUT REDUCING THE LIME CONTENT. THE USE OF UNCONTROLLED FINE CLAY, DIRT AND OTHER DELETERIOUS MATERIALS IS PROHIBITED.
- AGGREGATES, SANDS FOR MORTAR SHALL CONFORM TO ASTM C144 EXCEPT THAT NOT LESS THAN 3% OF THE SAND SHALL PASS THE NUMBER 100 SIEVE. SAND AND PEA GRAVEL FOR GROUT SHALL CONFORM TO ASTM C404, TABLE 1, COURSE AGGREGATE, EXCEPT WHEN OTHER GRADINGS ARE SPECIFICALLY APPROVED BY THE ENGINEER.
- QUICKLIME SHALL CONFORM TO ASTM C5. • MASONRY REBAR LAP LENGTHS SHALL BE PER LAP SCHEDULE
- UNLESS NOTED OTHERWISE ON THESE PLANS. • FOR PROPER MIXING PLACE THE SAND, CEMENT AND WATER IN THE MIXER IN THAT ORDER FOR EACH BATCH OF MORTAR OR GROUT AND MIX FOR A PERIOD OF AT LEAST (2) MINUTES. ADD THE LIME AND CONTINUE MIXING FOR AS LONG AS NEEDED TO SECURE A UNIFORM MASS BUT NOT IN NO CASE LESS THAN (10) MINUTES. USE MIXERS TO SECURE A UNIFORM CAPACITY. BATCHES REQUIRING FRACTIONAL SACKS WILL NOT BE PERMITTED UNLESS CEMENT IS WEIGHED FOR EACH SUCH BATCH. RETEMPER MORTAR ONLY BY ADDING WATER INTO A BATCH MADE WITH THE MORTAR AND THEN CAREFULLY WORKING THE WATER INTO THE MORTAR. RETEMPERING THE MORTAR BY DASHING WATER OVER THE MORTAR SHALL NOT BE PERMITTED. ANY MORTAR OR GROUT WHICH IS UNUSED WITHIN (1) HOUR OF THE INITIAL MIXING SHALL BE REMOVED FROM THE WORK. MORTAR SHALL BE MIXED AND MAINTAINED ON THE BOARDS TO A SLUMP OF (2-5/4") TO PLUS OR MINUS (1/4").
- WATER USED FOR MORTAR AND GROUT SHALL BE CLEAN AND FREE FROM DELETERIOUS AMOUNTS OF ACID, SALTS, ALKALI AND ORGANIC MATERIALS.
- WHEN GROUTING IS STOPPED FOR A PERIOD OF (1) HOUR OR LONGER, FORM HORIZONTAL CONSTRUCTION JOINTS BY STOPPING THE GROUT POUR (1-1/2")MINIMUM BELOW THE UPPER MOST UNIT.
- ALL MASONRY SHALL BE BUILT TO PRESERVE THE UNOBSTRUCTED VERTICAL CONTINUITY OF THE CELLS TO BE FILLED. THE VERTICAL ALIGNMENT SHALL BE SUFFICIENT TO MAINTAIN A CLEAR, UNOBSTRUCTED VERTICAL FLUE MEASURING NOT LESS THAN (3) INCHES, EXCEPT WHERE OPEN END UNITS ARE USED.
- REMOVE CONCRETE SCUM AND GROUT STAINS ON THE WALL IMMEDIATELY. AFTER THE WALL IS CONSTRUCTED, DO NOT SATURATE WITH WATER FOR CURING OR ANY OTHER PURPOSE. CHECK ALL JOINTS FOR TIGHTNESS AND, WHERE CRACKS ARE VISIBLE, CHIP OUT THE MORTAR, TUCK POINT AND TOOL TO MATCH ADJACENT JOINTING.
- GROUT FILL FOR CELLS SHALL CONSIST OF ONE PART PORTLAND CEMENT TO NOT MORE THAN (3) PARTS SAND, TO (2) PARTS PEA GRAVEL. (3/8") MAX. SIZE COURSE AGGREGATE. GROUT FILL USING COURSER AGGREGATE MAY BE USED IF THE MIX IS PROPERLY DESIGNED AND APPROVED BY THE ENGINEER. THE MAXIMUM SIZE OF AGGREGATE USED SHALL NOT EXCEED (1/3) THE LEAST LATERAL DIMENSION OF THE CELL TO BE FILLED. GROUT SHALL DEVELOP A MINIMUM COMPRESSIVE STRENGTH OF 2000 PSI AT (28) DAYS.
- ASSUMED COMPRESSIVE STRENGTH F'm SHALL BE 1500 PSI UNLESS OTHERWISE NOTED ON THESE PLANS. ULTIMATE COMPRESSIVE STRENGTH BASED ON THE AVERAGE OF (3) UNITS SHALL BE NOT LESS THAN 2000 PSI.
- SPECIAL INSPECTION SHALL NOT BE REQUIRED UNLESS SPECIFICALLY NOTED ON THESE PLANS.
- MASONRY LINTELS SHALL BE SOLID GROUTED FOR THE REQUIRED DEPTH. HORIZONTAL REINFORCING SHALL EXTEND (24") BEYOND THE OPENING ON EACH SIDE.
- MAXIMUM HEIGHT OF ANY GROUT POUR SHALL NOT BE GREATER THAN (4') UNLESS PROPER HIGH-LIFT METHODS ARE USED.
- MORTAR SHALL CONFORM TO ASTM C1329, TYPE S WITH A COMPRESSIVE STRENGTH OF 1800 (MIN SLUMP OF 9") PSI AT 28 DAYS UNLESS NOTED OTHERWISE. TYPE M WITH MINIMUM COMPRESSIVE STRENGTH OF 2,500 PSI AT 28 DAYS SHALL BE USED WHERE MASONRY IS BELOW GRADE OR IN CONTACT WITH EARTH. THE MIX SHALL BE REVIEWED BY THE ENGINEER WHEN SPECIAL INSPECTION IS REQUIRED.
- PLACE MORTAR IN HORIZONTAL JOINTS, COMPLETELY COVER THE FACE SHELLS OF THE UNITS WITH MORTAR. SOLID FILL ALL HEAD JOINTS. LAY ALL MASONRY WITH COMMON OR RUNNING BOND. HOLD RAKING TO A MINIMUM.
- CEMENT SHALL BE PORTLAND CEMENT CONFORMING TO ASTM C150, TYPE I OR TYPE II AND SHOULD BE ENTIRELY OF ONE MANUFACTURER.
- BOND BEAM HORIZONTAL REINFORCEMENT SHALL BE SOLIDLY ENCASED IN GROUT. WIRE MESH SHALL BE USED IN EACH CELL BELOW EACH BOND BEAM TO PREVENT THE FLOW OF GROUT INTO UNGROUTED CELLS.
- GROUT ALL CELLS CONTAINING VERTICAL REINFORCEMENT, ANCHOR BOLTS OR EMBEDDED ITEMS. PROVIDE (2") MINIMUM COVER TO EMBEDDED ITEMS.
- ALL VERTICAL WALL REINFOREMENT SHALL HAVE DOWELS EQUAL IN SIZE EMBEDDED INTO FOOTING UNLESS NOTED OTHERWISE IN THESE PLANS.
- REINFORCING COVER SHALL BE (2") MINIMUM THROUGHOUT. POSITIONING DEVICES SHALL BE USED TO INSURE THE CORRECT PLACEMENT OF THE REINFORCEMENT.



1. LAP VERTICAL REINFORCING WITH WALL DOWELS AND ALL OTHER VERT. BARS PER 6/S0.6 (SCHED.). 2. STAGGER SPLICES IN ADJACENT HORIZONTAL BARS IN THE SAME COURSE BY 24". 3. PROVIDE DOWEL BARS IN FOUNDATION TO MATCH ALL VERTICAL REINFORCING (U.O.N.). 4. FOR ADDED REINFORCING AT WALL INTERSECTIONS AND CORNERS, SEE DETAIL 2/SO.6. 5. GROUT EACH SIDE OF OPENING AS NOTED - SEE TYPICAL OPENING REINFORCING DETAIL 5/SO.6. REQUIRED MASONRY WALL REINFORCEMENT AT OPENINGS



1. LAP VERTICAL REINFORCING WITH WALL DOWELS AND ALL OTHER VERT. BARS PER 6/S0.6 (SCHED.). 2. STAGGER SPLICES IN ADJACENT HORIZONTAL BARS IN THE SAME COURSE BY 24". 3. PROVIDE DOWEL BARS IN FOUNDATION TO MATCH ALL VERTICAL REINFORCING (U.O.N.). 4. FOR ADDED REINFORCING AT WALL INTERSECTIONS AND CORNERS, SEE DETAIL 2/SO.6.



1. LAP = 48 BAR DIAMETERS OR 24 MINIMUM (U.O.N.). 2. PROVIDE A STANDARD 90 DEG. HOOK EACH END ON ANY HORIZONTAL BAR BETWEEN OPENINGS. CONTROL JOINTS. OR CORNER BARS LESS THAN 6'-0" IN LENGTH. 3. PROVIDE CORNER BARS FOR ALL BOND BEAM AND LINTEL REINFORCING.

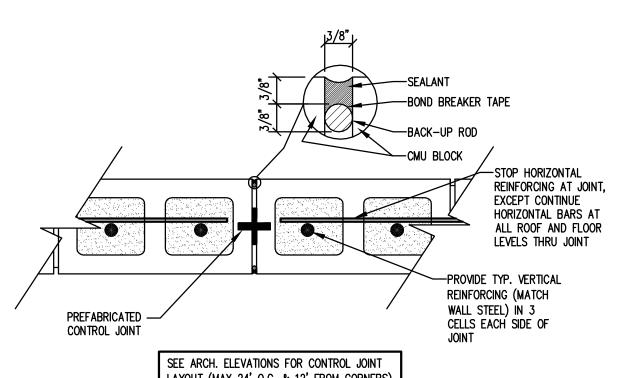
2 CMU WALL CORNERS

_ TYP. CMU WALL

(SEE NOTE 4)

VERTICAL REINFORCING

TAN TYPICAL MASONRY WALL REINFORCEMENT AWAY FROM OPENINGS



LAYOUT (MAX 24' O.C. & 12' FROM CORNERS)

TYPICAL CMU CONTROL JOINT

L_sVALUES (IN)

SIZE 1000 1500 2000 2500 3000 3500

L_a VALUES (IN)

SIZE 1000 1500 2000 2500 3000 3500

25 | 20 | 17 | 16 | 14 | 13

31 | 25 | 22 | 20 | 18 | 16

 73
 59
 51
 46
 42
 39

 112
 91
 79
 71
 64
 60

25 | 24 | 24 | 24 | 24 | 24

36 | 36 | 36 | 36 | 36

54 | 54 | 54 | 54 | 54 | 54

63 63 63 63 63

45 | 45 | 45 | 45 | 45 | 45

• TENSION & COMPRESSION SPLICE

• TENSION & COMPRESSION DEVELOPMENT

• ANCHORAGE OF FLEXURAL REINFORCEMENT

L_{ondhor} VALUES (IN)

SIZE 1000 1500 2000 2500 3000 3500

WHERE h (SEE ELEV. THIS SHEET)

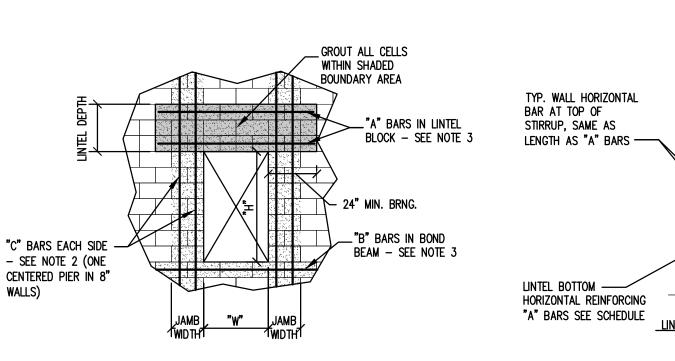
db (REINF. BAR DIA.)

 $-L_{onchor} = MAXIMUM VALUE OF (h OR 12db) -$

INTERIOR PLAN "C" VERT. BARS PER 5/S0.6 — - TYP. EA. END OF SHEAR PIER ∕—#4 PIER HORIZ. SHEAR REINF. FULL HEIGHT OF PIER @ 16" O.C. (U.O.N.) (PIER LENGTH > 48", HORIZ. SHEAR REINF. NOT "C" VERT. BARS PER 5/SOREQ'D) - TYP. EA. END OF SHEAR PIER — - SEE 2/S0.6 (SIM.) PIER LENGTH (48" MAX.) CORNER PLAN NOTES: 1. GROUT PIERS FULL WALL HEIGHT. 2. WHERE PIERS ARE CENTERED BELOW BEAM BEARING, REDUCE TIE SPACING TO 8".

PIER LENGTH (48" MAX.)

4 CMU WALL PIER BETWEEN OPENINGS NTS TYPICAL VERTICAL REINF. NOT SHOWN - SEE 1A/S0.6



CMU OPENING REINF. SCHEDULE										
8" WALLS										
W LINTEL "A" "B" "C" JAMB DEPTH BARS BARS WDTH										
< 2'-8"	8"	(1) #5	(1)	(1)	8"					
2'-8" \le 4'-0"	16"	(2) #5	(1)	(1)	8"					
4'-0" \le 6'-0"	24"	(2) #6	(1)	(2)	16"					
6'-0" \le 8'-0"	32"	(2) #6	(2)	(3)	24"					
8'-0" \le 10'-0"	48"	(2) #6	(2)	(3)	24"					
UP TO 13'-4"	48"	(2) #6	(2)	(3)	24"					

1. USE BAR QUANTITIES AND SIZES GIVEN IN LINTEL SCHEDULE UNLESS OTHERWISE NOTED ON THE DRAWINGS. 2. EXTEND "C" BARS 48 BAR DIAMETERS OR 24" MINIMUM BEYOND TOP AND BOTTOM OF OPENING EXCEPT THAT WHEN "H" OR "W" EXCEEDS 24". "C" BARS SHALL EXTEND FULL HEIGHT. WHERE THERE IS LESS THAN 8" BETWEEN ADJACENT OPENINGS, EXTEND REINFORCING CONTINUOUS TO 32" BEYOND FURTHEST OPENING. 3. "A" AND "B" BARS SHALL EXTEND 48 BAR DIAMETERS OR 24" MINIMUM EACH SIDE OF THE OPENINGS.

4. FOR BAR SIZES, MATCH TYPICAL WALL REINFORCING AS SHOWN ON THE BUILDING WALL SECTIONS, U.O.N.

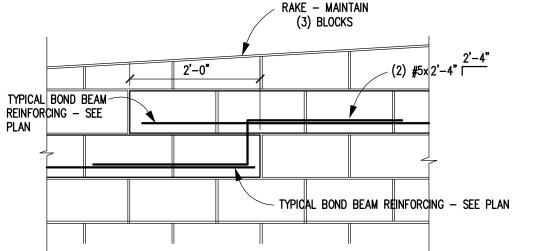
√5 CMU OPENING REINFORCING

SONR TES A M N O BDA DSGN. REV.

JJK Group, Inc.

3240 Juan Tabo Blda, C Albuquerque, New Mexico 87111 tel. 505 296 5706 fax 505 296 1672 www.jkgroup.com following professional engineering submittal and its contents, including, without limitation, general notes, conditions, details, sections, calculati

(6) REINFORCING DEVELOPMENT LENGTHS / SPLICES (WHERE APPLICABLE)



7 BOND BEAM STEP DETAIL

TENSION DÉVELOPMEN

COMPRESSIÓN DEVELOPMENT

COLUMN/PILASTER

LINTEL & JAMB REINFORCEMENT

RETAINING WALL

Consulting Structural Engineers

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PROJECT NO.: 1107 DRAWN: DATE 2/10/15

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STRUCTURAL STEEL

- STRUCTURAL STEEL SHALL BE SHOP FABRICATED IN ACCORDANCE WITH AISC "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES" LATEST EDITION, WELDING SHALL COMPLY WITH AWS 1.1 "STRUCTURAL STEEL WELDING".
- CONTRACTOR TO VERIFY ALL MEASUREMENTS AT JOB SITE.
- PROVIDE ALL LUGS, CLIPS, ANGLES AND MISCELLANEOUS FASTENERS
- NECESSARY FOR THE COMPLETE ASSEMBLY AND INSTALLATION. • DO ALL GROUTING OF BASE PLATES AND SIMILAR ITEMS WITH
- PROTECT ALL DISSIMILAR METALS FROM GALVANIC CORROSION.
- PROVIDE WASHERS ON ALL HEADS AND NUTS BEARING ON WOOD. DRAW ALL NUTS TIGHT AND UPSET THREADS OF PERMANENT CONNECTIONS TO PREVENT LOOSENING. USE BEVELED WASHERS WHERE BEARING IS ON SLOPED SURFACES.
- THOROUGHLY CLEAN ALL MILL SCALE, RUST, DIRT, GREASE AND OTHER FOREIGN MATTER FROM FERROUS METAL PRIOR TO PAINTING.
- AFTER MATERIAL HAS BEEN PROPERLY CLEANED AND TREATED, APPLY PRIME COAT OF PAINT TO ALL SURFACES EXCEPT THOSE ENCASED IN CONCRETE OR MASONRY. APPLY ALL PAINT AS PER MANUFACTURER'S DIRECTIONS. SPOT PAINT ALL ABRASIONS AND FIELD CONNECTIONS AFTER ASSEMBLY. SHOP COAT SHALL BE DRY PRIOR TO SHIPMENT TO JOB SITE.
- REFER TO SHEET SO.1 FOR ALL "ER" REQUIRED REPORTS.
- WELDING FOR STRUCTURAL STEEL SHALL BE IN ACCORDANCE WITH AWS CODE P1.1. WELDS SHALL BE MADE ONLY BY OPERATORS EXPERIENCED IN PERFORMING THE TYPE OF WORK INDICATED. WELDS NORMALLY EXPOSED TO VIEW IN THE FINISHED WORK SHALL BE UNIFORMLY MADE AND GROUND SMOOTH. WHERE WELDING IS DONE IN PROXIMITY TO GLASS OR FINISHED SURFACES, SUCH SURFACES SHALL BE PROTECTED FROM DAMAGE DUE TO WELD SPARKS, SPATTER OR TRAMP METAL
- INDIVIDUAL WELDERS FOR STRUCTURAL METAL WORK SHALL BE QUALIFIED FOR THE WELDS BEING PERFORMED.
- WELDING ELECTRODES TO BE E70 SERIES UNLESS OTHERWISE NOTED.
- ALL STRUCTURAL MISCELLANEOUS STEEL SHALL BE FABRICATED AND ERECTED IN ACCORDANCE WITH THE A.I.S.C SPECIFICATIONS FOR THE DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS, LATEST EDITION.
- ASTM MATERIAL REQUIREMENTS:

NON-SHRINK GROUT.

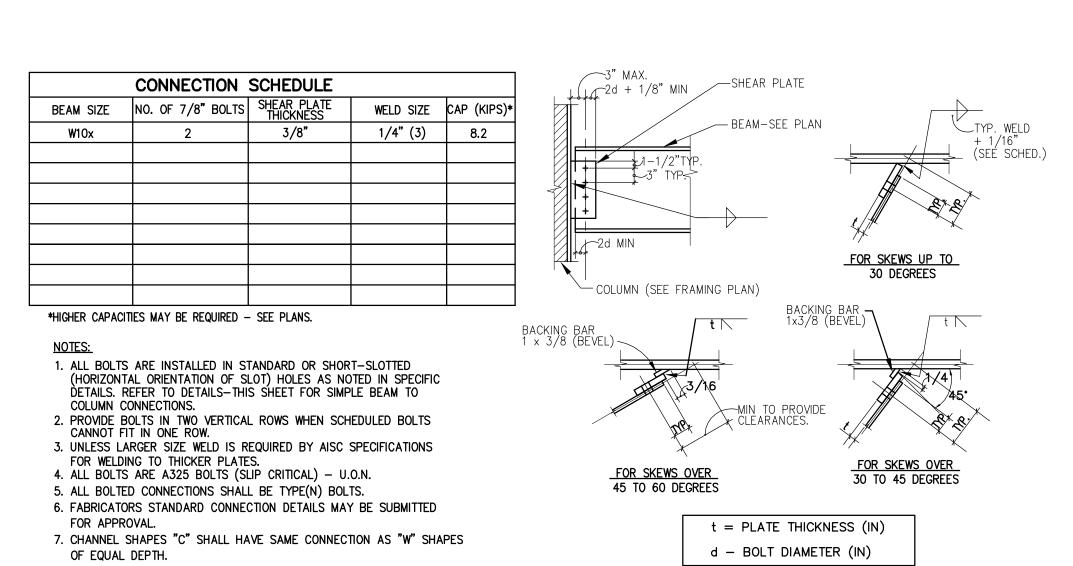
- ASTM A-992 GRADE 50 FOR ALL WIDE FLANGE W SECTIONS AND T SECTIONS
- ASTM A−36 FOR C, MC, BARS, ANGLES, AND PLATES.
- → ASTM A-53 GRADE B OR A-501 FOR STEEL PIPES - ASTM A-500 GRADE B, FY=46 KSI FOR TS/HSS TUBE STEEL FOR
- SIZES UP TO 5/8" THICK. - ASTM A-501 GRADE B, FY=36 KSI FOR TS/HSS TUBE STEEL FOR
- OTHERS UP TO 1" THICK.
- ASTM A−307 PLAIN BOLTS.
- ASTM F-1554 GRADE 36, A-307 OR A-36 PLAIN ANCHOR BOLTS. - ASTM A-325-N HIGH STRENGTH BOLTS; TESTED BY A CALIBRATED WRENCH UNLESS LOAD INDICATOR BOLTS ARE USED. HIGH STRENGTH BOLTS SHALL BE TIGHTENED TO "SNUG TIGHT" CONDITION PER AISC, UNLESS SPECIFICALLY CALLED OUT TO BE "TORQUED AND TESTED" ON PLANS BY A CALIBRATED TORQUE WRENCH.
- CLEARLY IDENTIFY EACH PIECE OF STRUCTURAL STEEL WITH A PIECE MARK AND THE CORRESPONDING STRUCTURAL ASTM GRADE AND YIELD STRESS.
- SEE MECHANICAL DRAWINGS FOR MECHANICAL EQUIPMENT SUPPORT FRAMING AND SPREADERS. SEE MECHANICAL & ARCHITECTURAL DRAWINGS FOR EXACT QUANTITY, SIZE, LOCATIONS, AND WEIGHT OF MECHANICAL UNITS.

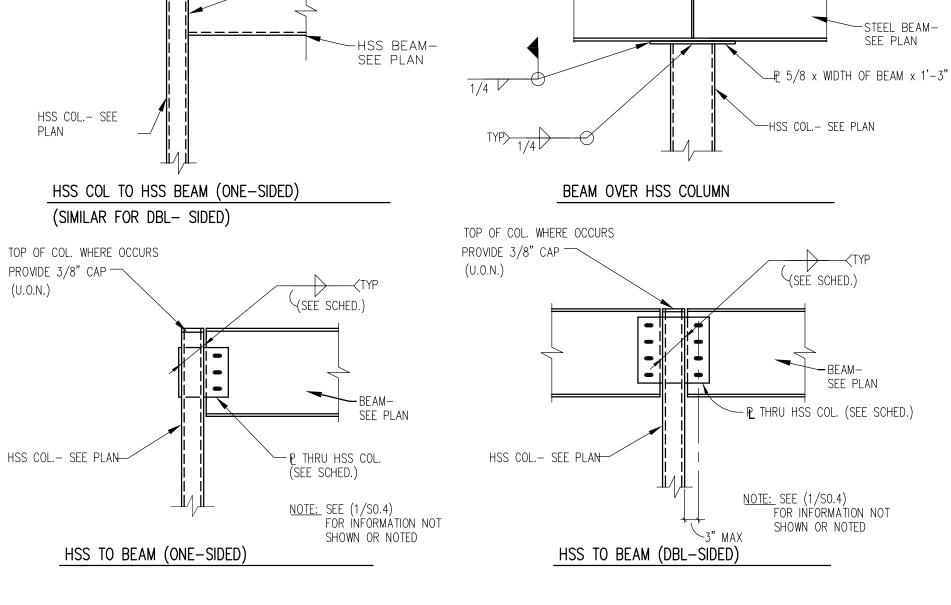
STRUCTURAL METAL DECK

- THE MANUFACTURING, DETAILING, AND ERECTION OF METAL DECKS SHALL BE IN ACCORDANCE WITH THE "STEEL DECK INSTITUTE SPECIFICATIONS," LATEST EDITION.
- ALL METAL DECK SHALL BE FABRICATED FROM STEEL HAVING A MINIMUM YIELD STRENGTH OF 33,000 PSI.
- REFER TO PLANS AND DETAILS FOR DECK SIZE.
- •PROVIDE DETAILED AND CHECKED SHOPE DRAWINGS INDICATING LOCATION, GAGE, AND SIZE OF EACH PIECE OF DECKING AND RELATED DECKING ACCESSORY. THE DRAWINGS SHALL CLEARLY SHOW WELDING DETAILS TO STRUCTURAL FRAMING ELEMENTS, SIDE LAP CONNECTION DEDTAILS, DECK OPENINGS, EDGE CLOSURES, AND WHERE REQUIRED, SUPPLEMENTARY DECK AND/OR CLOSURE REINFORCING.
- ALL DECKING SHALL BE WELDED TO STRUCTURAL STEEL BY QUALIFIED WELDERS USING PRE-QUALIFIED PROCEDURES. THE TECHNICAL SPECIFICATIONS ESTABLISH A PROCEDURE FOR PRE QUALIFICATION OF THE PLUG WELDING OF THE STEEL DECKING TO THE STRUCTURAL STEEL FOR THE PARTICULAR GAGES USED. PRIOR TO THE START OF ERECTION OF THE STEEL DECK, EACH WELDER SHALL BE QUALIFIED USING THIS PROCEDURE AS WITNESSED BY THE OWNER'S STRUCTURAL STEEL TESTING LABORATORY.
- THE METAL DECK SHALL BE DESIGNED TO BE UNSHORED AND CONTINUOUS OVER A MINIMUM OF THREE (3) SPANS IN THE DIRECTION INDICATED. METAL DECKING FOR SINGLE AND DOUBLE SPANS, IF REQUIRED, SHALL ALSO SATISFY THE SPECIFIED LOAD AND DEFLECTION REQUIREMENTS.
- THE DECK SHALL BE DESIGNED FOR AN ASSUMED SUITABLE CONSTRUCTION LIVE LOAD. THE ASSUMED CONSTRUCTION LIVE LOAD SHALL NOT BE LESS THAN 20 PSF. HOWEVER, CONTRACTOR SHALL FOLLOW ALL APPLICABLE CITY, LOCAL, STEEL DECK INSTITUTE, AND AISI REQUIREMENTS FOR TEMPORARY CONSTRUCTION LOADINGS.
- DO NOT HANG LOADS FROM METAL DECK. HANG ALL DUCTWORK, PIPING, ETC. DIRECTLY FROM STRUCTURAL STEEL FRAMING OR SUPPLEMENTARY MEMBERS. ALL HANGING LOAD DETAILS SHALL BE SUBMITTED FOR REVIEW. LATERAL ROOF LOADS SHALL BE TRANSFERREED THROUGH DIAPHRAGM ACTION PROVIDED BY THE METAL ROOF DECK.
- PROVIDE CONTINUOUS SHEET METAL CLOSURES AT ALL SLAB OPENINGS AND SLAB EDGES AND CONTINUOUS DECK CLOSURES AT ALL DECK ENDS.
- ALL METAL DECK SHALL BE WELDED TO THE SUPPORTING STEEL PER SCHEDULE (THIS SHEET). PUDDLE WELDS AT 12 INCHES ON CENTER (6" O.C. AT PERIMETER). THE USE OF WELDING WASHERS FOR ALL METAL DECK WELDING SHALL BE IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS. PROVIDE WELDED SIDE LAP FASTENERS AT 36" MAXIMUM ON CENTER OR #10 TEK SCREW SIDE LAP FASTENERS AT A MINIMUM OF 2 PER SPAN, UNLESS OTHERWISE NOTED. REFER TO ATTACHMENT PATTERN SCHEDULE THIS SHEET.
- •PROVIDE, AS REQUIRED, ALL RIDGE AND VALLEY PLATES, COLUMN CLOSURES, CANT STRIPS, SUMP PLATES AT PIPING PENETRATIONS, AND RECESSED SUMP PANS AT ROOF DRAINS. PROVIDE SUPPLEMENTAL FRAMING, INCLUDING REINFORCING PLATES, AT OPENINGS AS REQUIRED FOR SUPPORT OF THE METAL DECK. COORDINATE ALL OPENINGS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS.

COMPOSITE METAL DECK

- ALL SLAB COMPOSITE METAL DECK SHALL HAVE WIDE RIBS SUITABLE FOR SHEAR STUD PLACEMENT WHERE STUDS ARE REQUIRED. THE CONFIGURATION OF THEMETAL DECK SHALL BE SUCH AS TO DEVELOP THE FULL SHEAR VALUE OF THE STUD FOR THE PARTICULAR WEIGHTS OF THE CONCRETE AS LISTED IN THE AISC SPECIFICATIONS, LATEST EDITION. THE CONTRACTOR SHALL PROVIDE VERIFICATION OF THE STUD VALUES FOR THE SPECIFIC DECK TYPE AND STUD SPACING AND PROVIDE ADDITIONAL STUDS AS REQUIRED FOR SPECIFIC DECK TYPES UTILIZED.
- ●PROVIDE 3/4" DIA. x 3-1/2" HEADED STUDS AS DESIGNATED ON STEEL MEMBERS WHICH SUPPORT METAL DECK SLABS. OMIT STUDS WHERE NOT SHOWN ON PLANS.
- THE CONTRACTOR SHALL PROVIDE CHECKED SHOP DRAWINGS INDICATING EXACT LAYOUT OF STUDS FOR EACH BEAM, SPAN, AND DECK LAYOUT.
- •SHEAR STUDS SHALL BE WELDED THROUGH THE METAL DECK BY PRE-QUALIFIED METHODS. IF THROUGH-DECK WELDING IS UNFEASIBLE, THE STUDS SHALL BE INSTALLED IN PRE-PUNCHED HOLES IN THE DECK.



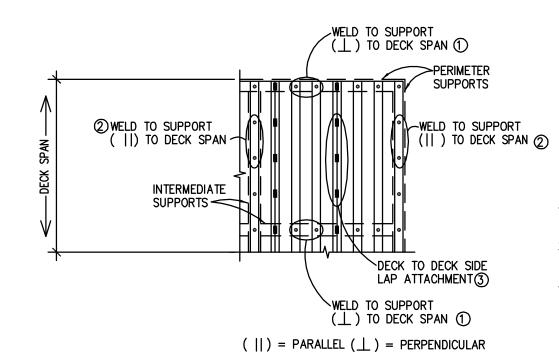


P 1/2" STIFFENER PLATE EA. SIDE-

2 TYPICAL SIMPLE SHEAR CONNECTIONS (WHERE APPLICABLE)

PROVIDE 3/8" CAP

(U.O.N.)



CONNECTION SCHEDULE AND REQUIREMENTS

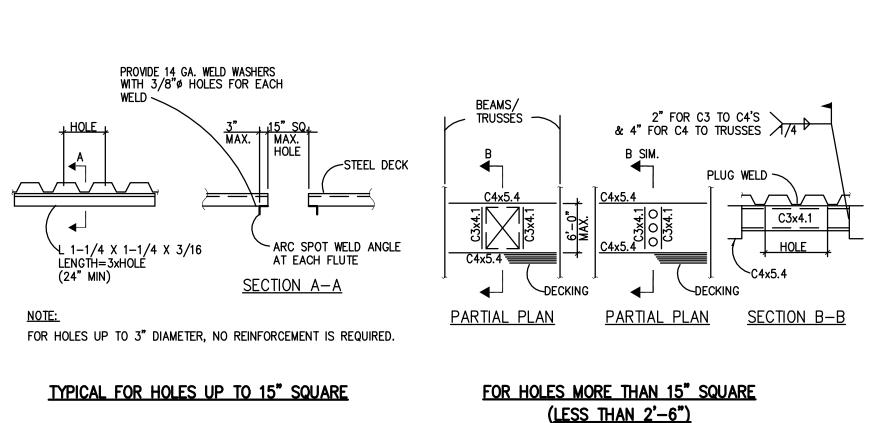
(WHERE APPLICABLE - SEE PLANS)

	ATTACHMENT PATTERN			DIAPHRAGM DECK	DEPTH CAUGE			l mlN	Sx min	Fy	
DECK TYPE	TO SUPPORT PERPENDICULAR TO SUPPORT PARALLEL TO DECK RIBS ①		DECK TO DECK SIDE LAPS ③	SHEAR T	TYPE	(INCHES)	GAUGE	CONFIGURATION	(IN ⁴ / FT)		(KSI)
3N	5/8"PUDDLE WELD AT ALL FLUTES	5/8" PUDDLE WELD @ 10" O.C.	(5) FASTENERS PER SPAN #10 TEK	471 LB/FT ON 6'-0" SPAN	В	1-1/2"	18	1-1/2 1	.292	.327	33
1.5B	5/8"PUDDLE WELD AT ALL FLUTES	(7) FASTENERS PER SPAN 455 L		455 LB/FT ON 6'-0" SPAN				36" + 1 34			

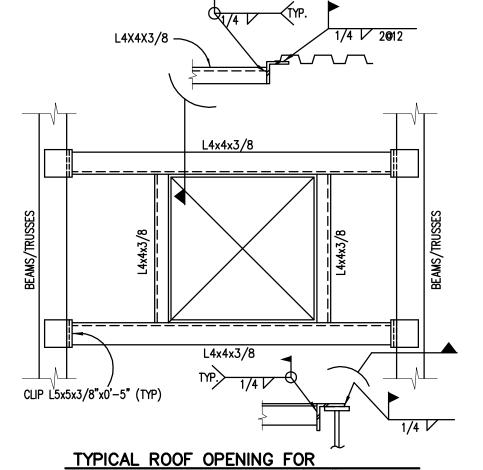
1. DIAPHRAGM SHEAR IS IBC ALLOWABLE DIAPHRAGM SHEAR FOR DECK INSTALLED IN ACCORDANCE WITH CONTRACT DOCUMENTS.

2. THE DETAILS SHOWN ON THIS SHEET SHALL BE INCORPORATED INTO THE PROJECT AT ALL LOCATIONS WHERE METAL DECK IS USED, WHETHER SPECIFICALLY CALLED OUT OR NOT.

3. DECK SHALL BE WELDED TO ALL BEAMS AND SUPPORTS.



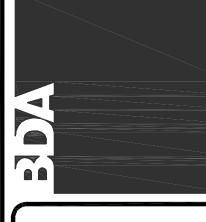
METAL DECK DETAILS

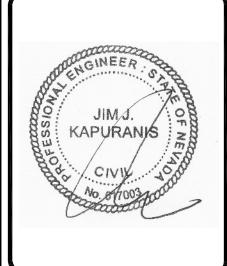


OPENINGS LARGER THAN 2'-6"

JK Group, Inc. Consulting Structural Engineers

3240 Juan Tabo Bldg. C - Albuquerque, New Mexico 87111 - tel. 505-296-5706 - fax - 505-296-1672 - www.jjkgroup.co





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BDA DSGN. REV. BDA TECH REV.

CCAS PROJECT NO.: 1107 DRAWN:

OF

EARTHWORK NOTES

• CRITERIA:

ALL INFORMATION BELOW IS A SUMMARY PROVIDED BY THE GEOTECHNICAL ENGINEER RESOURCE CONCEPTS, INC DRAFT SOILS REPORT BY: JAN 9, 2015

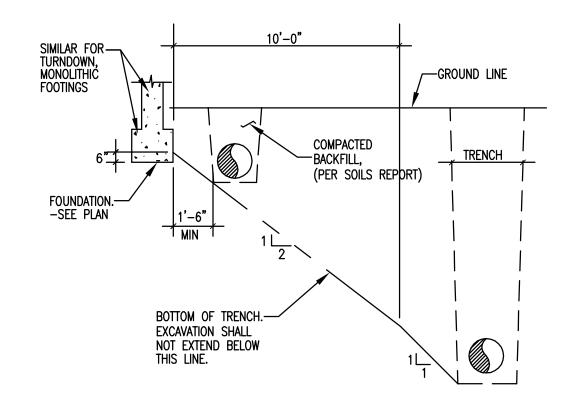
THE FOLLOWING INFORMATION BELOW BY JJK GROUP, INC. DOES NOT RELIEVE THE CONTRACTOR FROM REVIEWING THE SOILS REPORT AND FOLLOWING ALL REQUIRED SUBGRADE PREPARATIONS.

• GENERAL:

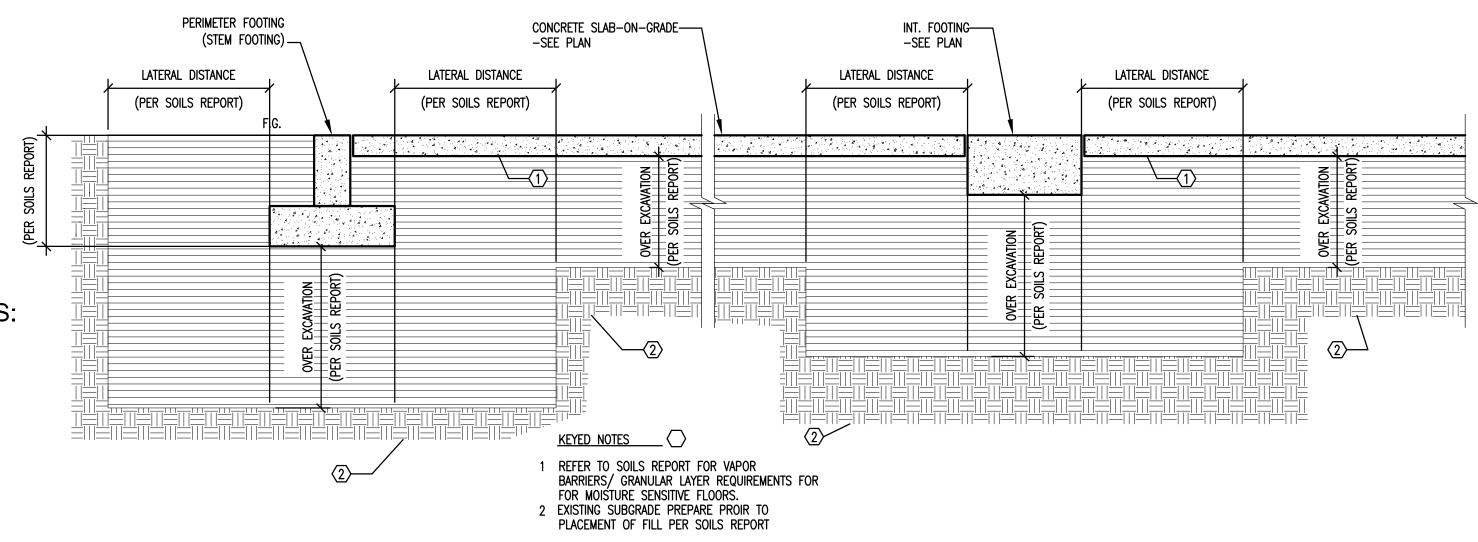
- THE GEOTECHNICAL ENGINEER SHALL ACT AS THE OWNER'S REPRESENTATIVE AND SHALL MAKE OBSERVATIONS AND TESTS AS CONSIDERED NECESSARY FOR QUALITY CONTROL. WHERE FOUNDATIONS OR OTHER CRITICAL ELEMENTS ARE TO BE SUPPORTED ON ENGINEERED FILL, CONTINUOUS OBSERVATIONS AND TESTS OF GRADING OPERATIONS SHALL BE MADE BY THE GEOTECHNICAL ENGINEER. ALL TESTS SHALL BE PERFORMED IN ACCORDANCE WITH PROCEDURES SET FORTH IN THE CURRENT BOOK OF STANDARDS OF THE AMERICAN SOCIETY OF TESTING AND MATERIALS (ASTM).
- FILL OR BACKFILL, CONSISTING OF SOIL APPROVED BY THE GEOTECHNICAL ENGINEER, SHALL BE PLACED IN CONTROLLED COMPACTED LAYERS WITH APPROVED COMPACTION EQUIPMENT. CONTROLLED COMPACTED LAYERS WITH APPROVED COMPACTION EQUIPMENT. ALL COMPACTION SHALL BE TO A MINIMUM OF 95 PERCENT (U.O.N. IN SOILS REPORT) OF THE MAXIMUM DRY DENSITY DETERMINED IN ACCORDANCE WITH ASTM D-1557 A TEST METHOD. SOILS MOISTURE CONTENT DURING COMPACTION SHALL BE AT OPTIMUM MOISTURE CONTENT PLUS OR MINUS 2% (U.O.N. IN SOILS REPORT).
- TESTS FOR DEGREE OF COMPACTION SHALL BE DETERMINED BY THE ASTM D-1556 OR ASTM D-2922 TEST METHODS. OBSERVATION AND FIELD TESTS SHALL BE CARRIED ON DURING FILL AND BACKFILL PLACEMENT BY THE GEOTECHNICAL ENGINEER TO ASSIST THE CONTRACTOR IN OBTAINING THE REQUIRED DEGREE OF COMPACTION. IF LESS THAN 95 (U.O.N. IN SOILS REPORT) PERCENT IS INDICATED, ADDITIONAL COMPACTION EFFORT SHALL BE MADE WITH ADJUSTMENT OF THE MOISTURE CONTENT AS REQUIRED COMPACTION IS OBTAINED.
- WHEREVER, IN THE OPINION OF THE GEOTECHNICAL ENGINEER, AN UNSTABLE CONDITION IS BEING CREATED, EITHER BY CUTTING OR FILLING, THE WORK SHALL NOT PROCEED IN THAT AREA UNTIL AN INVESTIGATION HAS BEEN MADE AND THE GRADING PLAN REVISED.

• GENERAL DRAINAGE, MAINTENANCE AND PRECAUTIONS:

- PRECAUTIONS SHALL BE TAKEN DURING AND AFTER CONSTRUCTION TO MINIMIZE SATURATION OF THE FOUNDATION SOILS. POSITIVE DRAINAGE SHALL BE ESTABLISHED AWAY FROM THE EXTERIOR WALLS OF THE STRUCTURE (FOUR PERCENT GRADIENT WITHIN TEN FEET OF THE STRUCTURE). ALL UTILITY TRENCHES LEADING INTO THE BUILDING SHALL BE BACKFILLED WITH COMPACTED FILL. SPECIAL CARE SHALL BE TAKEN DURING INSTALLATION OF WATER LINES TO REDUCE THE POSSIBILITY OF FUTURE SUBSURFACE SATURATION.
- DEPRESSIONS AND EXCAVATIONS SHOULD BE BACKFILLED WITH COMPACTED, NON-SWELLING, RELATIVELY-IMPERVIOUS SOILS SUCH AS CLAYEY SANDS.
- GUTTERS AND DOWN SPOUTS SHOULD BE INSTALLED TO CONTROL ROOF DRAINAGE. DOWNSPOUTS SHOULD DISCHARGE A MINIMUM OF TEN FEET AWAY FROM STRUCTURES. AREA DRAINS MAY BE INSTALLED AROUND STRUCTURES TO IMPROVE DRAINAGE. DISCHARGE PIPES SHOULD SLOPE A MINIMUM OF 1/8 TH INCH VERTICAL PER FOOT OF HORIZONTAL PIPE. DRAINAGE SEWERS AND DISCHARGE CHANNELS SHOULD BE KEPT FREE
- EXPANSION JOINTS WITHIN EXTERIOR CONCRETE FLATWORK SHOULD BE FILLED WITH A FLEXIBLE JOINT SEALER TO MINIMIZE WATER INFILTRATION.
- SOME MINOR CRACKING OF NEW CONCRETE FOUNDATIONS, CONCRETE FLATWORK, AND INTERIOR DRY WALL IS NORMAL THIS IS A RESULT OF CONCRETE SHRINKAGE AS IT CURES, "SETTLING IN" OF THE NEW STRUCTURE, DRYING OF TIMBERS USED IN CONSTRUCTION, ETC. NORMALLY THE MAJORITY OF THIS MOVEMENT SHOULD CEASE WITHIN THE FIRST YEAR FOLLOWING CONSTRUCTION. HOWEVER, DEPENDING ON THE STRUCTURE AND SITE CONDITIONS, MOVEMENT MAY CONTINUE AT A SLOW RATE FOR SEVERAL YEARS.
- ROOF GUTTERS AND DOWNSPOUTS SHOULD BE UTILIZED. DOWN SPOUTS SHOULD DISCHARGE DOWN SLOPE AND WELL AWAY FROM THE STRUCTURES. SPLASH BLOCKS SHOULD BE UTILIZED BELOW DOWN SPOUTS. SURFACE WATER SHOULD RUN OFF RAPIDLY.
- LANDSCAPING ADJACENT TO STRUCTURES SHOULD BE DESIGNED AND CONSTRUCTED TO MINIMIZE THE POTENTIAL FOR WETTING OF SOILS SUPPORTING THE PROPOSED FACILITIES. IT IS NOT RECOMMENDED THAT LANDSCAPING REQUIRING IRRIGATION BE INSTALLED CLOSER THAN FIVE FEET OF STRUCTURES. TREES AND SHRUBS SHOULD BE HAND WATERED OR WATERED USING CONTROLLED DRIP IRRIGATION. IF DRIP IRRIGATION IS USED, EMITTERS SHOULD DISCHARGE NO MORE THAN ONE GALLON PER HOUR. WATERING SHOULD BE CAREFULLY CONTROLLED TO PREVENT OVER WATERING. GRASSED AREAS ADJACENT TO STRUCTURES SHOULD BE SLOPED SO THAT EXCESS IRRIGATION WATER WILL RUN OFF PROMPTLY. SPRINKLER LINES AND DRIP IRRIGATION MAINS SHOULD BE LOCATED A MINIMUM OF FIVE FEET AWAY FROM FOUNDATIONS. MOWING STRIPS, PLANTERS AND SIDEWALKS SHOULD NOT "DAM" WATER ADJACENT TO STRUCTURES.



GENERAL TRENCH REQUIREMENTS PARALLEL TO FOUNDATION

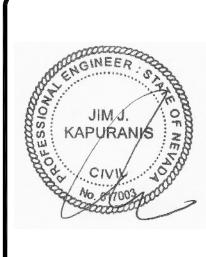


EARTHWORK REQUIREMENTS

THE ABOVE EARTHWORK INFORMATION IS BASED ON THE SOILS REPORT PROVIDED. THIS DOES NOT RELIEVE THE CONTRACTOR FROM THE COMPLETE REVIEW OF THE SOILS REPORT AND ALL

REQUIREMENTS/RECOMMENDATIONS OF SUCH REPORT MUST BE FOLLOWED.





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GENERAL DETAILS ORK ARTHW

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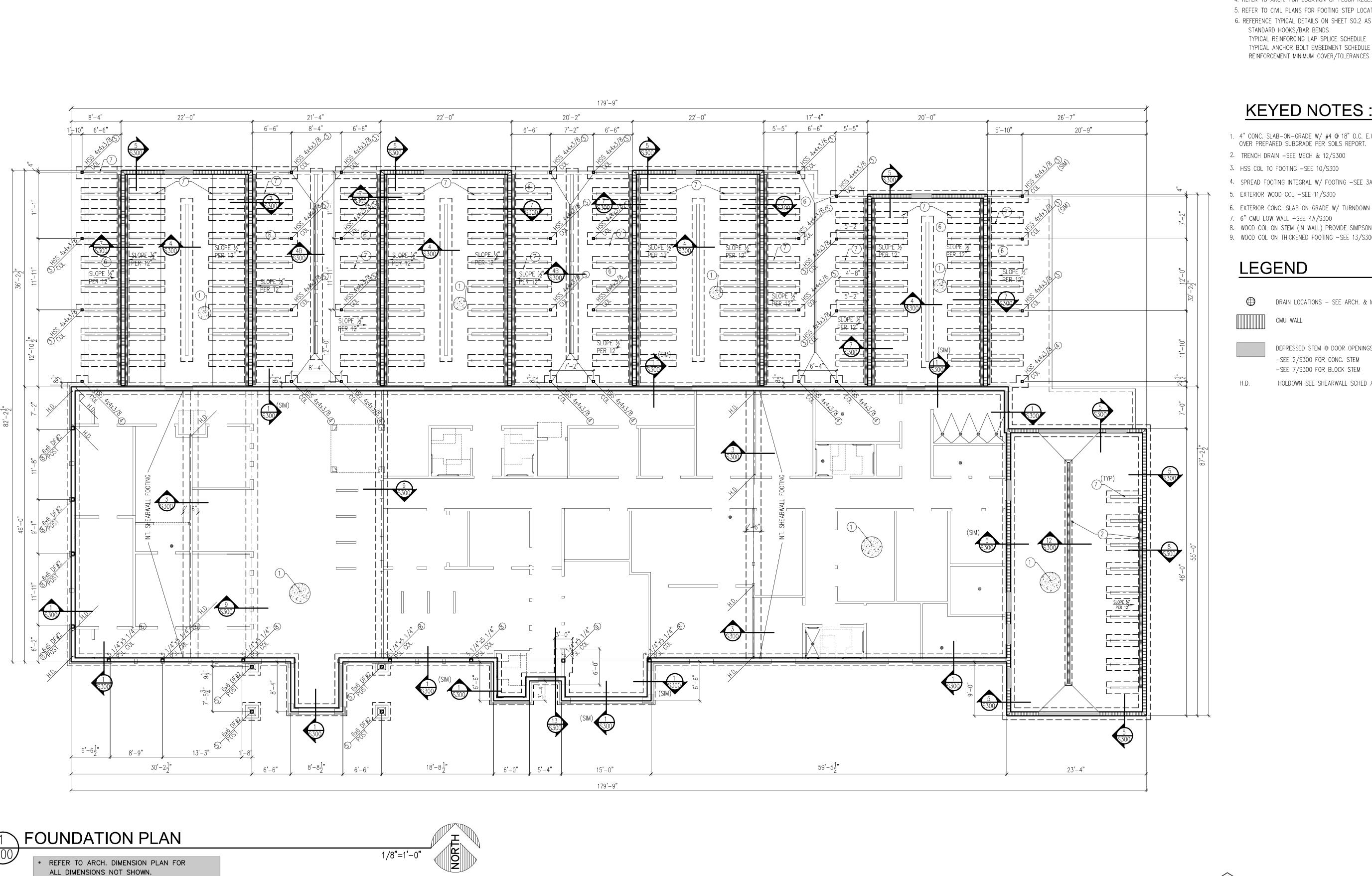
JJK Group, Inc.

Consulting Structural Engineers

3240 Juan Tabo Bldg. C Albuquerque, New Mexico 87111 tel. 505 296 5706 fax 505 296 1672 www.jjkgroup.com

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GENERAL NOTES:

- 1. REFER TO GENERAL NOTES SHEETS FOR ALL REQUIREMENTS.
- 2. AT NON-BEARING PARTITION WALLS PROVIDE 0.145" DIAMETER SHOT-PINS OF SUFFICENT LENGTH FOR 1-3/8" PENETRATION INTO CONCRETE SPACED AT NO MORE THAN 16" O.C. FROM SILL PLATE INTO SLAB AT BASE LEVEL.
- 3. REFER TO MECH./ELEC. FOR ALL INSERTS AND DRAIN LOCATIONS.
- 4. REFER TO ARCH. FOR LOCATION OF FLOOR RECESSES. 5. REFER TO CIVIL PLANS FOR FOOTING STEP LOCATIONS
- 6. REFERENCE TYPICAL DETAILS ON SHEET SO.2 AS FOLLOWS: STANDARD HOOKS/BAR BENDS TYPICAL REINFORCING LAP SPLICE SCHEDULE TYPICAL ANCHOR BOLT EMBEDMENT SCHEDULE

KEYED NOTES:

- 1. 4" CONC. SLAB-ON-GRADE W/ #4 @ 18" O.C. E.W. @ MID-DEPTH OVER PREPARED SUBGRADE PER SOILS REPORT.
- 2. TRENCH DRAIN -SEE MECH & 12/S300
- 3. HSS COL TO FOOTING -SEE 10/S300
- 4. SPREAD FOOTING INTEGRAL W/ FOOTING -SEE 3A/S300
- 5. EXTERIOR WOOD COL -SEE 11/S300
- 6. EXTERIOR CONC. SLAB ON GRADE W/ TURNDOWN -SEE SO.2
- 7. 6" CMU LOW WALL -SEE 4A/S300
- 8. WOOD COL ON STEM (IN WALL) PROVIDE SIMPSON "ABA" POST BASE
- 9. WOOD COL ON THICKENED FOOTING -SEE 13/S300

LEGEND

DRAIN LOCATIONS - SEE ARCH. & MECH.



DEPRESSED STEM @ DOOR OPENINGS -SEE 2/S300 FOR CONC. STEM

-SEE 7/S300 FOR BLOCK STEM HOLDOWN SEE SHEARWALL SCHED AND PLAN S201 SERVICES

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DATE:

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COORDINATE ALL ARCHITECTURAL

DIMENSIONS/SLAB ELEVATIONS PRIOR TO

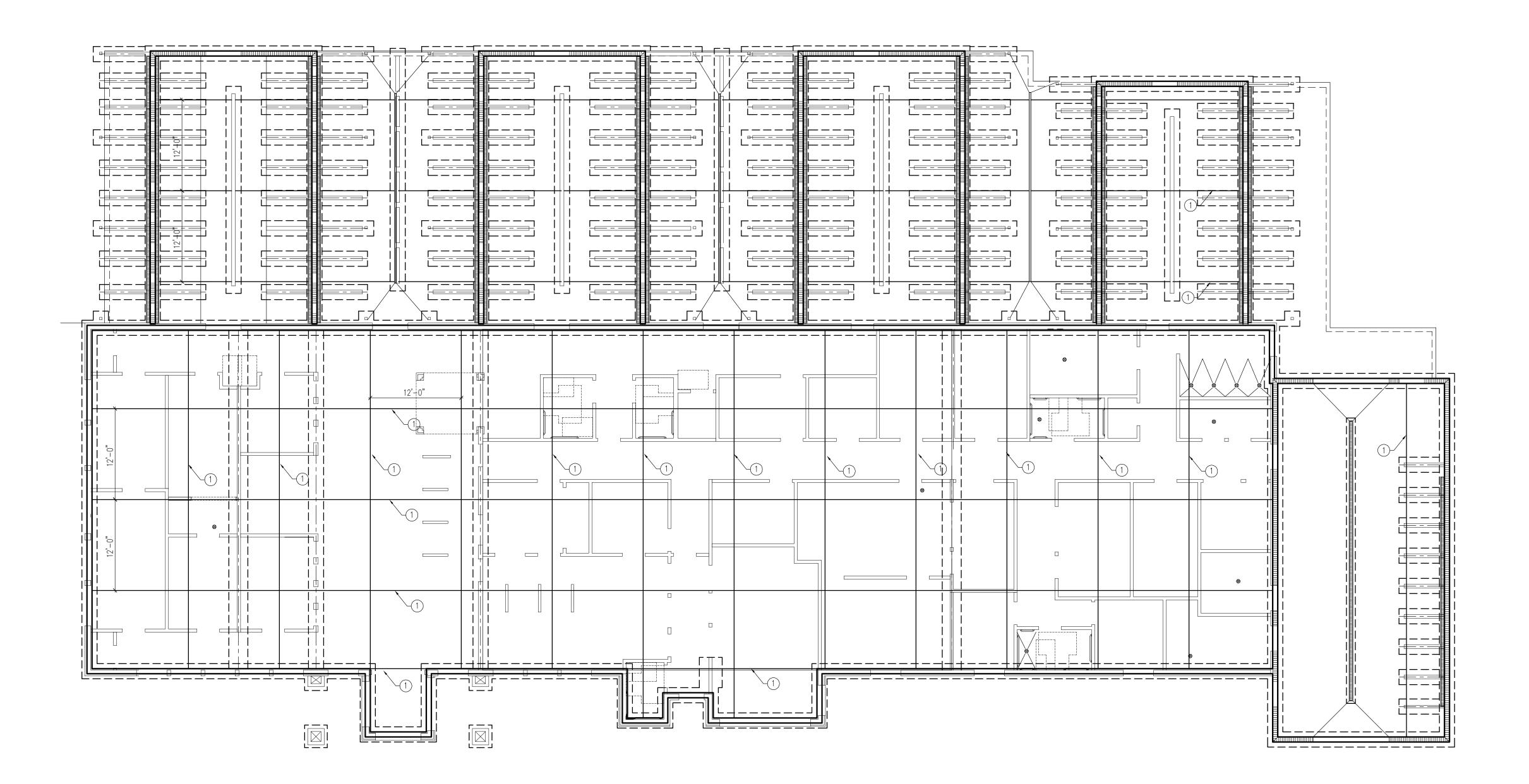
ANY EXCAVATIONS. **DO NOT SCALE DRAWINGS.**

GENERAL NOTES:

- 1. REFER TO GENERAL NOTES SHEETS FOR ALL REQUIREMENTS.
- REFERENCE DETAILS ON SHEET SO.2A AS FOLLOWS: TYPICAL JOINT LAYOUT DETAILS CONTROL/CONTRACTION JOINT DETAILS ISOLATION/EXPANSION JOINT DETAILS CONSTRUCTION JOINT DETAILS TYPICAL REINFORCEMENT AT RE-ENTRANT CORNERS TYPICAL REINFORCING AROUND SLAB-ON-GRADE OPENINGS

KEYED NOTES:

1. CONTROL JOINT - SEE SO.2A FOR DETAILS.



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JOINT PLAN

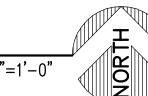
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FOUNDATION CONTROL JOINT PLAN



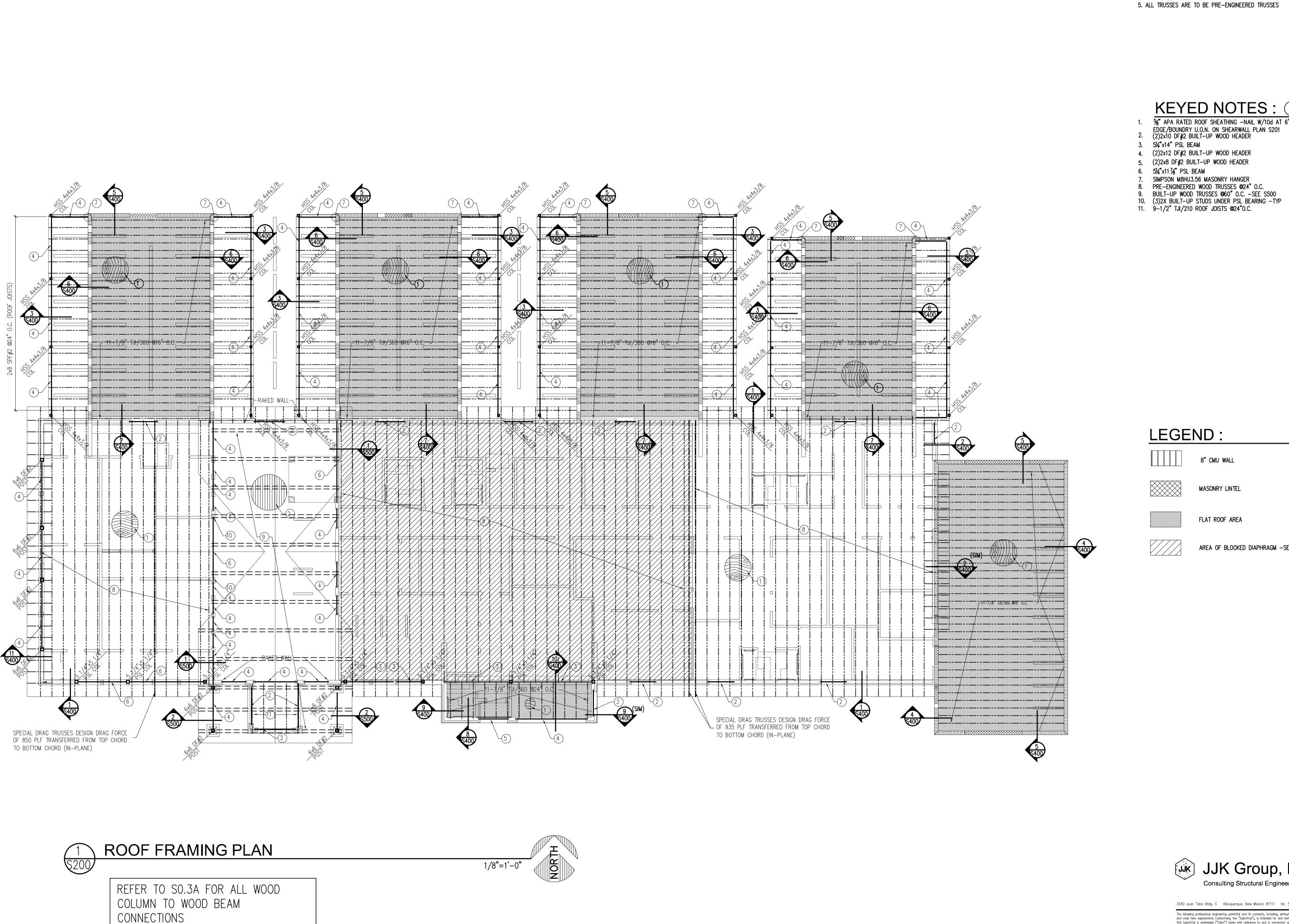
1/8"=1'-0"

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GENERAL NOTES:

- 1. REFER TO GENERAL NOTES SHEETS FOR ALL REQUIREMENTS.
- 2. REFER TO ARCH. FOR ALL BEARING HEIGHTS.
- 3. CONNECTION HARDWARE TO BE SIMPSON U.O.N..
- 4. REFER TO SHEET SO.3A FOR WOOD COLUMN POST CAP BASE CONNECTIONS.

KEYED NOTES:

- 1. 5%" APA RATED ROOF SHEATHING -NAIL W/10d AT 6" O.C. EDGE/BOUNDRY U.O.N. ON SHEARWALL PLAN S201
 2. (2)2x10 DF#2 BUILT-UP WOOD HEADER

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LEGEND:



FLAT ROOF AREA

MASONRY LINTEL

AREA OF BLOCKED DIAPHRAGM -SEE S201

RAMIN

ROOF

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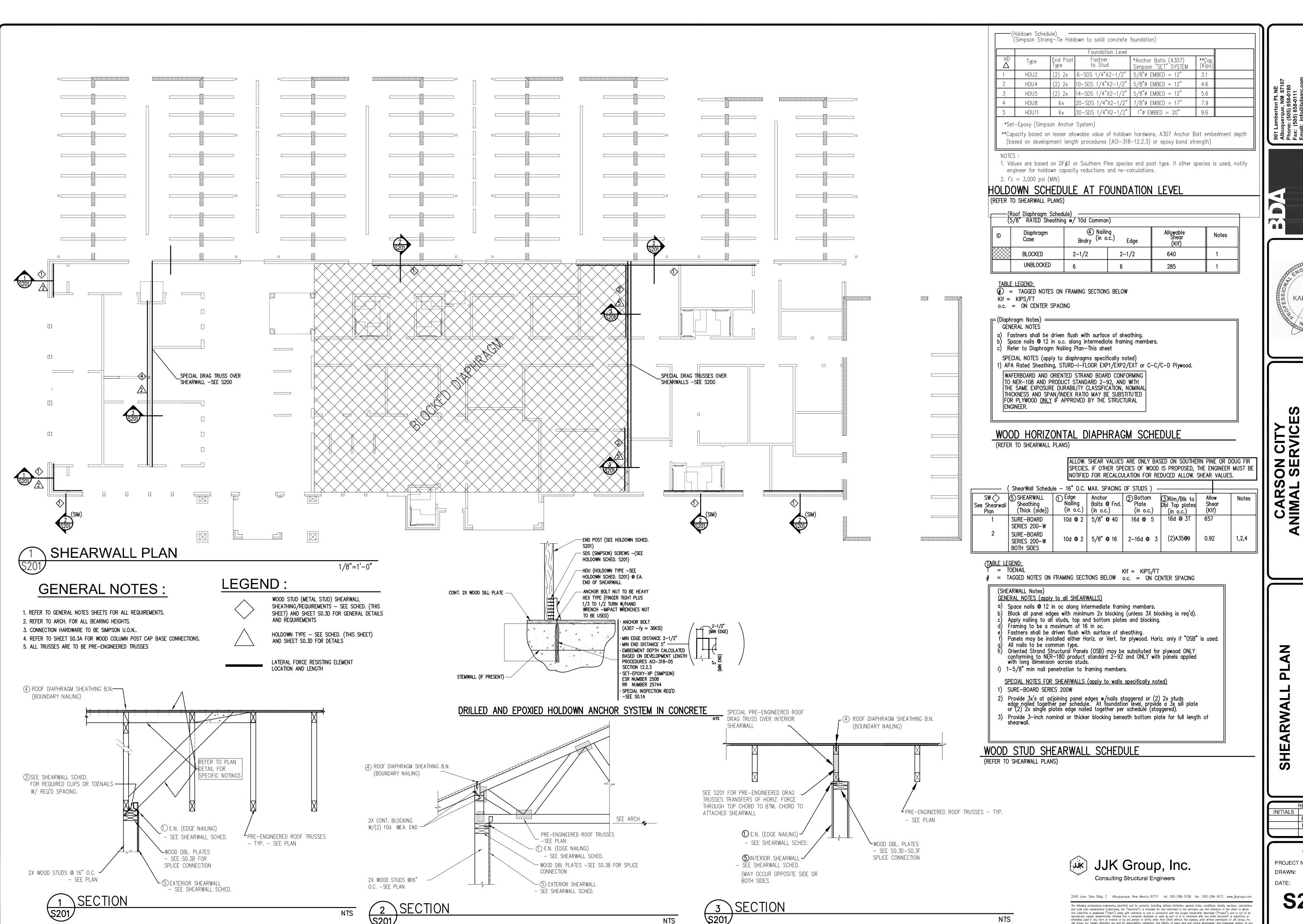
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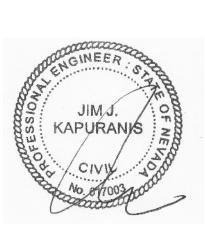
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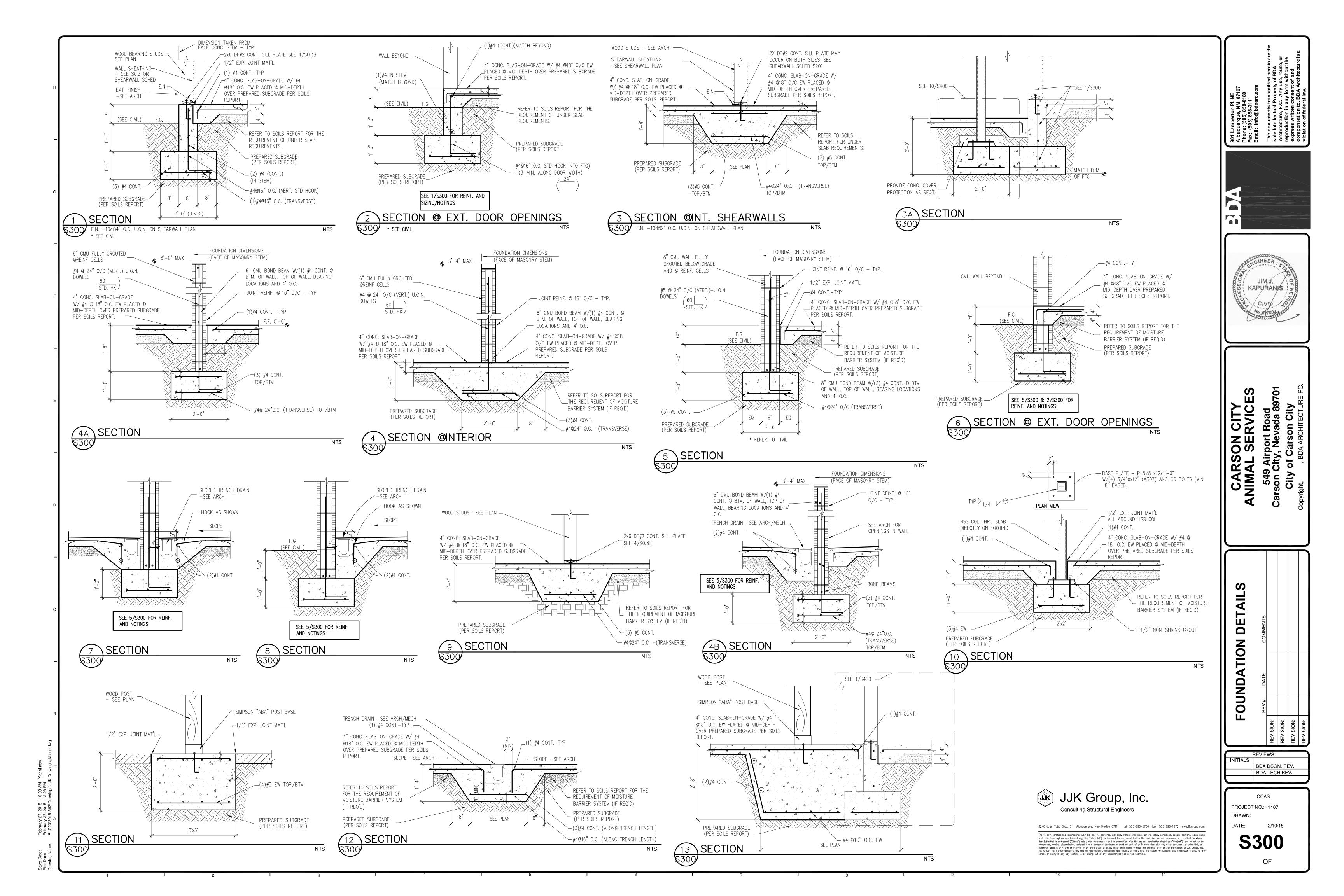
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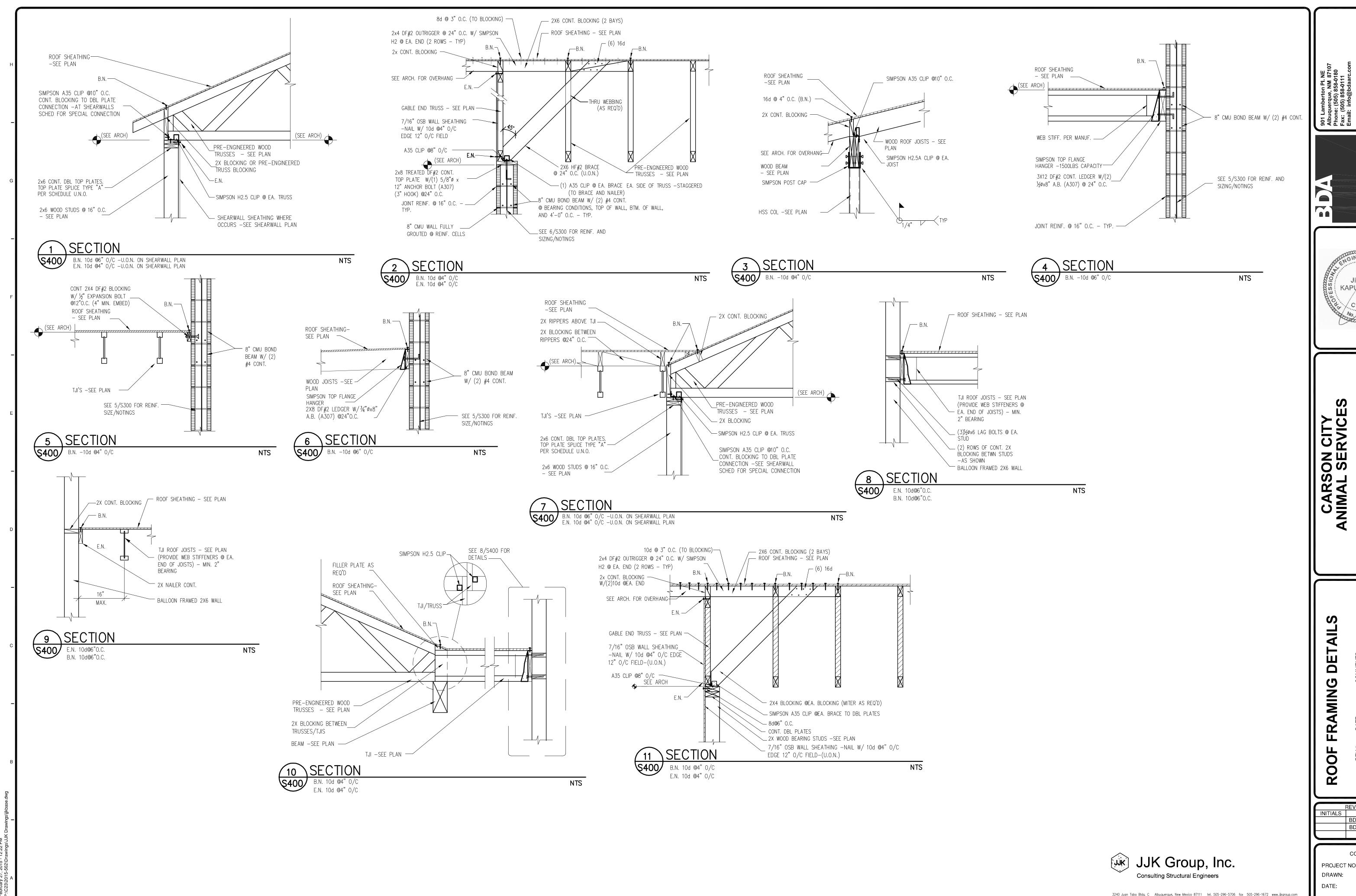
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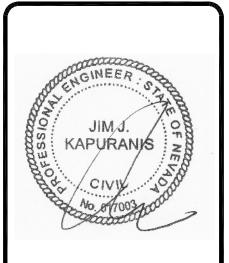
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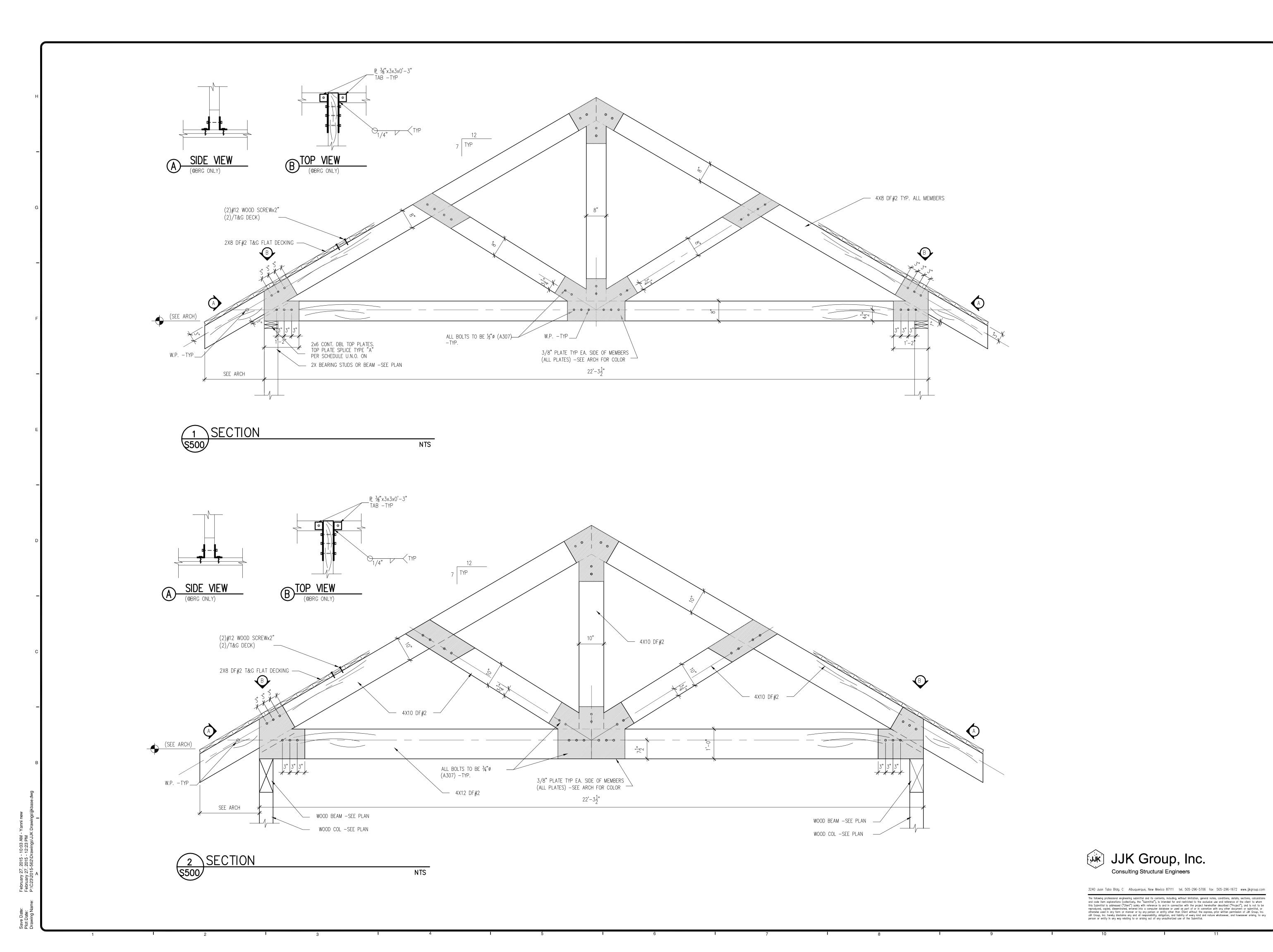




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