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CARSON CITY ANIMAL SERVICES

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IRRIGATION PLAN

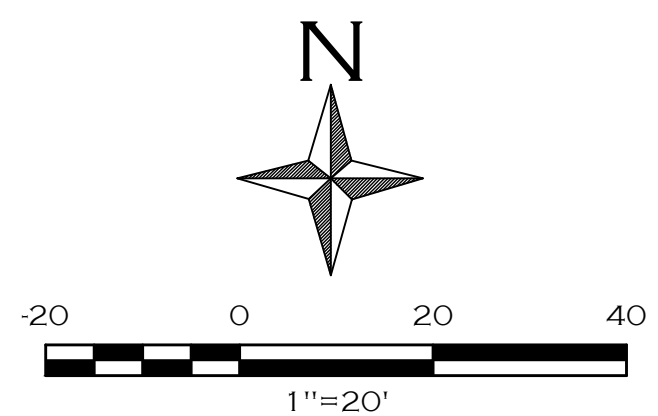
REV.#	DATE	COMMENTS

REVIEWS

INITIALS	REVISIONS
SW	SWA DSGN. REV.
SW	SWA TECH REV.

CCAS
 PROJECT NO: 14-227
 DRAWN: SW
 SCALE: 1"=20'-0"
 DATE: 07/23/15

L5



IRRIGATION LEGEND

- 1" Mueller Curb Stop and Waste Valve (or approved equal)
 - 1" RPA Wilkins or Watts (Reduced Pressure Backflow Preventer) Provide Winterization Blanket sized to unit.
 - 1" Brass manual drain with crosstop. Locate after backflow preventer and at low point(s) on mainline.
 - Irrigation Controller: Hunter Pro-C FC-4 (4 station module) with outdoor rated, locking cabinet.
 - 1" Quick Coupler, Rain Bird 44 RC with locking cover
 - RainBird Sprinkler Valve: 1" FEB valve.
 - RainBird Drip Valves: XCZ-FRB-100-COM, 1" size. Medium Flow Commercial Control Zone Kit with Pressure Regulating Basket Filter.
- Piping:** all schedule 40 PVC. Pressure test lines and lay valve wires prior to backfilling trenches.
- 1" and 1.5" Pressure mainlines, min. 36" deep up to backflow and 24" deep up to valves.
 - Sprinkler laterals. Size per plan. Minimum 1.5" deep.
 - Sleeving min. 2x the diameter of the piping to be sleeved. All sleeving min. 24" under drives, and 18" (for lateral) or 12" (for drip tube) under walks.
 - Extra 1" PVC sleeving for valve wires.
 - 3/4" polytubing drip line, buried 6" under soil and decomposed granite mulch. Stakes with wire staples 25" on center to secure in place.
 - Valve Size 1.40
Gallons per minute (GPM) or Drip

SPRINKLER LEGEND

Hunter FGJ Rotor, Radius 15'-31'

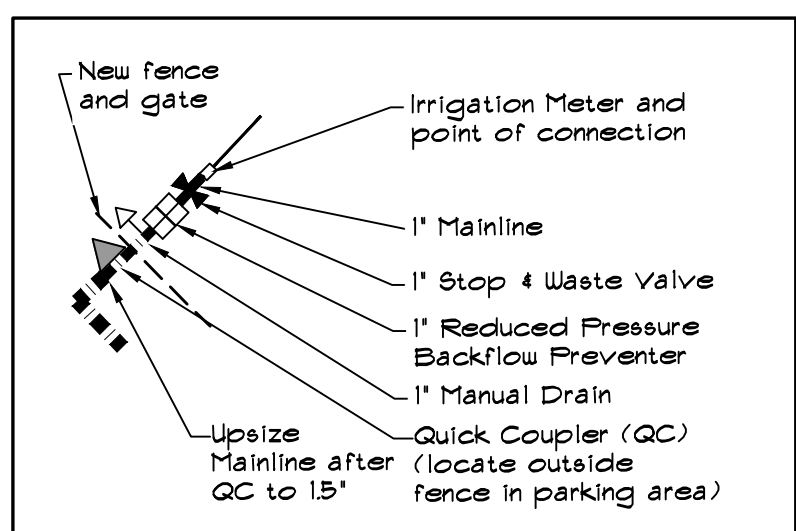
- Nozzle 2.0, 25" Radius at 50 psi 23 GPM
- Nozzle 1.5, 22" Radius at 50 psi 17 GPM
- Nozzle 1.5, 17" Radius at 50 psi 25 GPM

Adjust radius and nozzle sizes as needed to provide optimal coverage and prevent overspray.

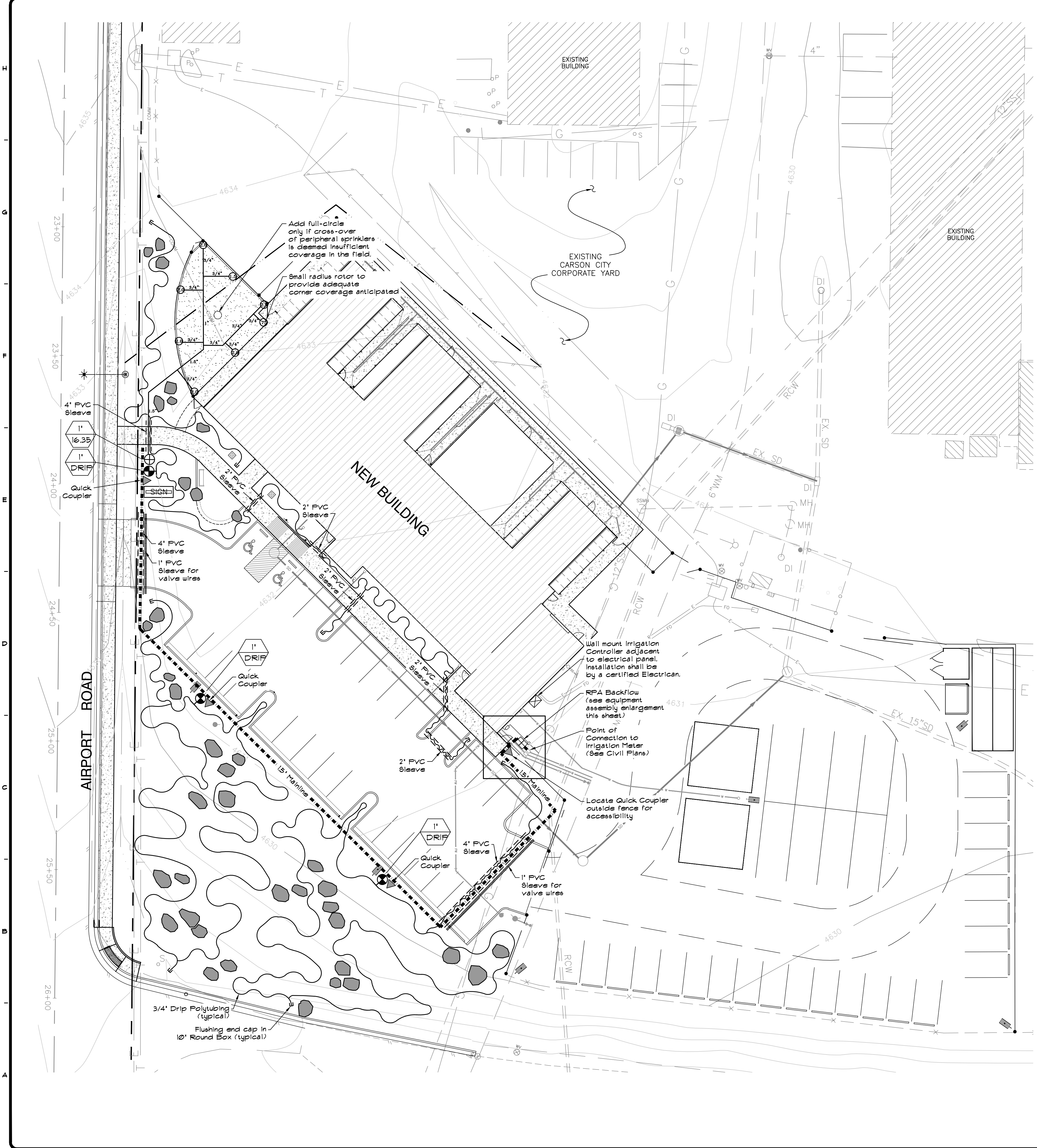
DRIP EMITTER QUANTITIES

See Section 3.0 Execution, 3.3 Drip Irrigation for the number of required emitters on this project. Carson City no longer recommends (2) half-gallon emitters on one gallon plant material due to clogging. All emitter quantities have been adjusted to conform to a new requirement for (2) one gallon emitters/one gallon plant. Adjust valve irrigation times to compensate for higher-per-minute irrigation volumes.

SEE NOTES SHEET L4.

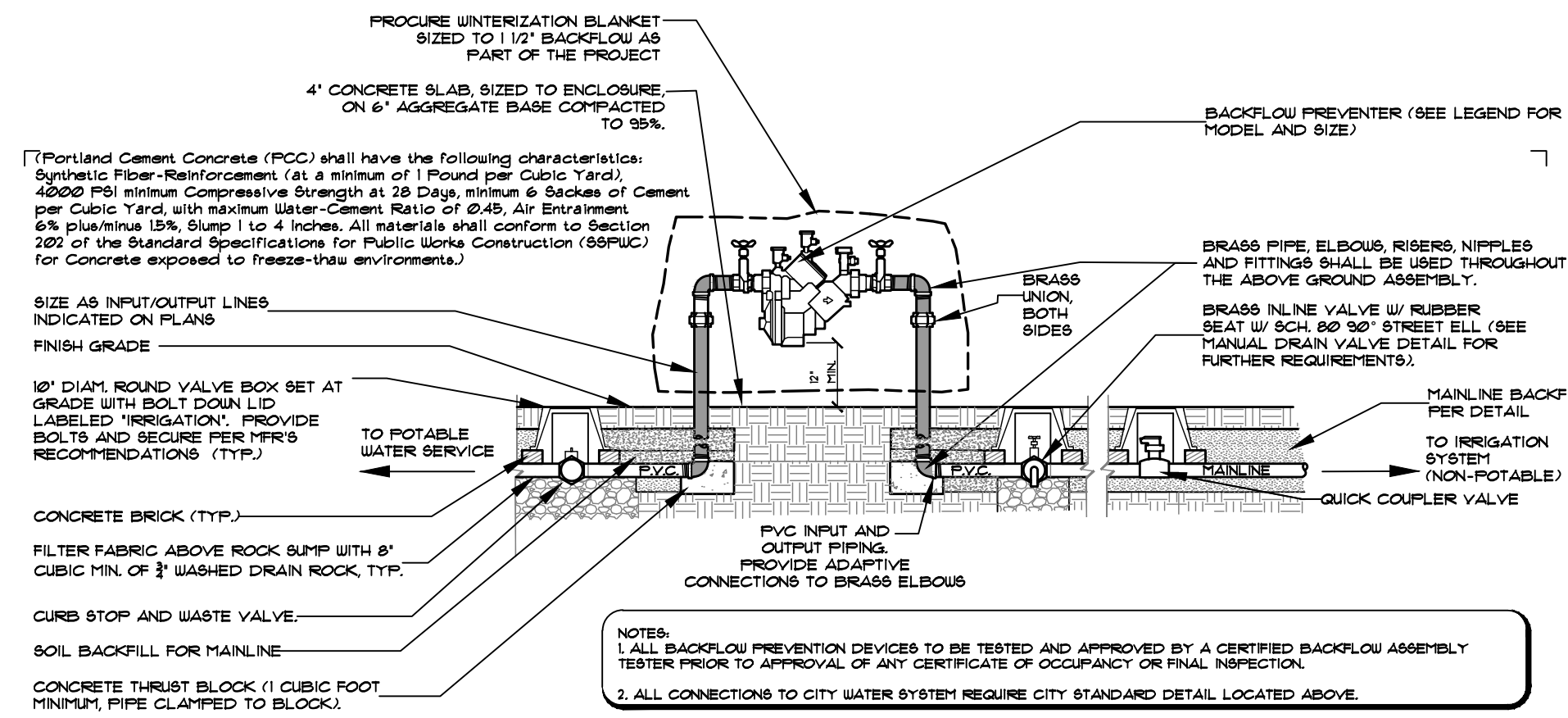


IRRIGATION RPA ASSEMBLY ENLARGEMENT

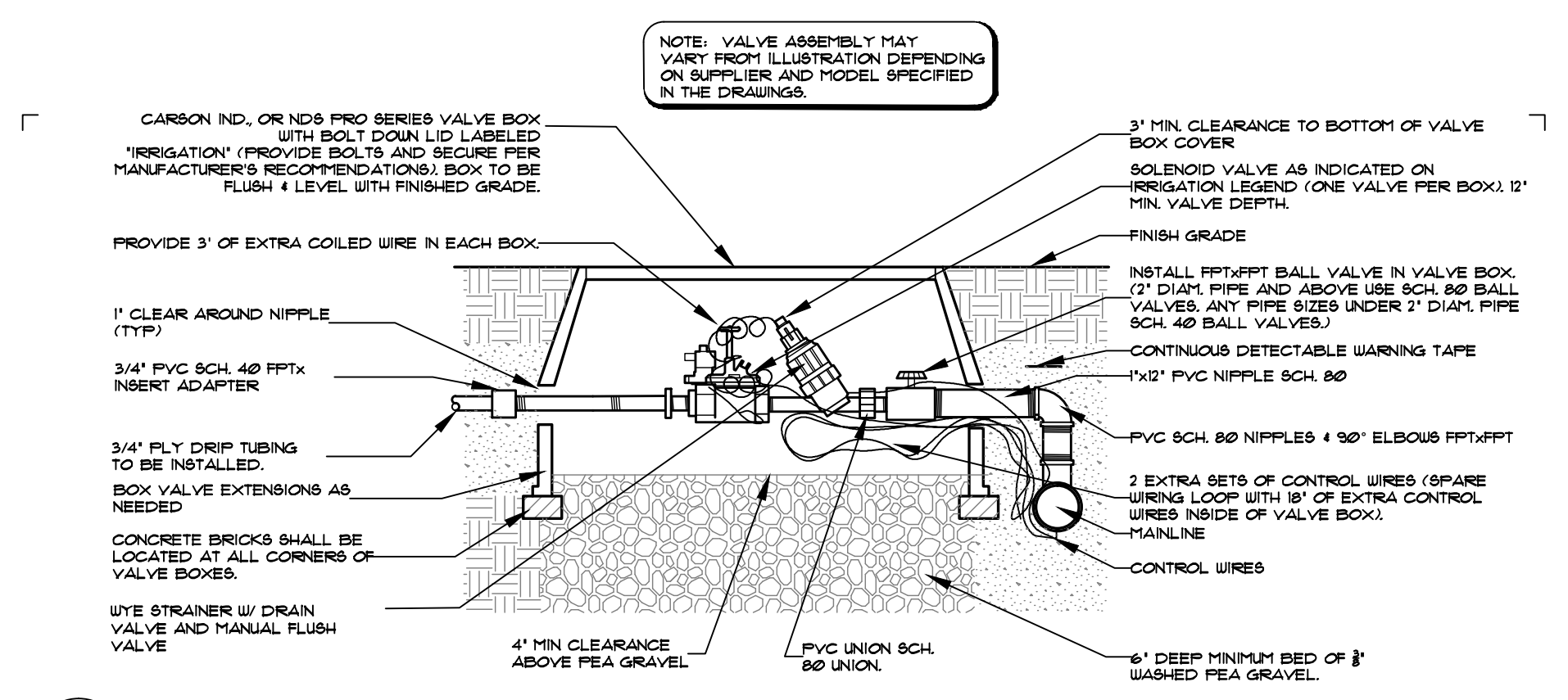


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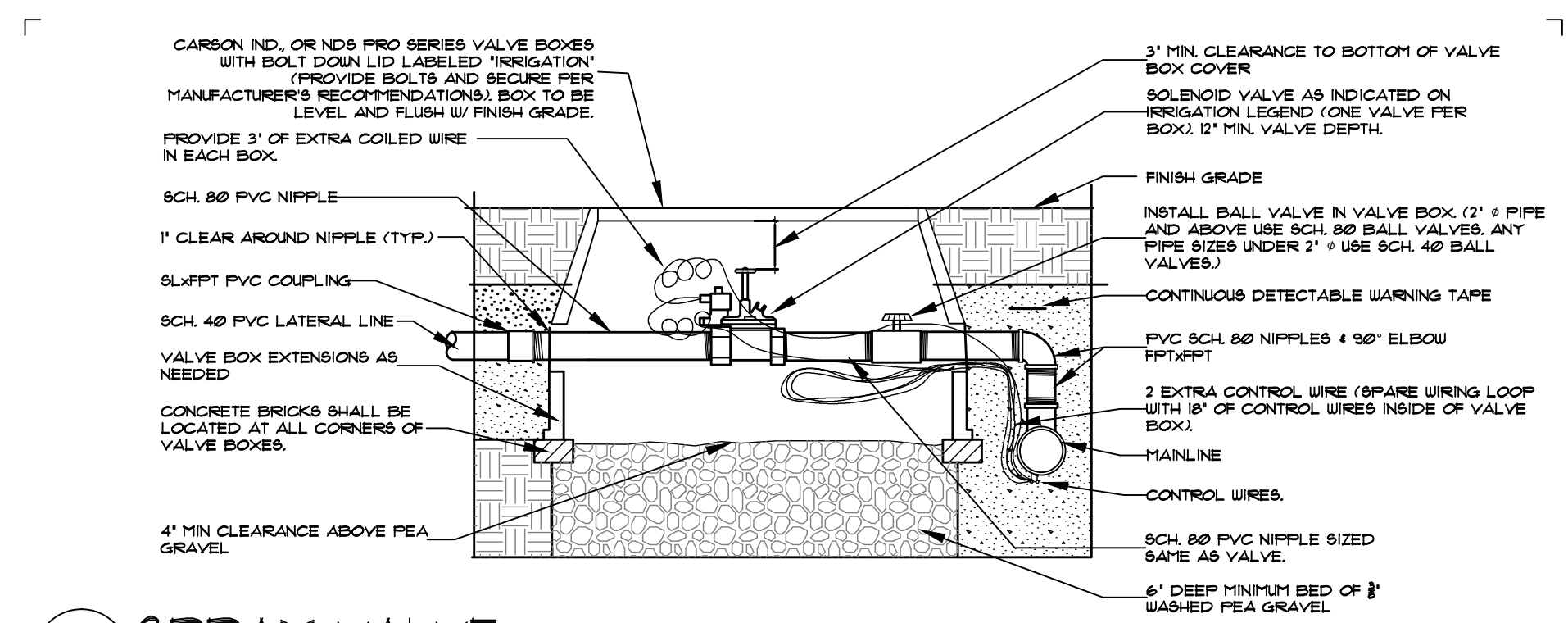
DRAWING ISSUE DATE 07/23/15



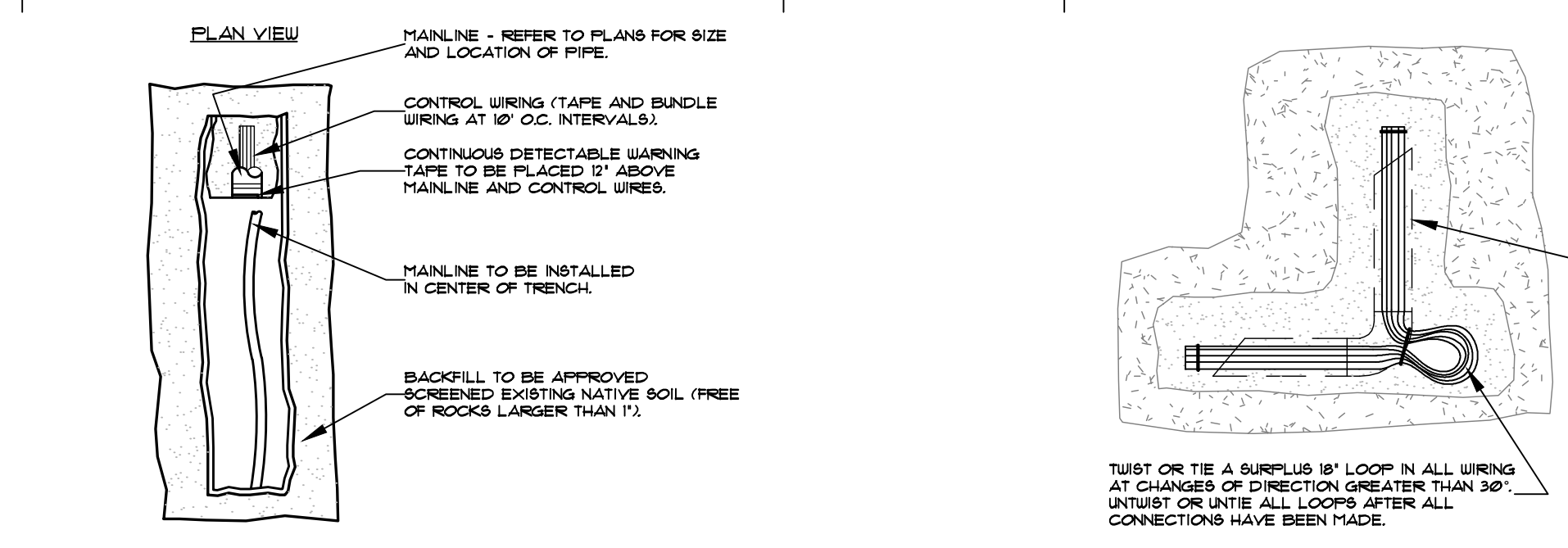
1.1 BACKFLOW PREVENTER
NOT TO SCALE



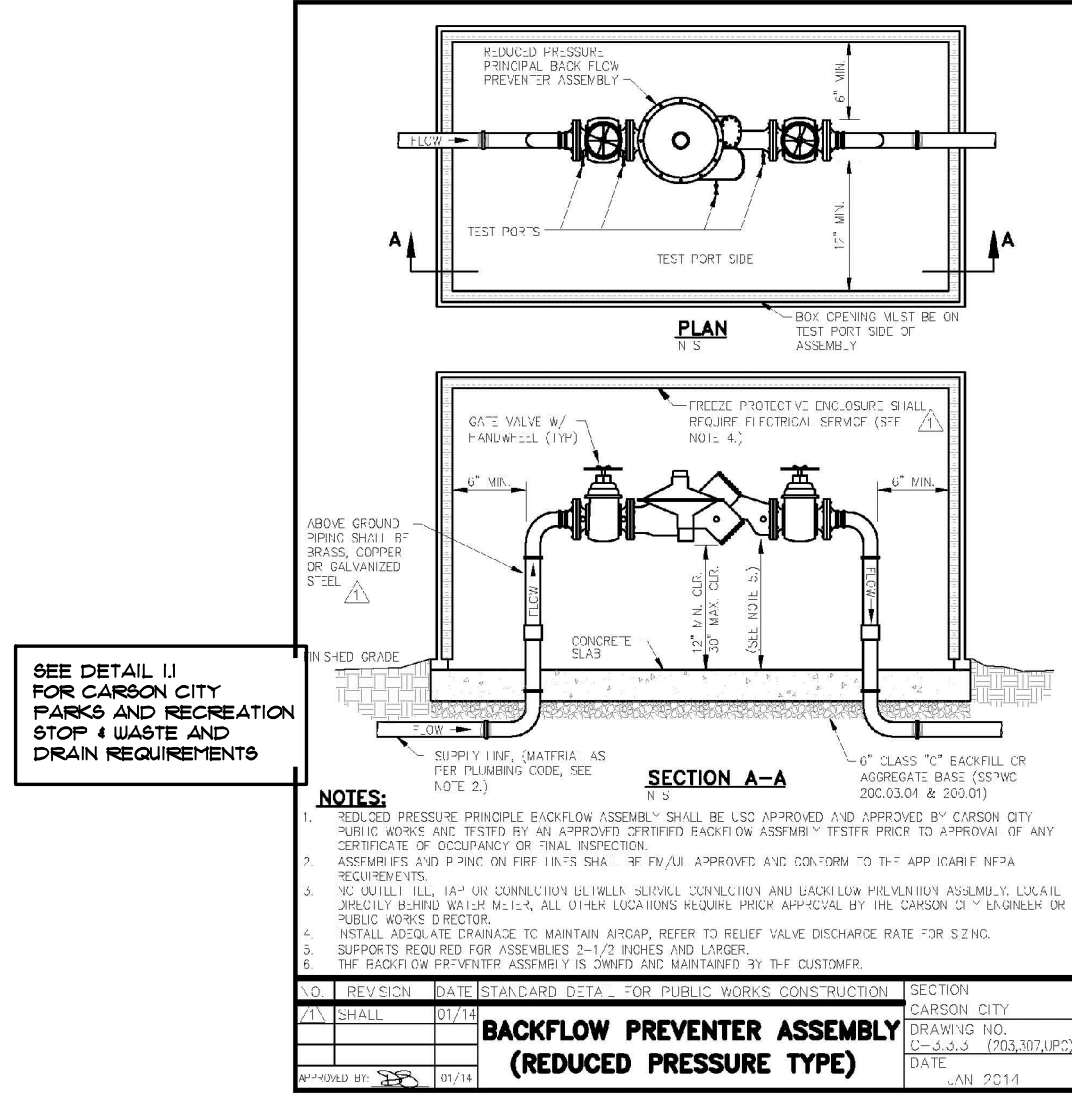
1.4 RAINBIRD XGZ-PRB-100-COM ASSEMBLY
NOT TO SCALE



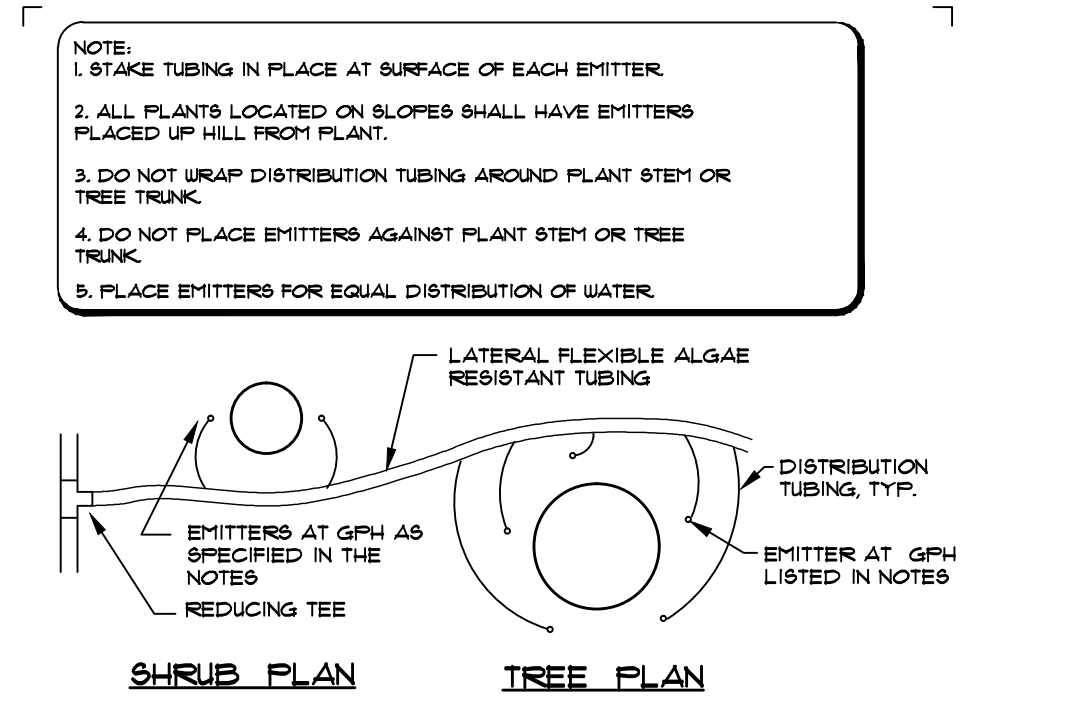
1.8 SPRAY VALVE
NOT TO SCALE



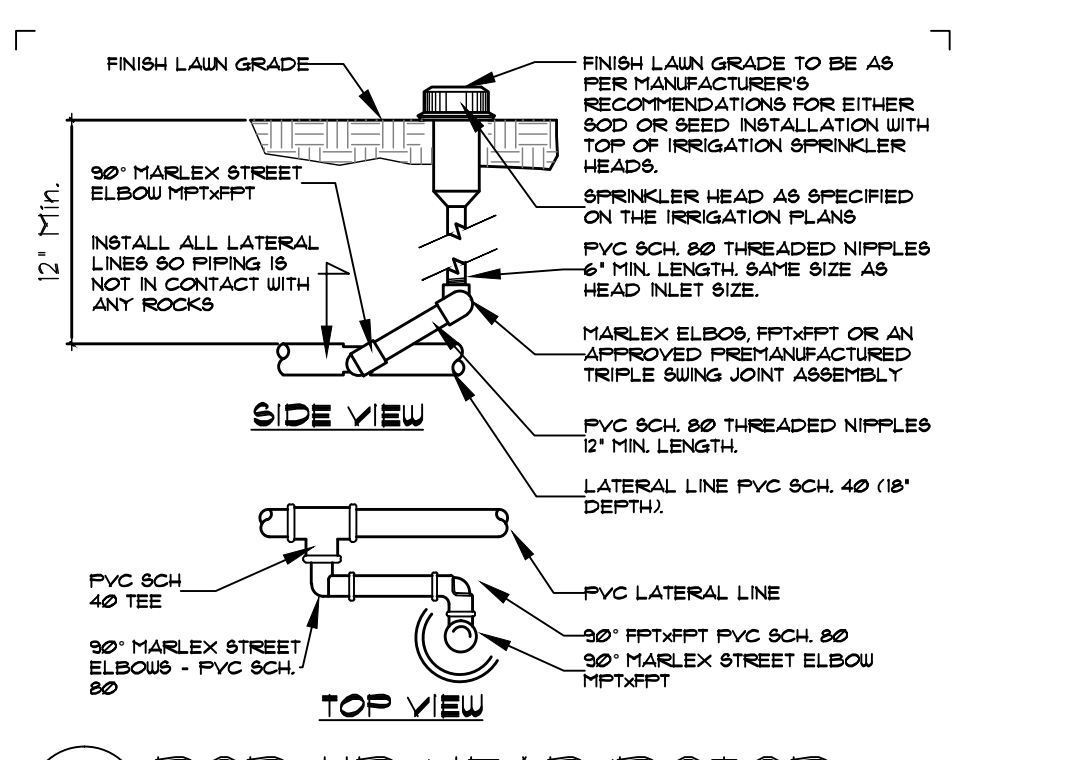
1.12 TRENCH SECTION (with wire)
NOT TO SCALE



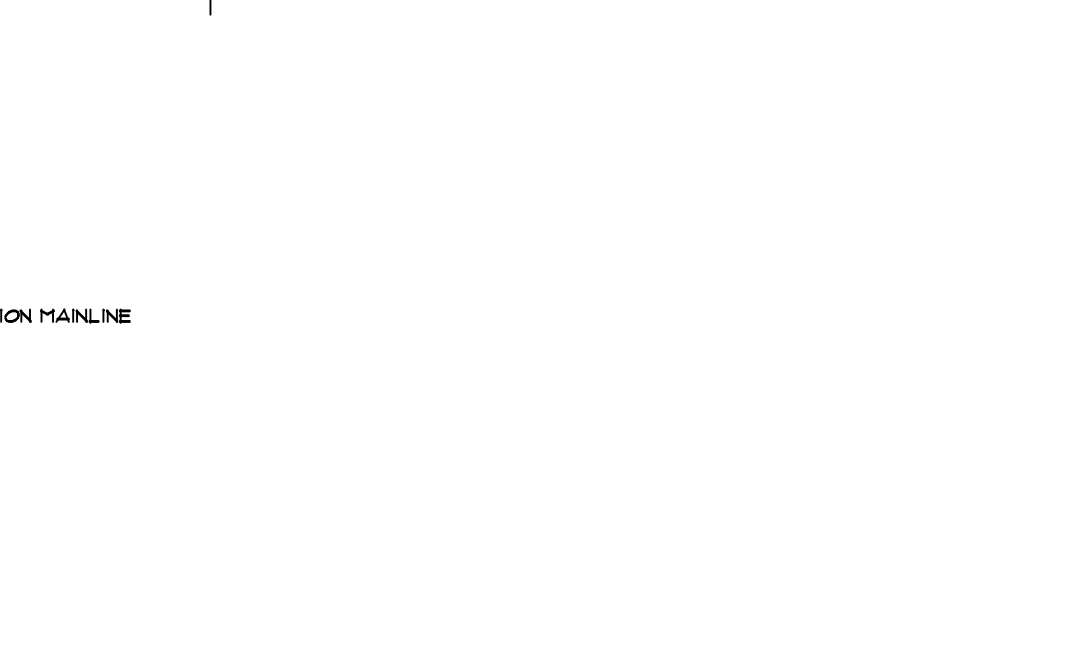
1.2 THRUST BLOCK DETAIL
NOT TO SCALE



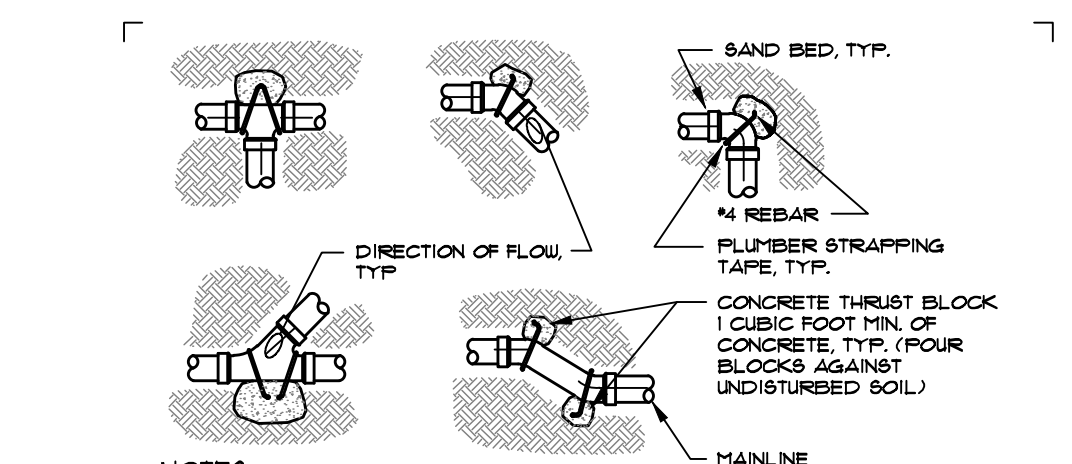
1.5 DRIP EMITTER
NOT TO SCALE



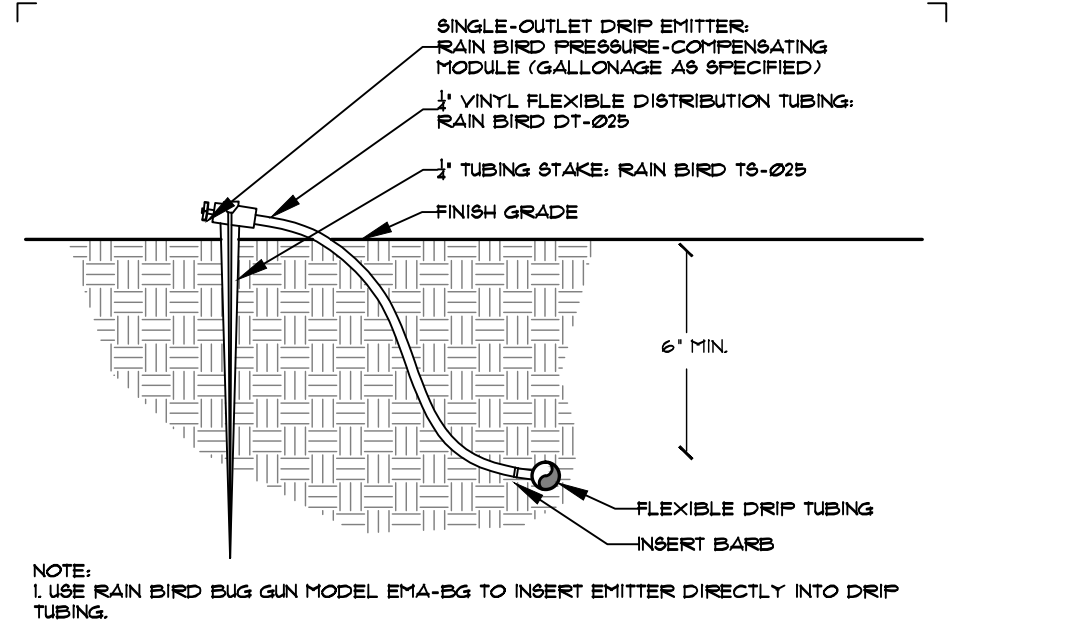
1.9 POP-UP HEAD/ROTOR
NOT TO SCALE



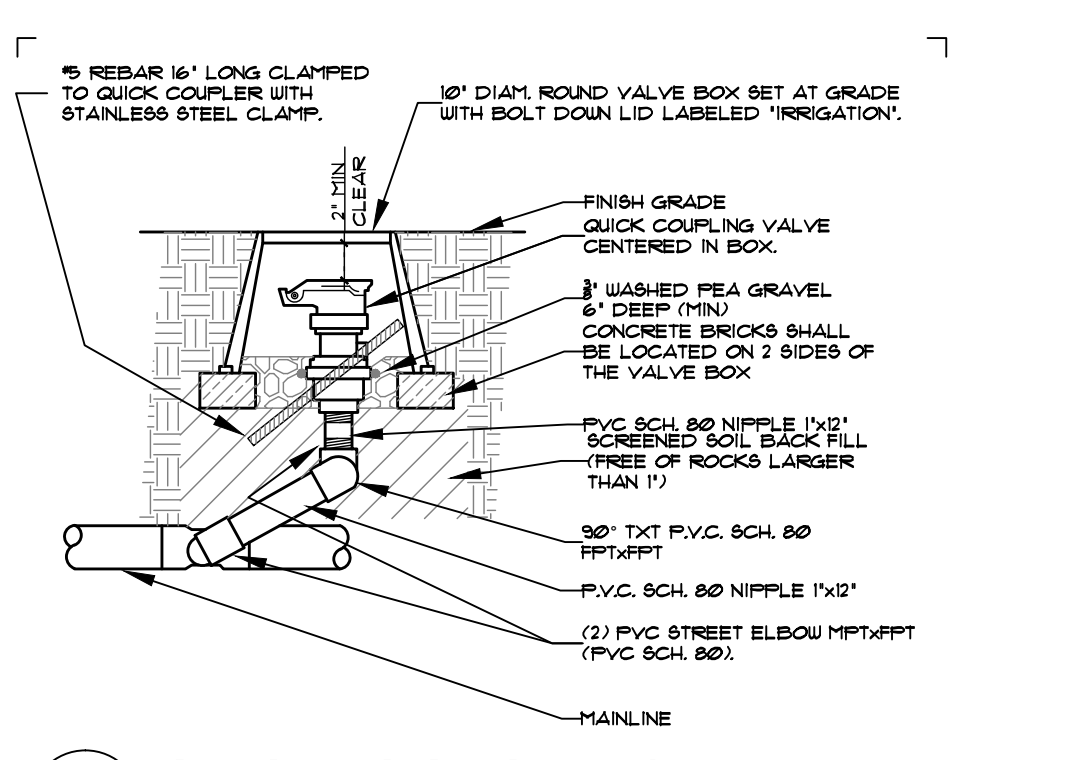
1.13 CONTROLLER WIRING
NOT TO SCALE



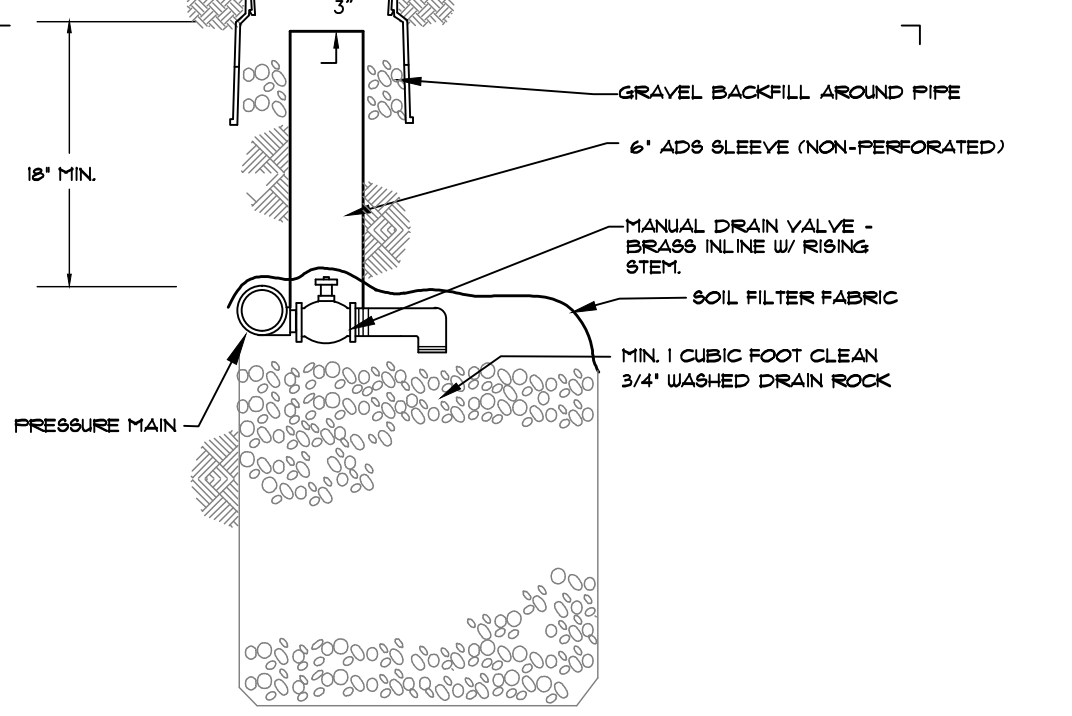
1.3 MANUAL DRAIN VALVE
NOT TO SCALE



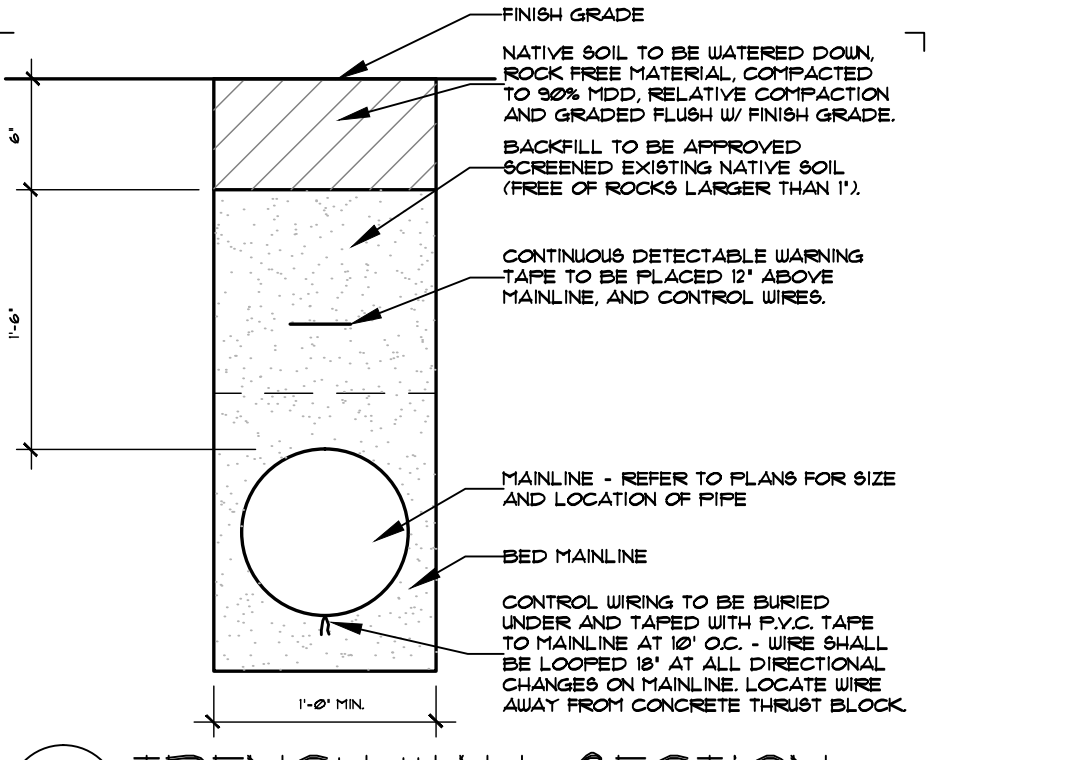
1.6 DRIP EMITTER STAKING DETAIL
NOT TO SCALE



1.10 QUICK COUPLING VALVE
NOT TO SCALE



1.7 FLUSHING END CAP
NOT TO SCALE



1.11 TRENCH WALL SECTION
NOT TO SCALE

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L6



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DRAWING ISSUE DATE 07/23/15

CONCRETE SLAB-ON-GRADE REQUIREMENTS

GENERAL

1. SAW-CUT JOINTS AS SOON AS THE SLAB WILL SUPPORT THE WEIGHT OF THE SAW AND OPERATOR WITHOUT DISTURBING THE FINAL FINISH.
2. THE DEPTH OF SAW-CUT WHEN USING A WET CUT SAW SHALL BE 1/4" THE SLAB THICKNESS.
3. PROVIDE CONSTRUCTION JOINT AT THE END OF CONCRETE PLACEMENT FOR THE DAY. SEE DETAIL A (THIS SHEET).
4. SEE FOUNDATION AND/OR CONTROL JOINT PLAN FOR ADDITIONAL INFORMATION.
5. SEE GENERAL NOTE SHEET S0.2 FOR ADDITIONAL CONCRETE REQUIREMENTS.
6. CONTRACTOR TO FAMILIARIZE HIMSELF WITH SOILS REPORT FOR SUBGRADE PREPARATION.

DESIGN

THE FOLLOWING MINIMUM ALLOWABLE REINFORCING RATIO USED IN THE DESIGN IS AS FOLLOWS:
 .1 % FOR WELDED WIRE FABRIC
 .2 % FOR REINFORCING BARS
 THIS MINIMUM REINFORCING RATIO IS FOR SHRINKAGE AND KEEPING RANDOM CRACKING TIGHT. IT ALSO ALLOWS FOR LONGER JOINT SPANS.

CONTROL /CONTRACTION JOINT SPACING WITH MINIMUM REINFORCING IS BASED ON THE FOLLOWING SUBGRADE DRAG FORMULA:

$$As = FLW / 2 Fs \quad \text{WHERE:}$$

- As CROSS-SECTION AREA OF STEEL, IN SQUARE INCHES PER LINEAL FOOT OF SLAB WIDTH
- F COEFFICIENT OF SUBGRADE FRICTION. (DESIGNERS USE 1.5 OR 2.0 FOR PAVEMENTS; 1.5 IS RECOMMENDED FOR CONCRETE FLOORS ON GROUND.)
- L SLAB LENGTH (OR WIDTH IF APPROPRIATE) BETWEEN FREE ENDS, IN FEET. (A FREE END IS ANY JOINT FREE TO MOVE IN A HORIZONTAL PLANE.)
- W WEIGHT OF SLAB, IN POUNDS PER SQUARE FOOT. (FOR NORMAL-WEIGHT CONCRETE, DESIGNERS USE 12.5 POUNDS PER INCH OF FLOOR THICKNESS.)
- Fs ALLOWABLE WORKING STRESS OF REINFORCEMENT, IN POUNDS PER SQUARE INCH. (THE WORKING STRESS OF STEEL IS USUALLY 0.67 TO 0.75 TIMES THE YIELD STRENGTH OF THE STEEL IN POUNDS PER SQUARE INCH.)

DEFINITIONS

ISOLATION JOINTS

ISOLATION JOINTS ARE PLACED WHEREVER COMPLETE SEPARATION BETWEEN THE FLOOR AND ADJOINING CONCRETE IS NEEDED TO ALLOW THEM TO MOVE INDEPENDENTLY WITHOUT DAMAGE. ISOLATION JOINTS PERMIT HORIZONTAL AND VERTICAL MOVEMENT BETWEEN THE ABUTTING FACES OF THE FLOOR SLAB AND OTHER PARTS OF THE BUILDING BECAUSE THERE IS NO KEYWAY, BOND OR MECHANICAL CONSTRUCTION ACROSS THE JOINT.

CONTROL / CONTRACTION JOINTS

CONTROL JOINTS (ALSO CALLED CONTRACTION JOINTS) ACT TO RELIEVE STRESS AND WITH PROPER SPACING (SEE CONTROL JOINT PLAN) THEY ELIMINATE THE CAUSE OF UNCONTROLLED RANDOM CRACKING. THEY ALLOW HORIZONTAL MOVEMENT OF THE SLAB. THE OBJECTIVE IS TO FORM A PLANE OF WEAKNESS IN THE SLAB SO THAT THE CRACK WILL OCCUR ALONG THAT LINE AND NOWHERE ELSE. AS SHOWN ON SECTIONS ON THIS SHEET ALL SLAB REINFORCING MUST BE DISCONTINUOUS THROUGH JOINT. LOAD TRANSFER ACROSS THE CONSTRUCTION JOINT IS PROVIDED BY USE OF DOWELS (A BOND BREAKER IS USED ON ONE END TO ALLOW HORIZONTAL MOVEMENT).

CONSTRUCTION JOINT

CONSTRUCTION JOINTS ARE STOPPING PLACES AND FORM THE EDGE OF EACH DAY'S WORK. THEY FREQUENTLY ALIGN WITH CONTROL / CONTRACTION JOINTS OR ISOLATION JOINTS. WHENEVER CONTINUOUS CONCRETE PLACEMENT WILL BE INTERRUPTED FOR 30 MINUTES OR MORE, A BONDED OR TIED CONSTRUCTION JOINT SHOULD BE FORMED AND DEFORMED REINFORCING BARS ADDED. IF THE CONSTRUCTION JOINT OCCURS WITHIN THE PANEL (I.E. BETWEEN SPECIFIED CONTROL / CONTRACTION JOINTS) ALL REINFORCING MUST CONTINUE THROUGH THE CONSTRUCTION JOINT.

VISIBLE CONDITIONS THAT MAY OCCUR DURING CONSTRUCTION

RANDOM CRACKING

WHEN RANDOM CRACKING OCCURS ON A NEWLY PLACED SLAB, IT IS USUALLY RELATED TO IMPROPER TIMING OF JOINT SAWING. THE PURPOSE OF CUTTING THE SLAB IS TO INDUCE A CRACK BENEATH THE CUT.

CONCRETE NEEDS TO GAIN ADEQUATE STRENGTH BEFORE HAVING JOINTS CUT INTO IT. IDEALLY, THE TENSILE STRENGTH HOLDS THE SLAB TOGETHER. THE SAWCUT NOTCH CREATES A REDUCED SLAB SECTION, WHICH INCREASES TENSILE STRESSING IN THE CONCRETE BELOW THE NOTCH. IN THE REDUCED SECTION, THE TENSILE STRESS IS OF GREATER MAGNITUDE THAN THE CONCRETE TENSILE STRENGTH. THIS A CRACK OCCURS BELOW THE NOTCH. THE CRACK AND SAWCUT COMBINE TO RELIEVE THE STRESSES AND THUS PREVENT UNWANTED RANDOM CRACKING. BUT NEW CONCRETE IS ALWAYS TRYING TO SHRINK. AS THE SAWBLADE CUTS A JOINT IN THE CONCRETE, THE SAWCUT WEAKENS THE CONCRETE SLAB. IF SAWCUTTING IS STARTED WHEN CONTRACTION STRESS (AS A RESULT OF CONCRETE SHRINKAGE) IS GREAT AND TENSILE STRENGTH IS NOT YET ADEQUATE TO RESIST IT, CRACKS CAN JUMP AHEAD OF THE BLADE DURING JOINT CUTTING.

IF COOLING WATER (USED WITH WET SAWING) HITS THE WARM SLAB, IT CAN BE A THERMAL SHOCK THAT ADDS TO THE POTENTIAL FOR RANDOM CRACKING AHEAD OF THE SAW BLADE.

TO AVERT RANDOM CRACKING, SAWCUT JOINTING MUST BE DONE BEFORE CONCRETE COOLING AND DRYING STARTS, BUT AFTER SOME (TENSILE) STRENGTH HAS DEVELOPED (7 HOURS MAXIMUM AFTER CONCRETE IS POURED). THE NOTCH INSTALLED BY SAWCUTTING SHOULD BE DEEP ENOUGH SO THAT THE CRACK OCCURS BELOW THE SAWCUT (3/4 OF THE SLAB THICKNESS IS SUFFICIENT).

BLEEDING AND SET RETARDING

EXCESSIVE BLEEDING THAT OCCURS AFTER CONCRETE PLACING, STRIKEOFF, AND BULLFLOATING CAN DELAY SUBSEQUENT FINISHING STEPS. IN MOST INSTANCES, THE CAUSE OF EXCESSIVE BLEEDING IS DUE TO ONE OF THE FOLLOWING:

- A WATER-CEMENT RATIO THAT IS TOO HIGH
- POOR AGGREGATE GRADATION
- SLOW SET TIMES
- AMBIENT CONDITIONS THAT HINDER SURFACE WATER EVAPORATION: LOW TEMPERATURES, HIGH HUMIDITY, OR LACK OF AIR MOVEMENT

BUSTLING AND DELAMINATIONS

BUSTLING IS THE CONVEX RAISING OF THE SURFACE MORTAR LAYER WHILE THE CONCRETE IS STILL PLASTIC. THE BUSTLES ARE ATTRIBUTED TO SEALING THE FLOOR SURFACE BEFORE ALL THE BLEEDWATER AND AIR HAVE ESCAPED.

SIMILAR TO BUSTLING, DELAMINATION OF SURFACE MORTAR CAN OCCUR DUE TO ENTRAPMENT OF BLEEDWATER AND AIR BELOW THE PREMATURELY SEALED MORTAR SURFACE. DELAMINATIONS AFFECT LARGER SURFACE AREAS THAN BUSTLES, AND ARE VERY DIFFICULT TO DETECT DURING FINISHING. THEY BECOME APPARENT AFTER CONCRETE SURFACE DRYING WHEN THE DELAMINATED AREA IS CRUSHED UNDER TRAFFIC. THE THICKNESS OF DELAMINATED MORTAR RANGES FROM ABOUT 3 MM TO 9 MM (1/8 IN TO 3/8 IN). THE AFFECTED AREA CAN BE ANYWHERE FROM A FEW SQUARE CENTIMETERS (INCHES) TO A FEW SQUARE METERS (YARDS).

IF THE CONCRETE HAS STIFFENED FROM THE TOP DOWN, AS IT OFTEN DOES WHEN WIND SPEEDS ARE HIGHER, THERE IS A TENDENCY TO FINISH THE SLAB TOO SOON, BEFORE BLEEDING IS COMPLETE. FINISHING OPERATIONS PERFORMED WHILE THE UNDERLYING CONCRETE IS STILL SOFT (AND BLEEDING) WILL SEAL THE SLAB SURFACE, POTENTIALLY TRAPPING BLEEDWATER AND LEADING TO DELAMINATIONS.

PLASTIC SHRINKAGE CRACKING

PLASTIC SHRINKAGE CRACKING IS DUE TO CONCRETE AT THE SURFACE DRYING (AND SHRINKING) BEFORE INITIAL SET OF THE CONCRETE OCCURS. PLASTIC SHRINKAGE OCCURS DURING AND AFTER FINISHING, USUALLY WHEN THERE IS RAPID EVAPORATION OF BLEED WATER. THE CONDITIONS THAT LEAD TO RAPID WATER EVAPORATION ARE LOW RELATIVE HUMIDITY, HIGH AIR TEMPERATURES, RAPID AIR MOVEMENT (WIND) ACROSS THE CONCRETE SURFACE, AND ELEVATED CONCRETE TEMPERATURES. UNDER THESE CONDITIONS THE CONCRETE SURFACE CAN CRUST OVER WHILE THE UNDERLYING CONCRETE IS STILL PLASTIC. AS PLASTIC SHRINKAGE CRACKS FORM, THEY START AT THE SURFACE AND MAY EXTEND SOME DEPTH INTO THE UNHARDENED CONCRETE. FLOATING THE CONCRETE SLAB CAN REPAIR PLASTIC SHRINKAGE CRACKS, BUT ONLY IF DONE IMMEDIATELY AS THE CRACKS OCCUR.

CRAZING

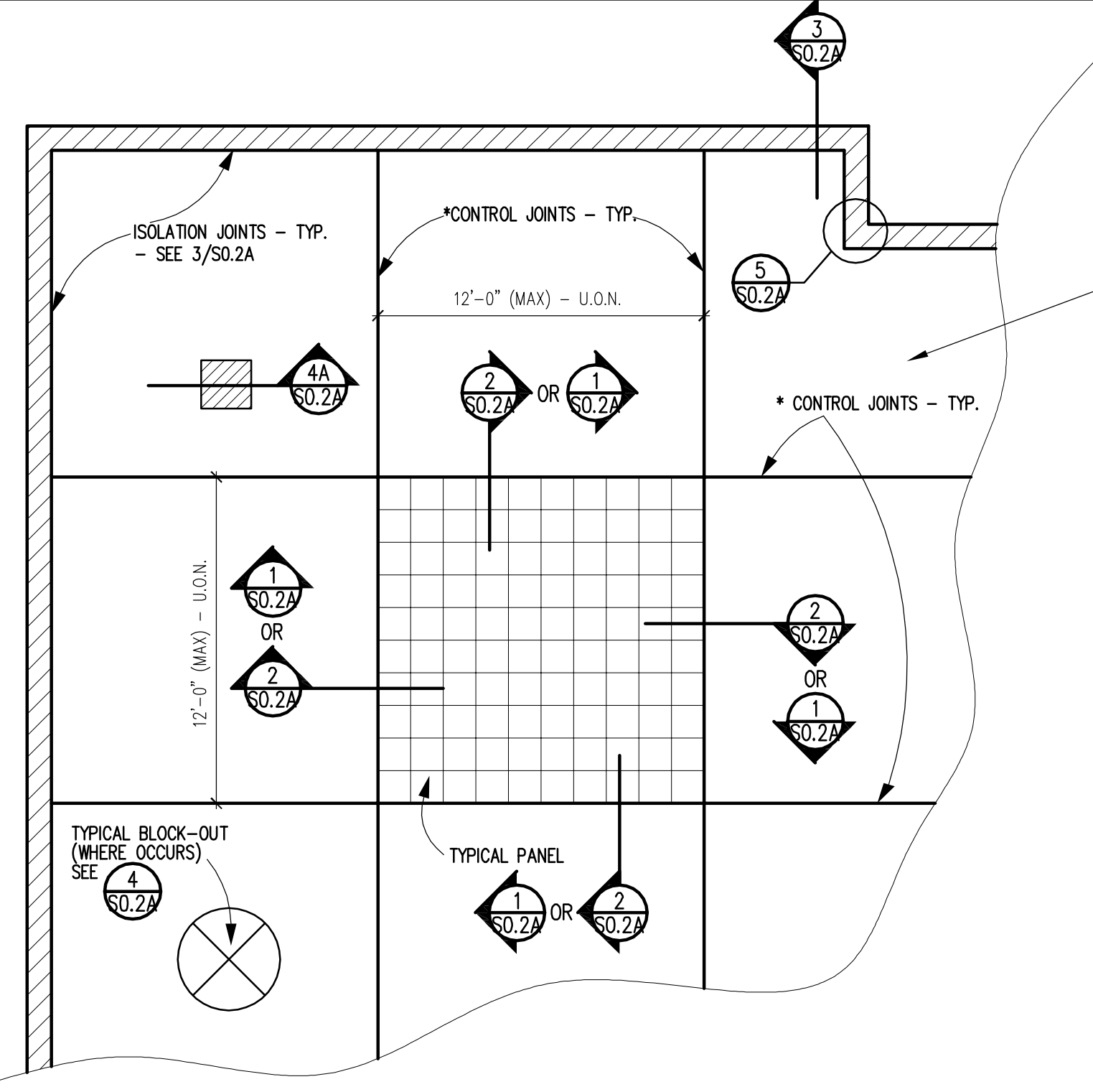
GRAZE CRACKS ARE FINE RANDOM CRACKS OR FISSURES IN A CONCRETE SURFACE. ON CONCRETE FLATWORK, THEY USUALLY EXTEND LESS THAN 3 MM (1/4 IN) BELOW THE SURFACE. THE CRACKS OCCUR WITHIN THE PASTE-RICH SURFACE MORTAR AND GENERALLY PASS THROUGH THE PASTE AND NOT THROUGH AGGREGATE PARTICLES. IT IS TYPICAL FOR THE CRACKS TO FORM A MAP PATTERN. THE NARROW CRACKS ARE SO FINE THAT THEY ARE DIFFICULT TO SEE. IN MANY INSTANCES, THEY ARE ONLY VISIBLE DURING THE DRYING PHASE OF A WETTED SURFACE OR WHEN A TRANSLUCENT COATING IS INSTALLED. CRAZE CRACKS ARE ATTRIBUTED TO INADEQUATE CURING THAT LEADS TO CONCRETE SURFACE DRYING AND COOLING BEFORE THE MORTAR HAS GAINED SUFFICIENT STRENGTH. THESE ARE COSMETIC BLEMISHES THAT GENERALLY HAVE NO EFFECT ON THE SERVICEABILITY OR DURABILITY OF THE FLOOR.

CURLING

WHEN THE EDGES AND CORNERS OF A FLOOR SLAB ON GROUND DISH UPWARD IN THE ABSENCE OF ANY LOADS OTHER THAN GRAVITY, THE SLAB IS SAID TO BE CURLING. IT IS USUALLY ATTRIBUTED TO DIFFERENCES IN MOISTURE CONTENT OR TEMPERATURE FROM TOP TO BOTTOM WITHIN THE SLAB. THESE TEMPERATURE AND MOISTURE GRADIENTS DEVELOP BETWEEN THE TOP AND BOTTOM SURFACES AS THE CONCRETE IN A FLOOR SLAB HARDENS. THE SLAB WILL CURL UP IF THE TOP IS TRYING AND COOLING (SHORTENING) WHILE THE BOTTOM REMAINS MOST AND WARM. UNDER OPPOSITE CONDITIONS, THE SLAB SHOULD THEORETICALLY CURL DOWN. DOWNWARD CURL AS SUCH, HOWEVER, DOES NOT OCCUR DUE TO SUBBASE RESTRAINT.

POPOUTS

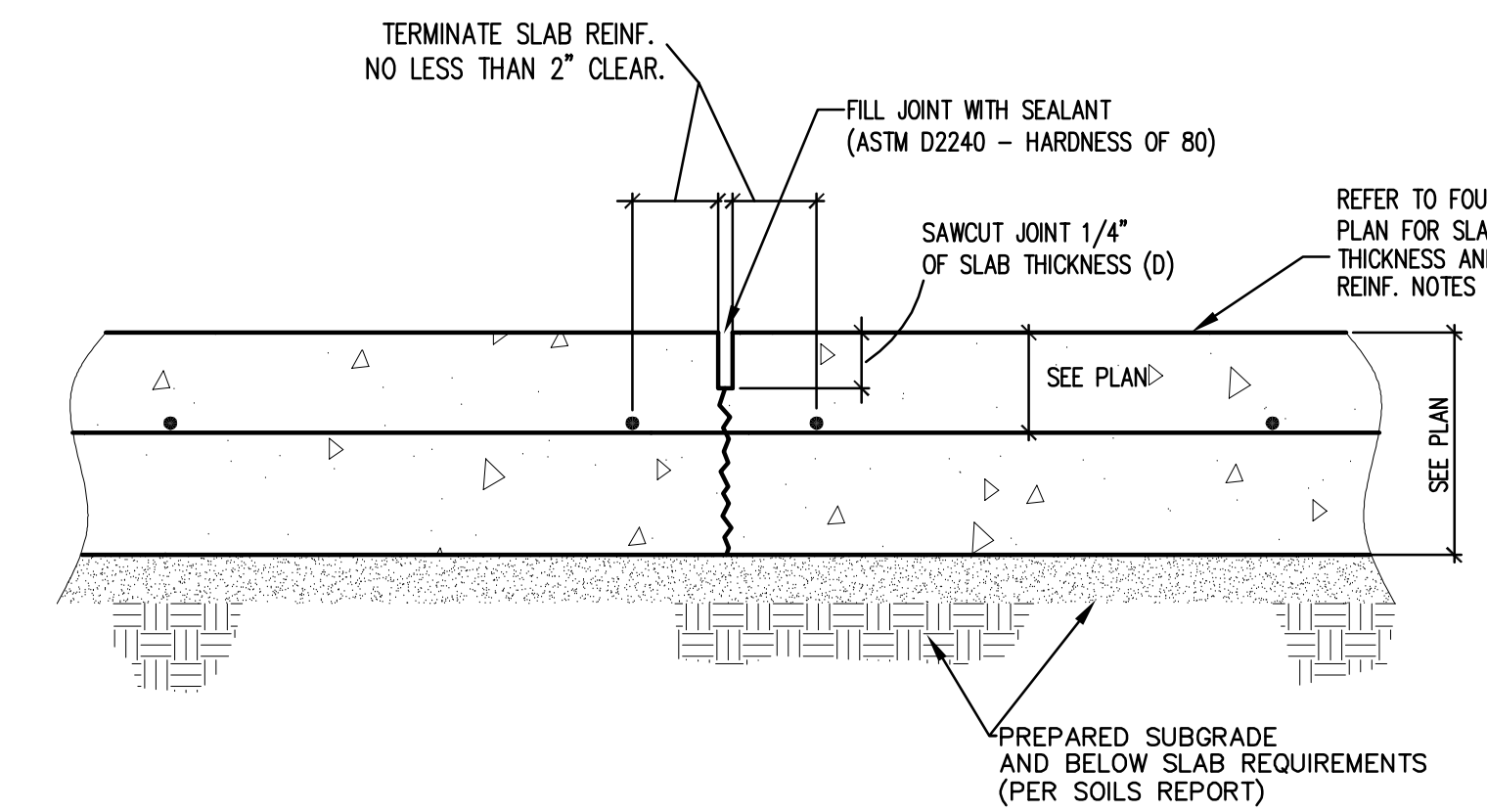
A POPOUT IS A CONICAL FRAGMENT THAT BREAKS OUT OF A CONCRETE SURFACE, LEAVING A HOLE. THE HOLE VARIES IN SIZE FROM 5 MM TO 50 MM (1/4 IN TO 2 IN), THOUGH LARGER POPOUTS ARE POSSIBLE. USUALLY, A FRACTURED AGGREGATE PARTICLE IS LOCATED AT THE BOTTOM OF THE HOLE. THE MATCHING PIECE OF THE FRACTURED PARTICLE ADHERES TO THE POINT OF THE POPOUT CONE. POPOUTS ARE CONSIDERED A COSMETIC DETRACTION AND GENERALLY DO NOT AFFECT THE SERVICE LIFE OF THE CONCRETE.



3 CONSTRUCTION JOINT LOCATION MAY ALSO OCCUR @ SAME LOCATION

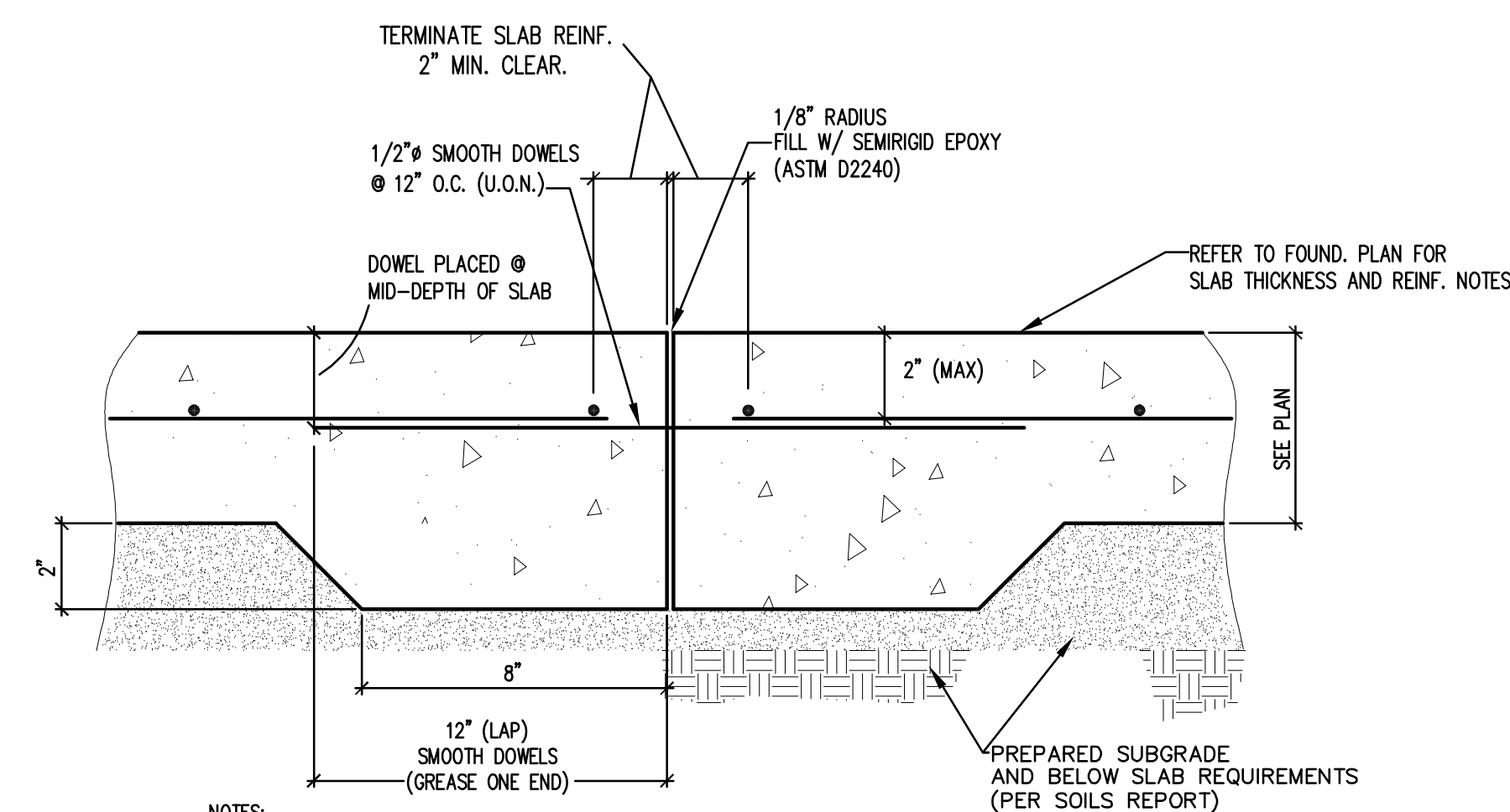
1 TYPICAL JOINT LAYOUT PLAN (REFER TO CONTROL JOINT PLAN FOR ACTUAL LOCATIONS) NTS

THE CONCRETE SLAB-ON-GRADE FOR THIS PROJECT IS A NON-LOAD BEARING ELEMENT AND THE ONLY DESIGN PARAMETERS USED IS FOR JOINT SPACING AND MINIMUM SHRINKAGE REINFORCEMENT. ALL CONCENTRATED OR UNIFORM LOADS IMPOSED ON THE SLAB-ON-GRADE HAVE DESIGNED FOOTINGS FOR THESE SITUATIONS.



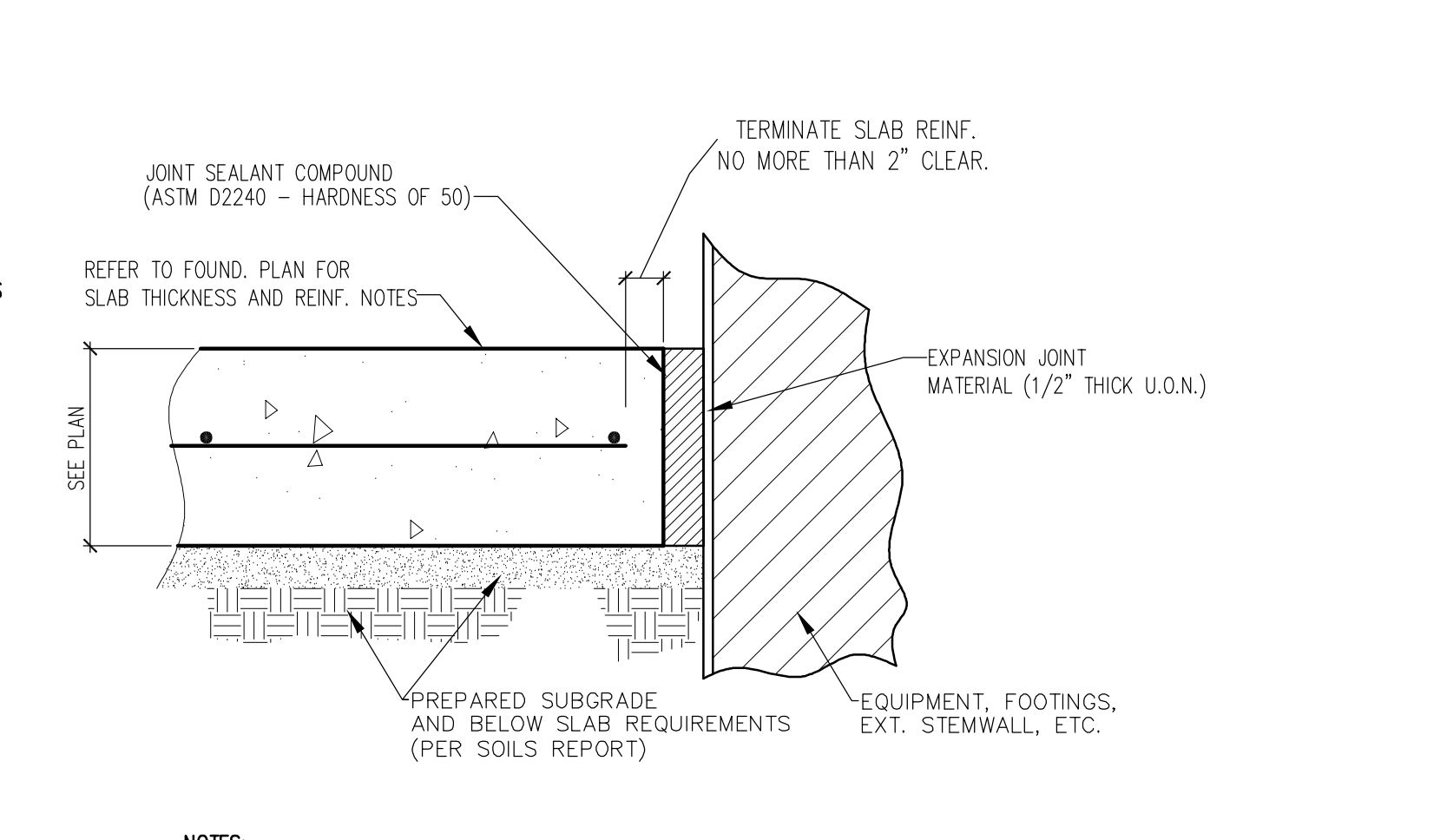
- NOTES:
1. SAWCUT JOINT MUST OCCUR WITHIN 7 HOURS OF CONCRETE POUR.
 2. NOT USED.
 3. SPACING OF JOINTS TO BE 12'-0" O.C. MAX EACH WAY (U.O.N.) ON PLAN.
 4. REFER TO CONTROL JOINT PLAN OR FOUNDATION PLAN FOR JOINT LOCATIONS.
 5. REFER TO DEFINITIONS (THIS SHEET)

2 CONTROL / CONTRACTION JOINT SECTION NTS



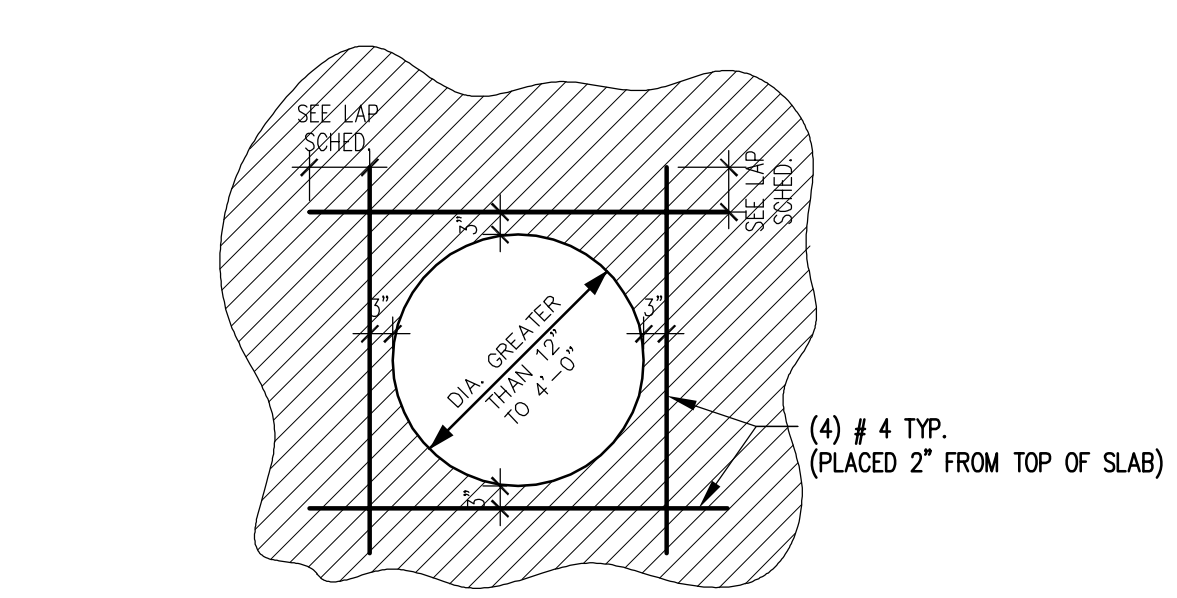
- NOTES:
1. CONSTRUCTION JOINTS TO MATCH CONTROL JOINT LOCATIONS. IF NOT POSSIBLE AND LOCATED IN PANEL AREA, CONTINUE ALL REINFORCING THRU JOINT.
 2. REFER TO DEFINITIONS (THIS SHEET)

1 CONSTRUCTION JOINT SECTION (TO OCCUR @ CONTROL JOINT LOCATION) NTS

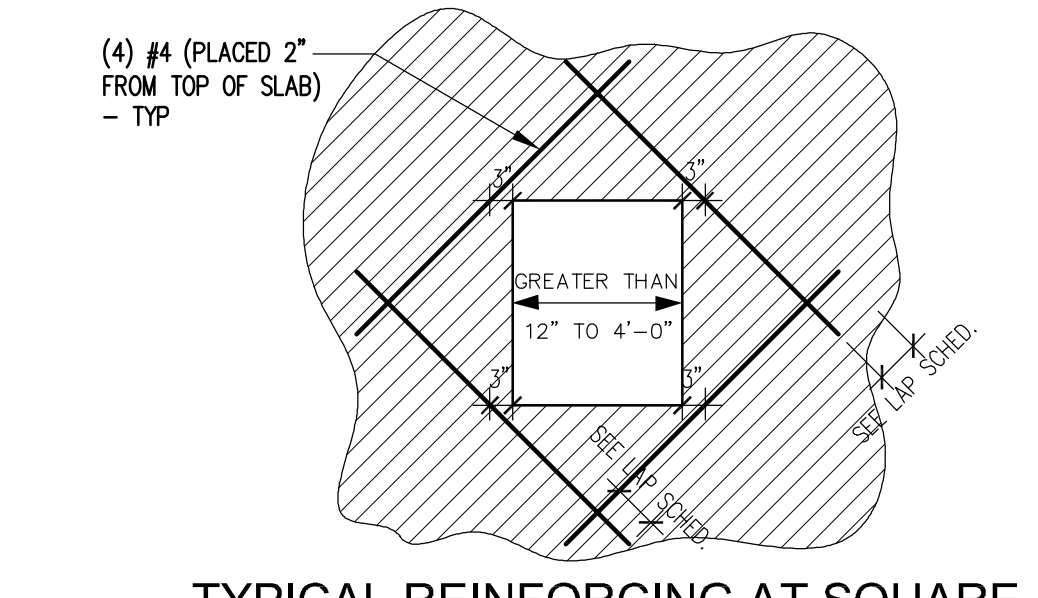


- NOTES:
1. EXPANSION JOINT MATERIAL TO BE MINIMUM 1/2" THICK (U.O.N.)
 2. REFER TO DEFINITIONS (THIS SHEET)

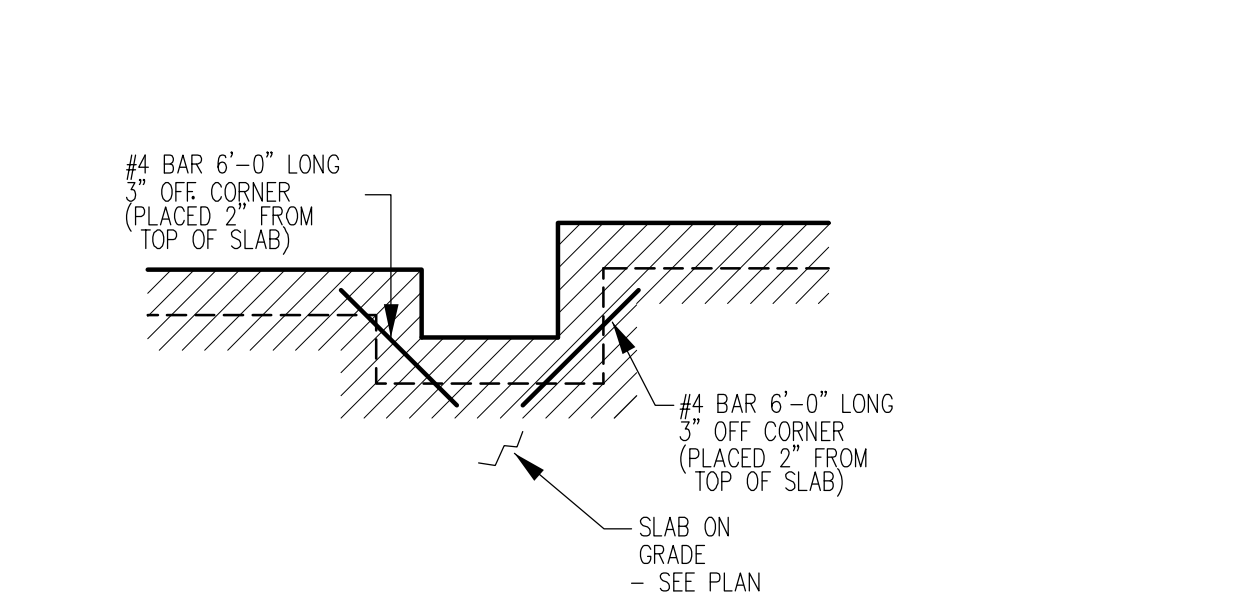
3 ISOLATION / EXPANSION JOINT SECTION NTS



4 TYPICAL REINFORCING AT ROUND OPENINGS AT CONCRETE SLAB-ON-GRADE NTS



4A TYPICAL REINFORCING AT SQUARE OR RECTANGULAR OPENINGS AT CONCRETE SLAB-ON-GRADE NTS



5 TYPICAL REINFORCING @ RE-ENTRANT CORNERS NTS

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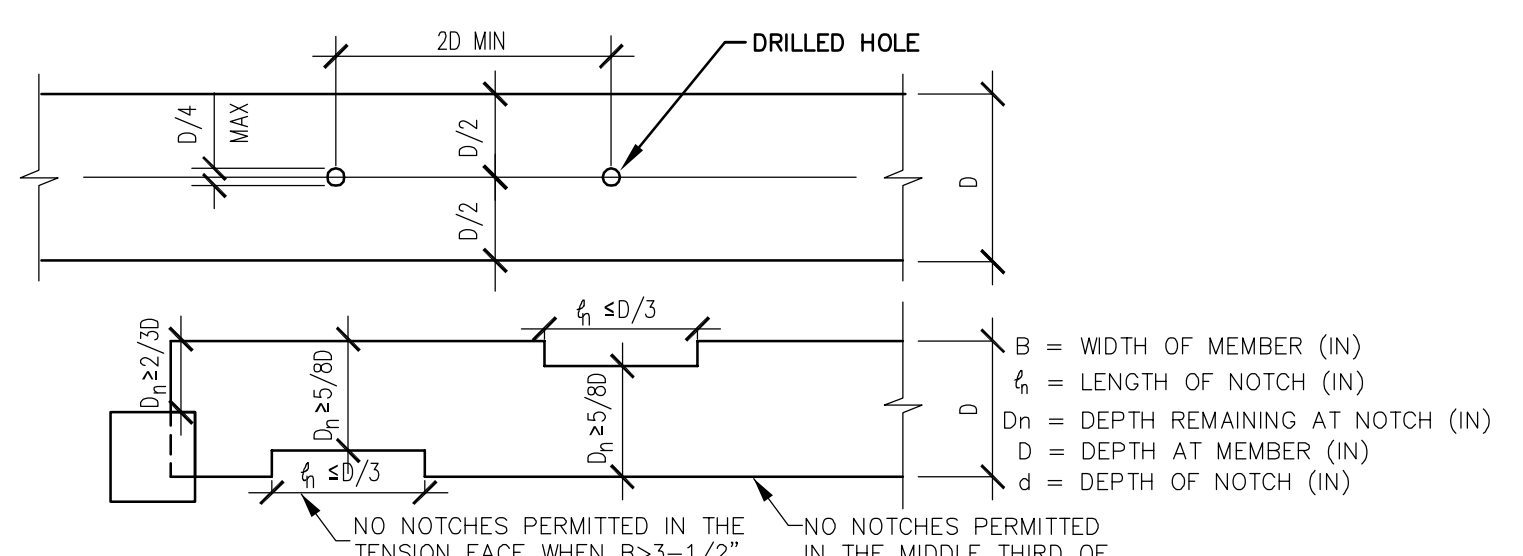
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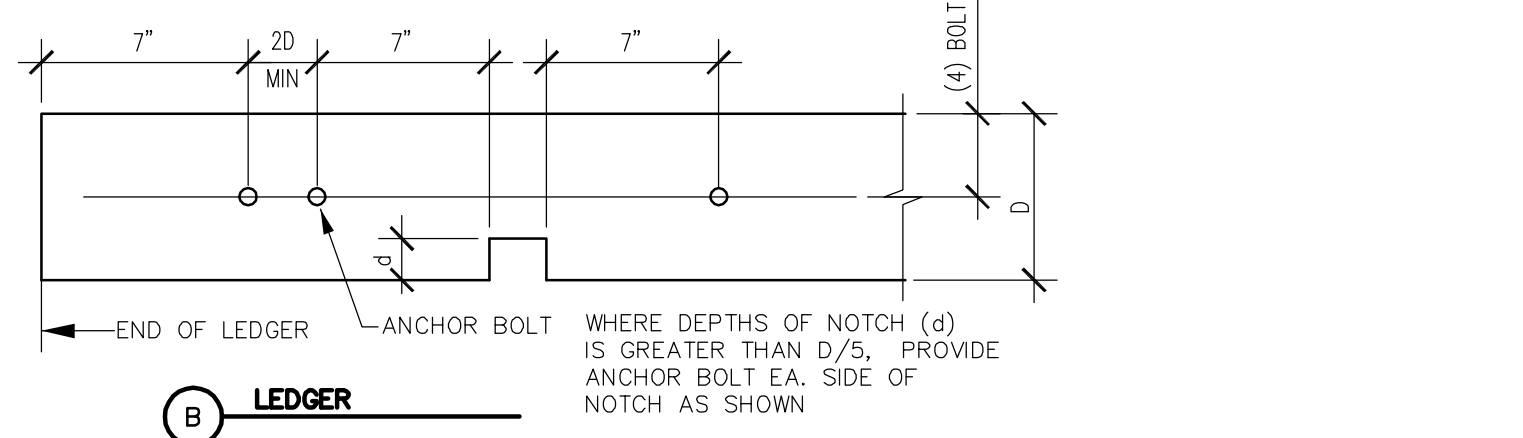
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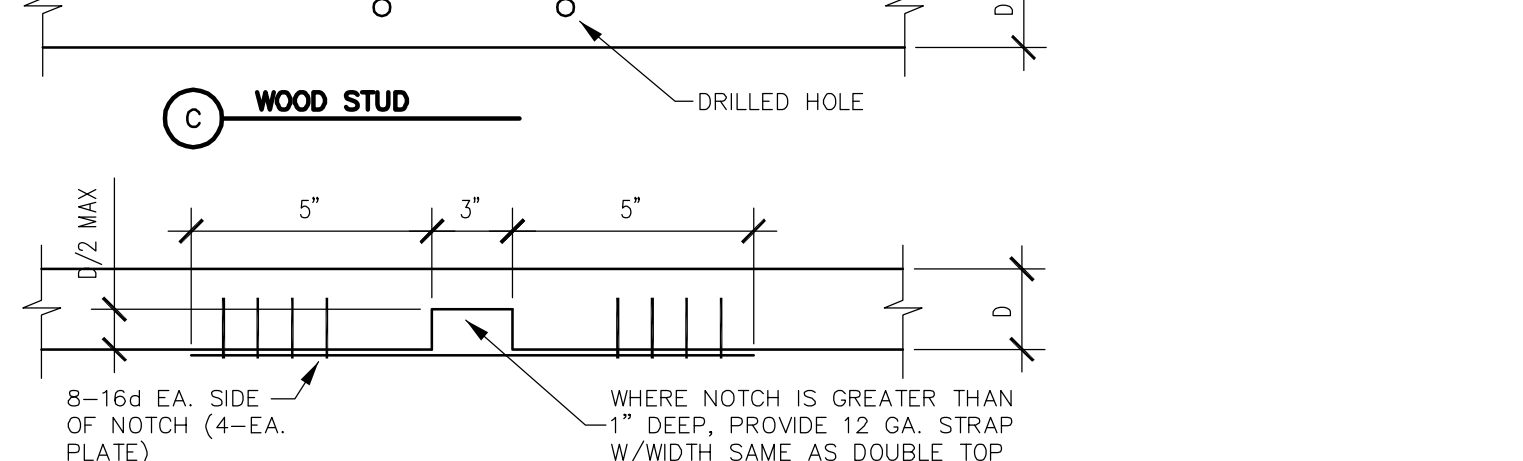
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 Drawing Name: S0.2A



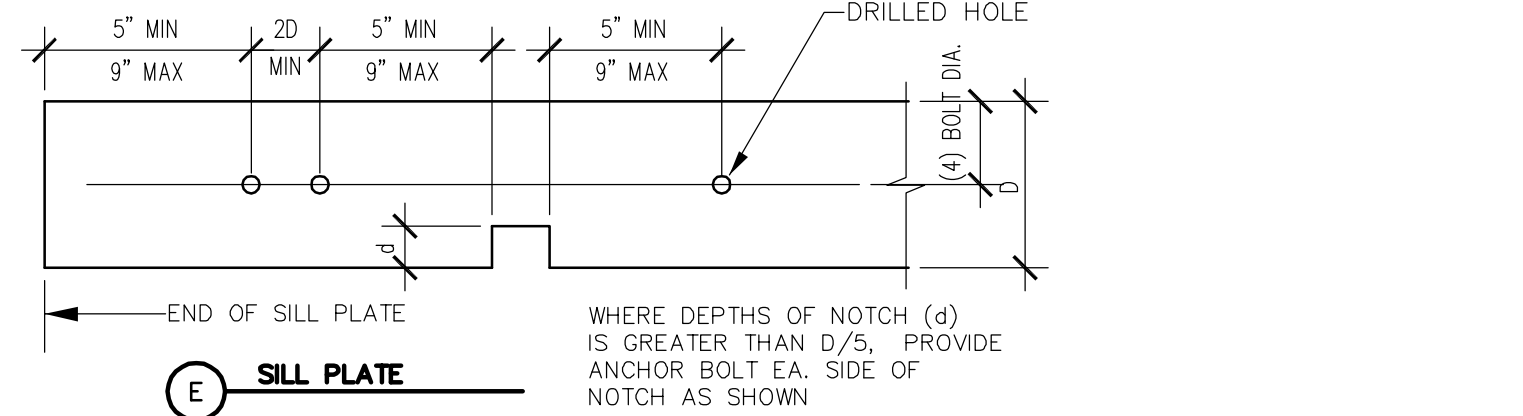
1 HOLES AND NOTCHES NTS



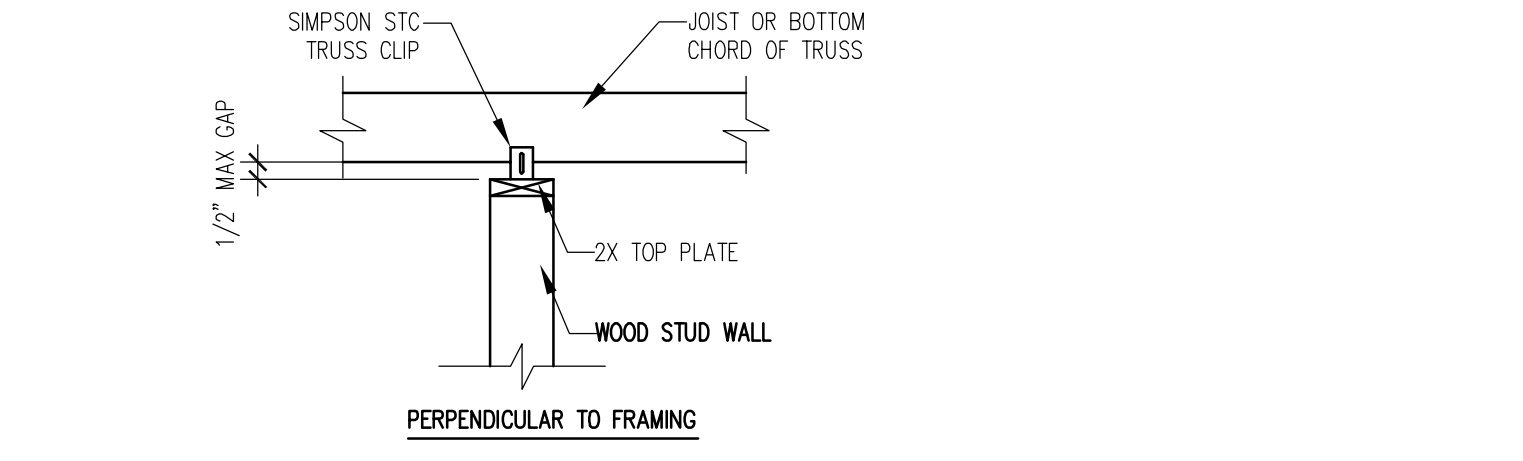
2 JOIST AND RAFTER LAP @ SUPPORTS NTS



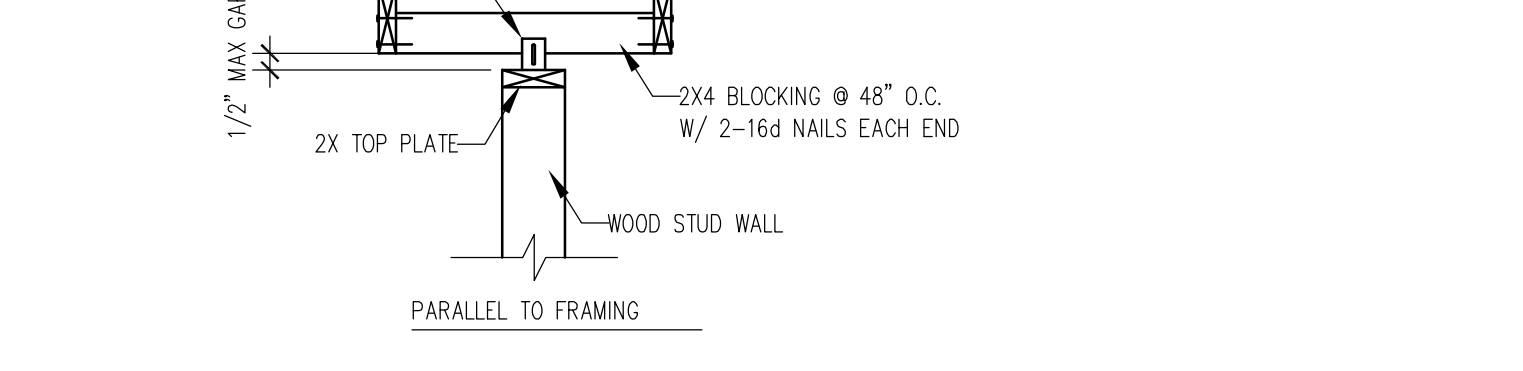
3 WOOD STUD NTS



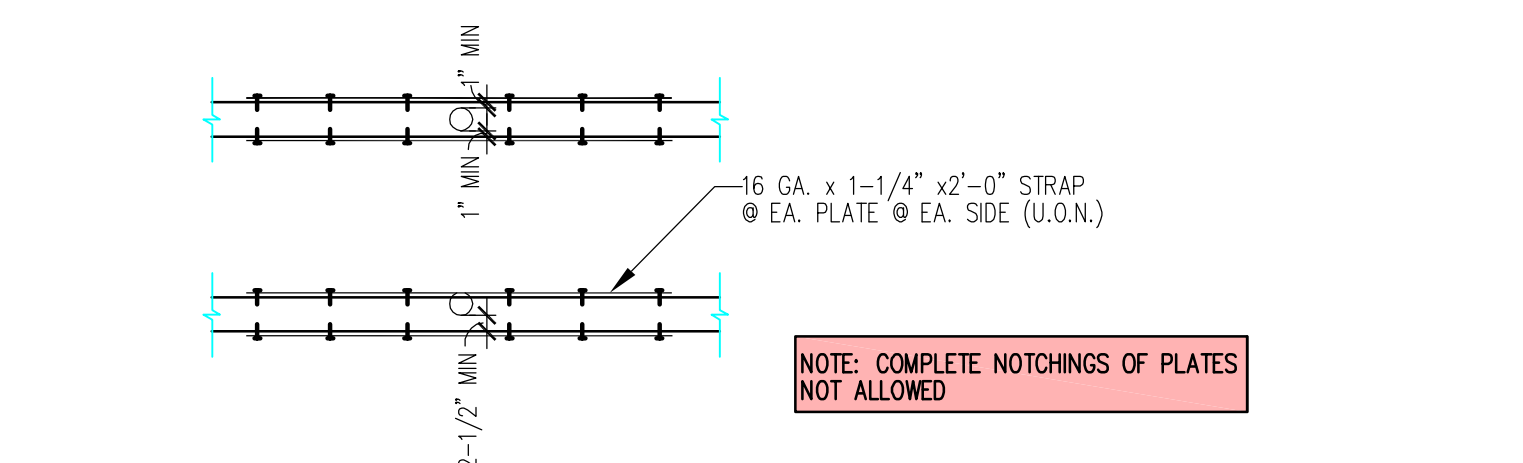
4 DOUBLE TOP PLATE NTS



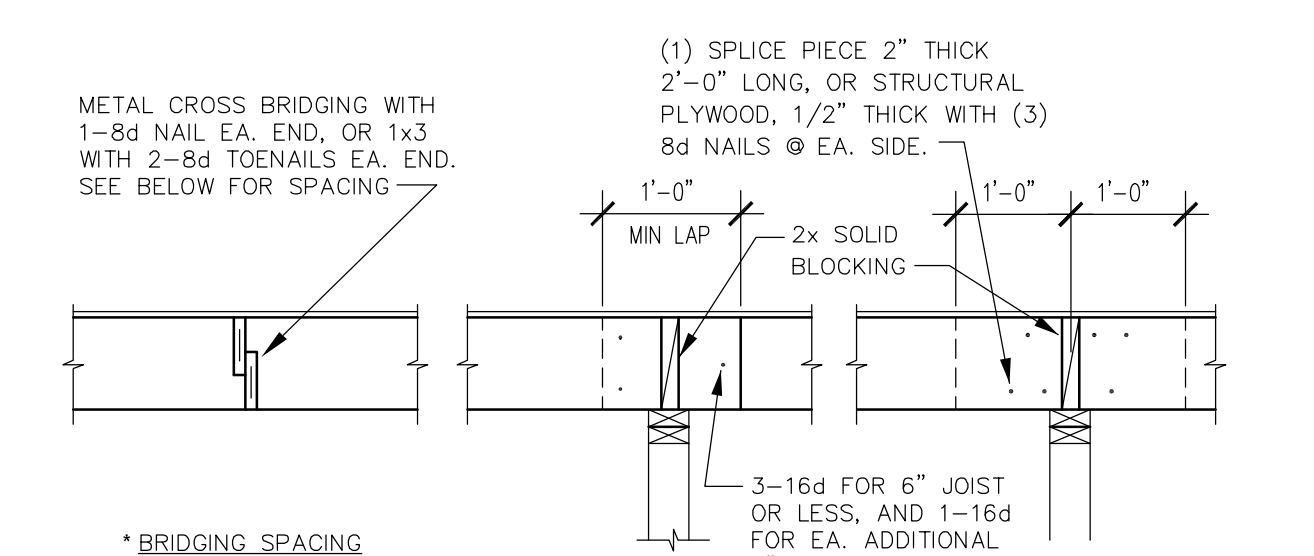
5 SILL PLATE NTS



6 WOOD STUDS @ NON-LOAD BEARING WALLS NTS



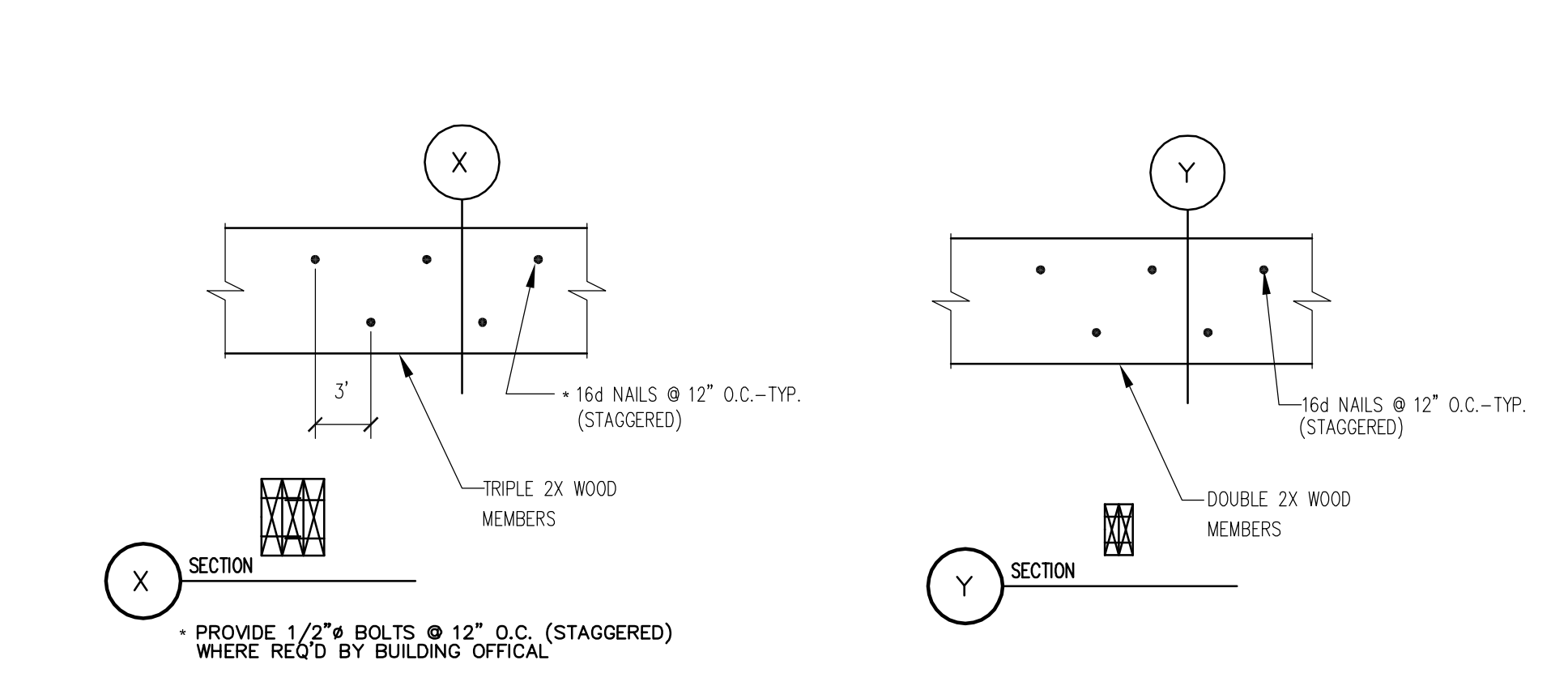
7 HOLE THROUGH PLATES @ SHEAR/BEARING WALL NTS



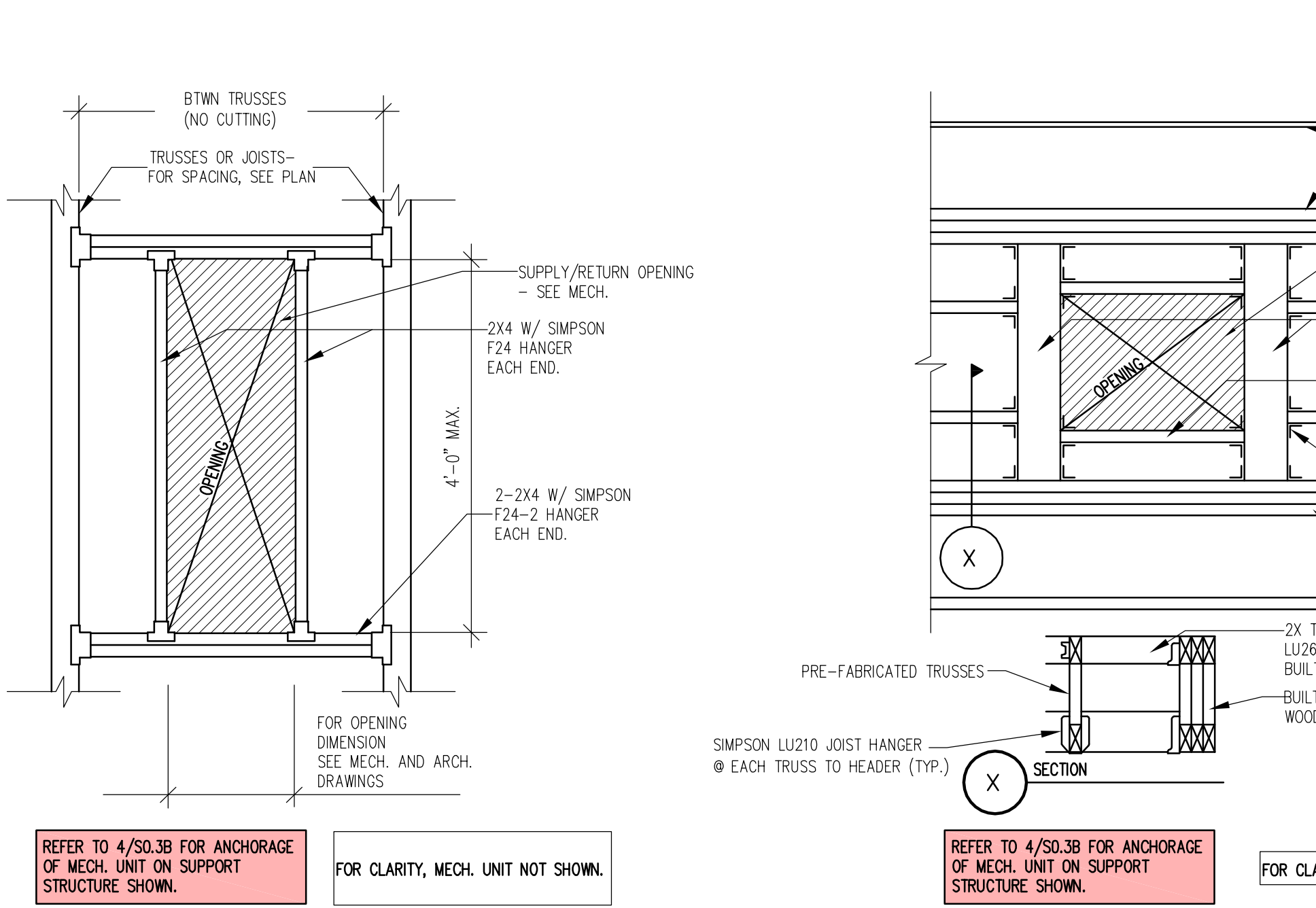
8 TYP. CROSS BRIDGING NTS



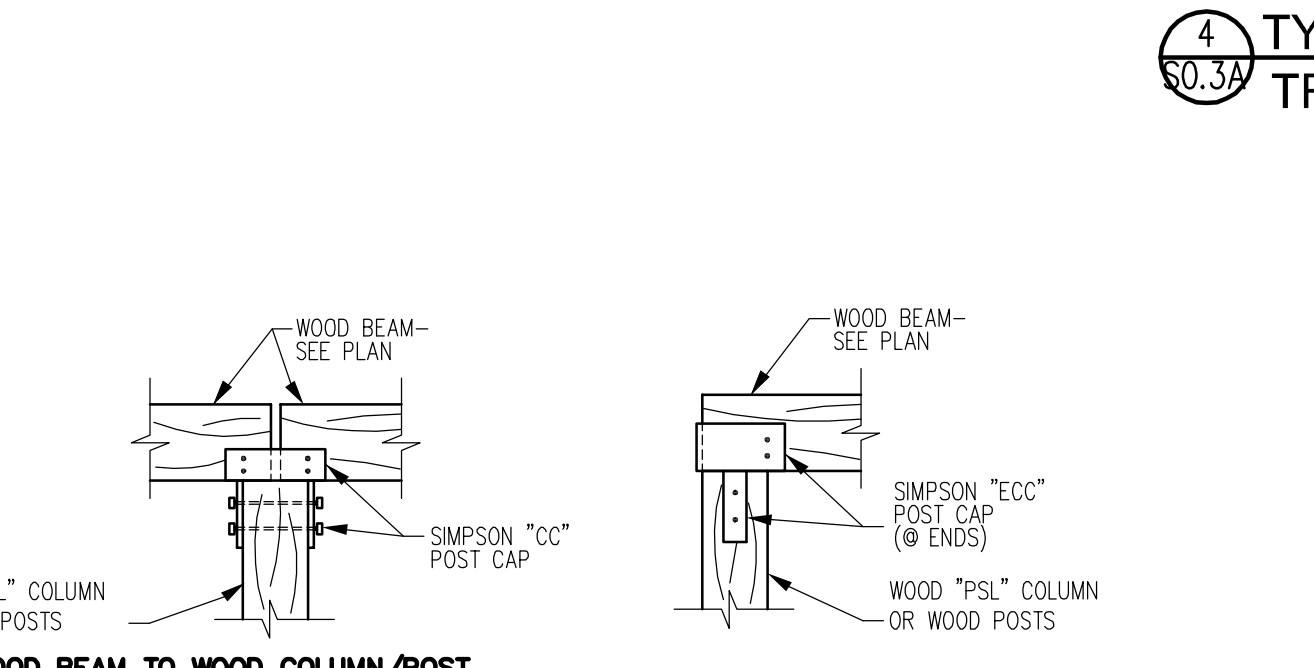
9 JOIST AND RAFTER LAP @ JOISTS NTS



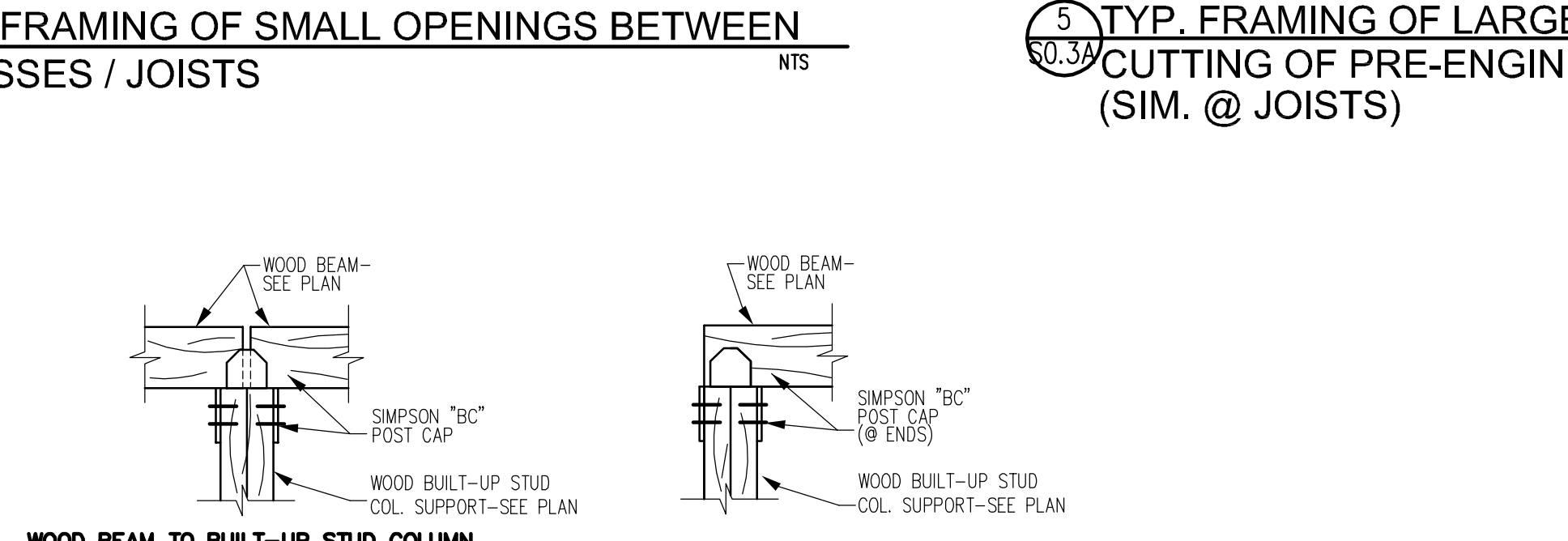
10 TYP. BUILT-UP WOOD MEMBERS (BUILT-UP STUD COLUMNS OR HEADERS) NTS



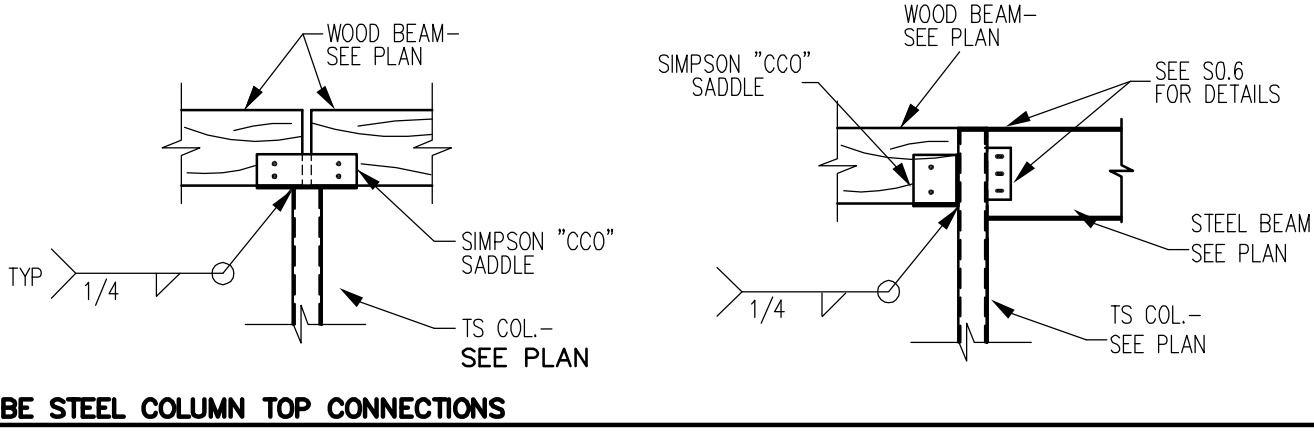
11 TYP. FRAMING OF LARGE OPENINGS CAUSING CUTTING OF PRE-ENGINEERED TRUSSES (SIM. @ JOISTS) NTS



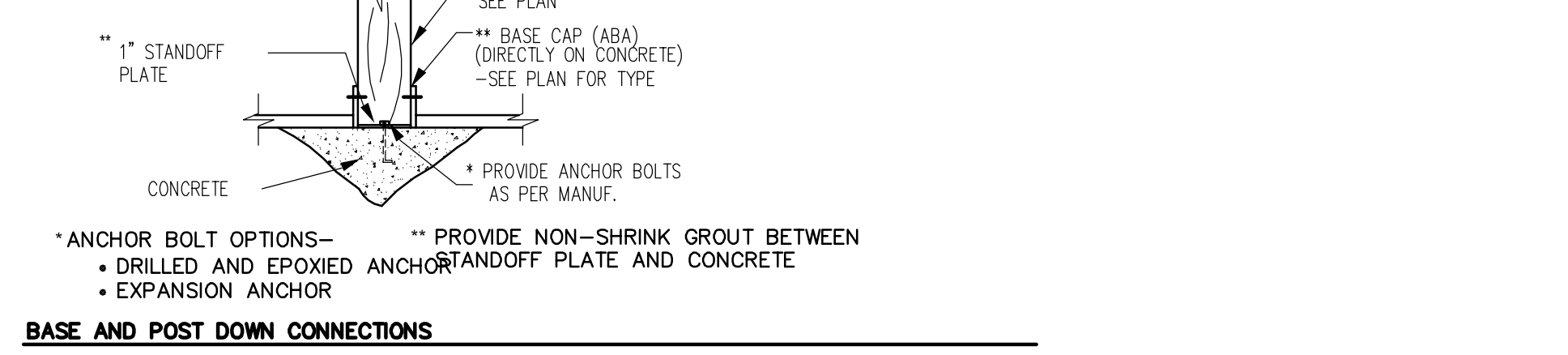
12 WOOD BEAM TO WOOD COLUMN/POST NTS



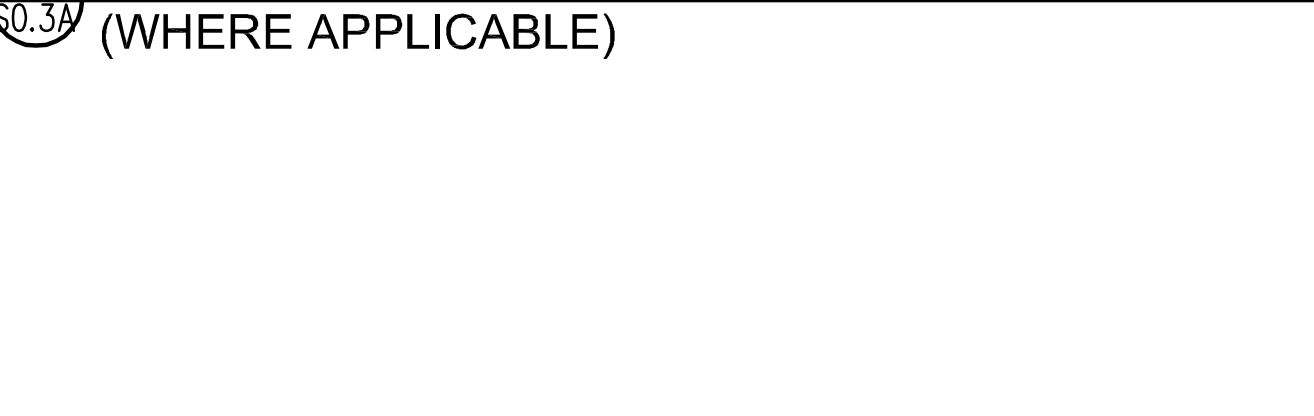
13 WOOD BEAM TO BUILT-UP STUD COLUMN NTS



14 TUBE STEEL COLUMN TOP CONNECTIONS NTS



15 BASE AND POST DOWN CONNECTIONS NTS



16 TYP. WOOD COLUMN POST CAP/BASE CONNECTION CONDITIONS (WHERE APPLICABLE) NTS

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TIMBER GENERAL DETAILS

REV#	DATE	COMMENTS

REVIEWS

INITIALS	BDA DSGN. REV.
	BDA TECH REV.

CCAS
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DRAWING:
DATE: 7/23/15

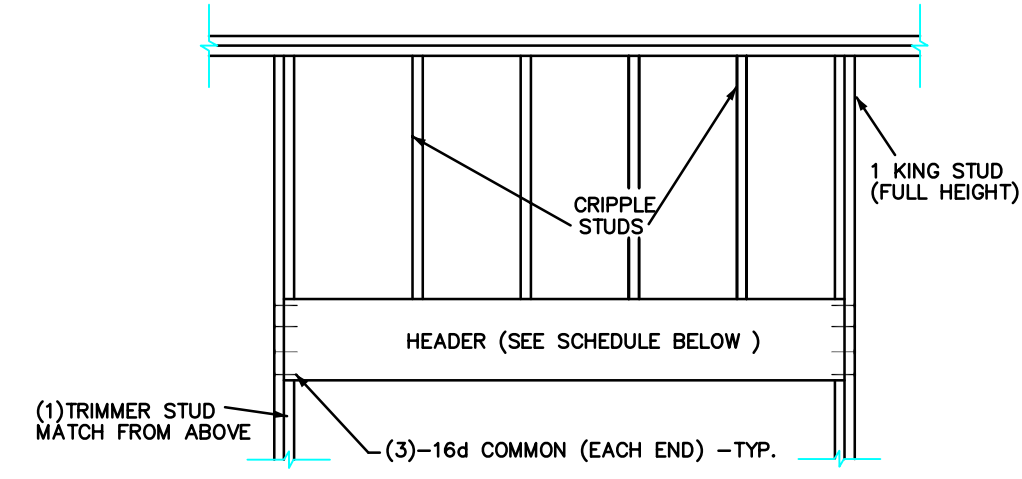
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HEADER SCHEDULE	
OPENING SIZE	HEADER SIZE
0'-4"	(2)-2X FLAT
4'-1" - 8'-0"	(2)-2X8
8'-1" - 12'-0"	(2)-2X12 HF#2

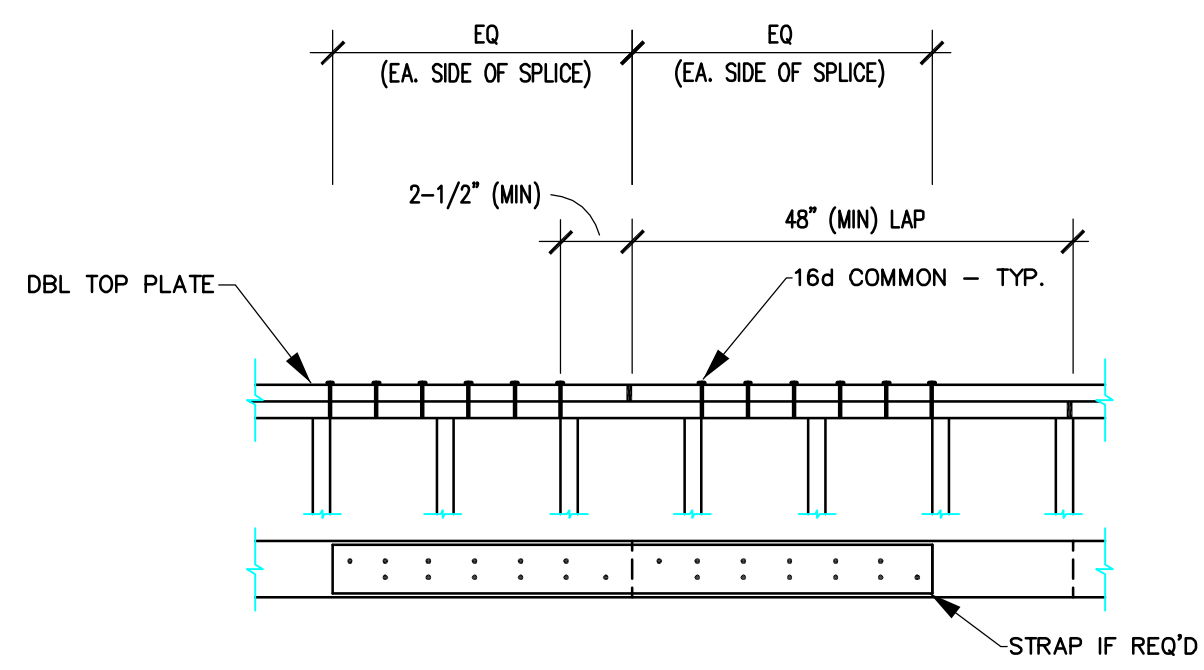
(DF#2, HF#2, OR SPF#2 MAY BE USED)

1 TYP. NON-LOAD BEARING HEADER NTS

TYPICAL CEILING JOIST SCHEDULE				
JOIST SIZE	MAXIMUM SPAN	BRIDGING SPACING	LEDGER AT MASONRY WALL	LEDGER AT STUD WALL
2 X 4	8'-0"	2'-9" O.C.	N/A	2X4 W/ 2-16d NAILS EA. STUD
2 X 8	14'-0"	4'-8" O.C.	N/A	2X8 W/ 3-16d NAILS EA. STUD
2 X 10	18'-0"	6'-0" O.C.	N/A	2X10 W/ 3-16d NAILS EA. STUD

NOTES:
 1. SPACE CEILING JOISTS @ 24" O.C. MAX.
 2. HANG CEILING JOISTS FROM LEDGERS W/ SIMPSON LU FACE MOUNTED JOIST HANGER.
 3. PROVIDE CONTINUOUS FLAT 2X4 BRIDGING W/ 1-16d NAIL EACH JOIST AT THE TOP OF THE JOISTS.
 4. IF JOIST SPANS ARE GREATER THAN THOSE SHOWN ABOVE, NOTIFY THE STRUCTURAL ENGINEER.
 5. JOISTS ARE DESIGNED FOR 5 PSF DEAD LOAD AND 10 PSF LIVE LOAD. IF ACTUAL LOADS ARE GREATER, NOTIFY THE STRUCTURAL ENGINEER.
 6. ALL MATERIAL TO BE DF#2 OR SPF#2

2 TYP. CEILING JOIST SCHEDULE NTS



TYPE	16d BTWN BUTTS	STRAP TIE CTR. W/BUTT	CAPACITY (LBS)
A	(2) ROWS - 10 NAILS/ROW 20 - TOTAL/SIDE	N/A	0 ≤ 7,500
B	(32) 16d/SIDE	CMST-16	7,500 ≤ 12,000 (MAX)

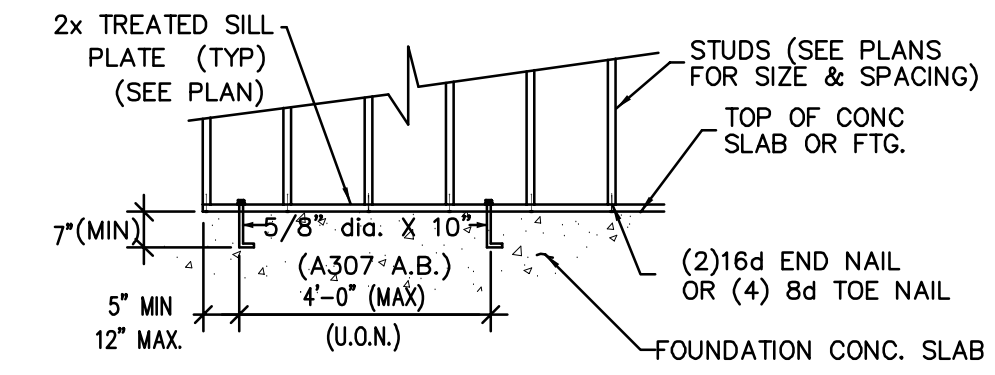
REFER TO FRAMING PLAN GENERAL NOTES FOR TYPE.

NOTES:
 • ALL NAILS TO BE 16d (COMMON)

3 TYP. DOUBLE TOP PLATE SPLICE NTS

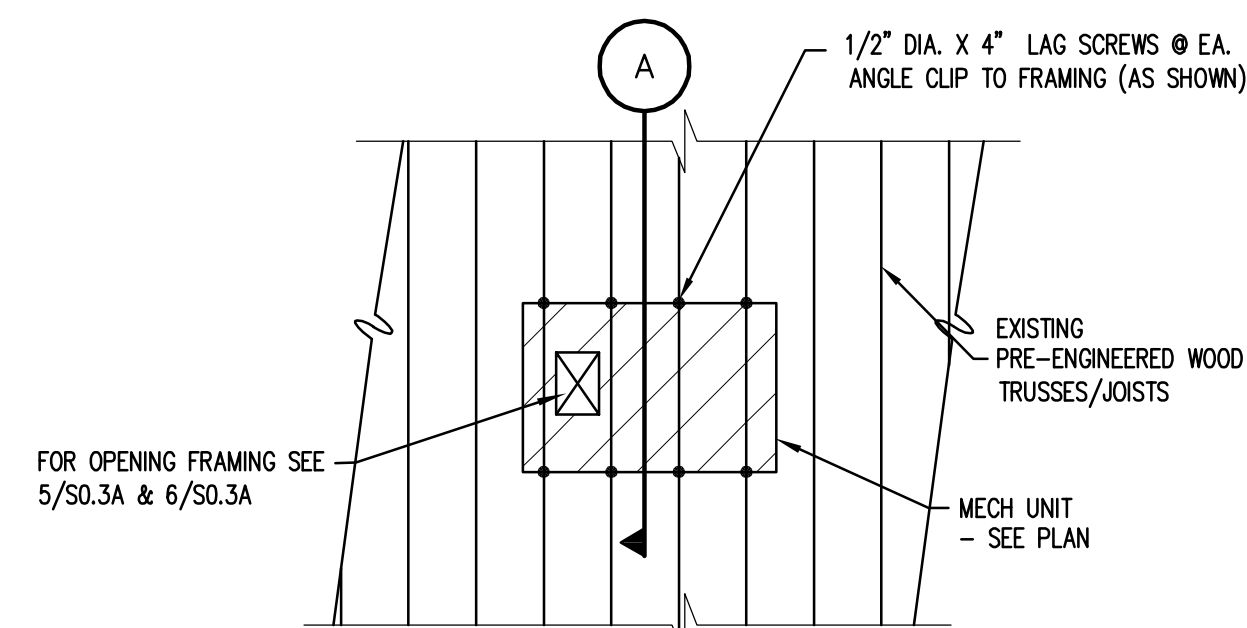
WASHER SCHEDULE			
BOLT	MALLEABLE IRON WASHER	HEAVY PLATE WASHER	STANDARD CUT WASHER
1/2"	2-1/2" X 1/4"	3"X3"X5/16"	1-3/8" X 3/32"
5/8"	2-3/4" X 5/16"	3"X3"X1/4"	1-3/4" X 1/8"
3/4"	3" X 7/16"	3"X3"X5/16"	2"X5/32"
7/8"	3-1/2" X 7/16"	3"X3"X5/16"	2-1/4"X11/64"
1"	4" X 1/2"	3-1/2"X3-1/2"X5/16"	2-1/4"X11/64"

NOTES:
 1. USE STANDARD CUT WASHER FOR ALL BOLTS, U.O.N.
 2. HEAVY PLATE WASHERS ARE REQUIRED @ SILL PLATE TO FOUNDATION AT ALL SHEARWALLS.
 3. HEAVY PLATE WASHERS ARE REQUIRED @ HOLDOWNS

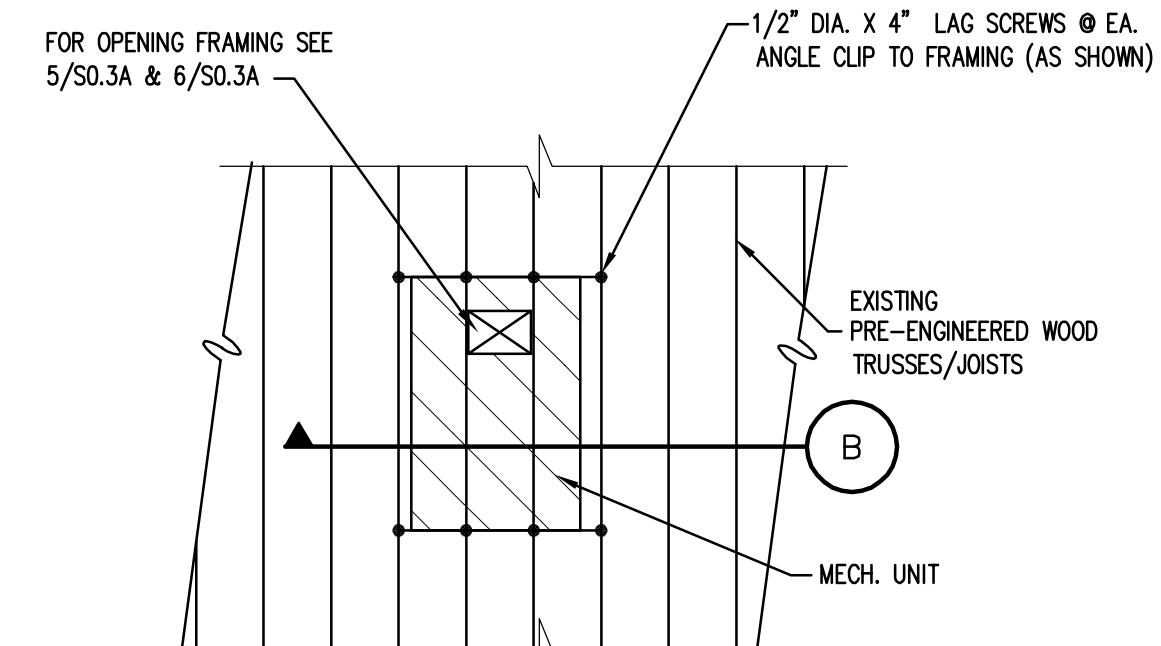


NOTE:
 1. PROVIDE (2) BOLTS MIN PER SILL PIECE.
 2. SEE PLANS FOR SIZE AND SPACING OF ANCHOR BOLTS AT SHEARWALLS.
 3. PROVIDE ANCHOR BOLTS AT ALL PERIMETER WALLS AND SHEARWALLS.
 4. PROVIDE WASHERS @ EA. ANCHOR BOLT. - SEE SCHEDULE THIS SHEET

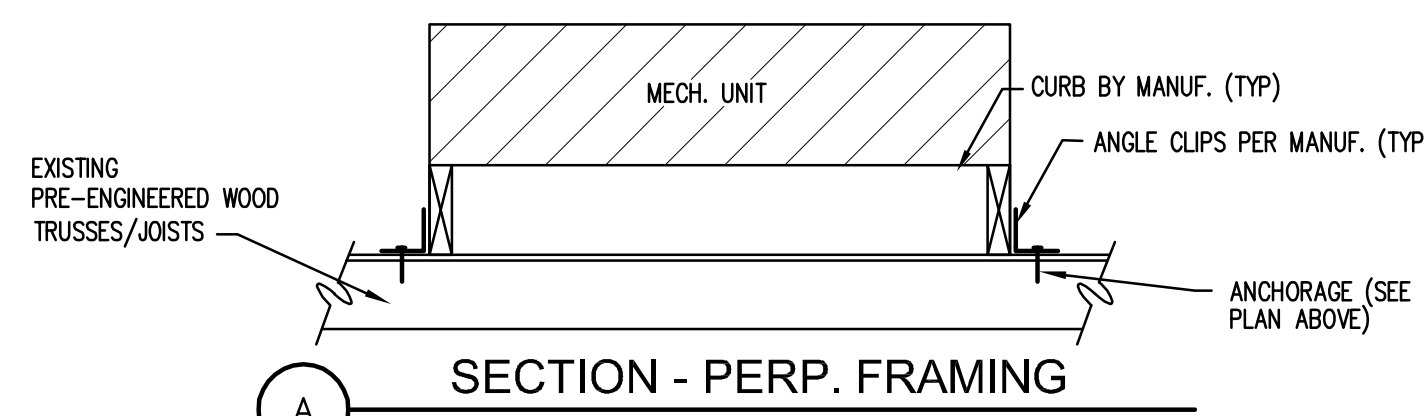
4 WASHER SCHEDULE NTS



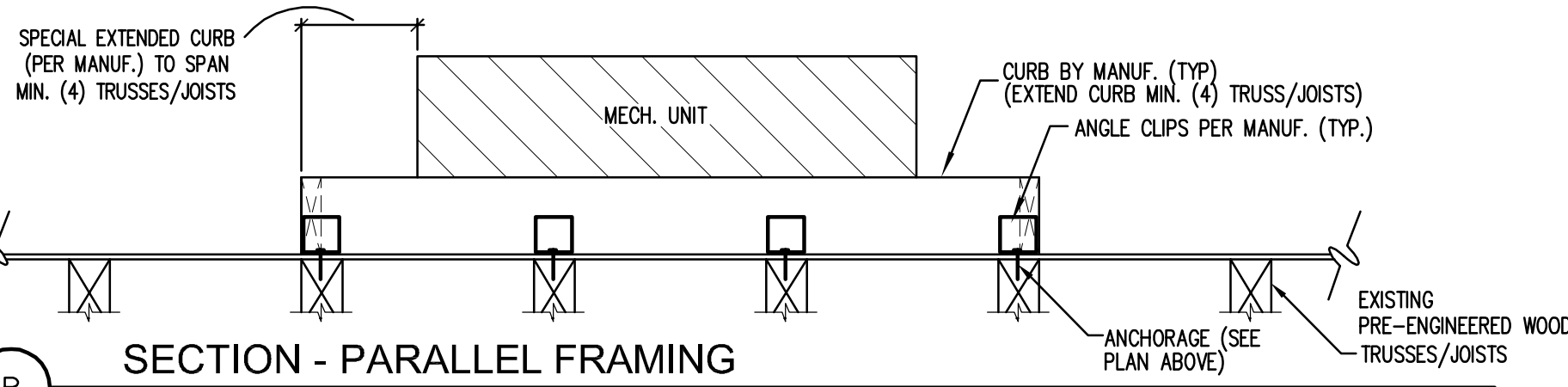
PLAN - PERP. FRAMING



PLAN - PARALLEL FRAMING



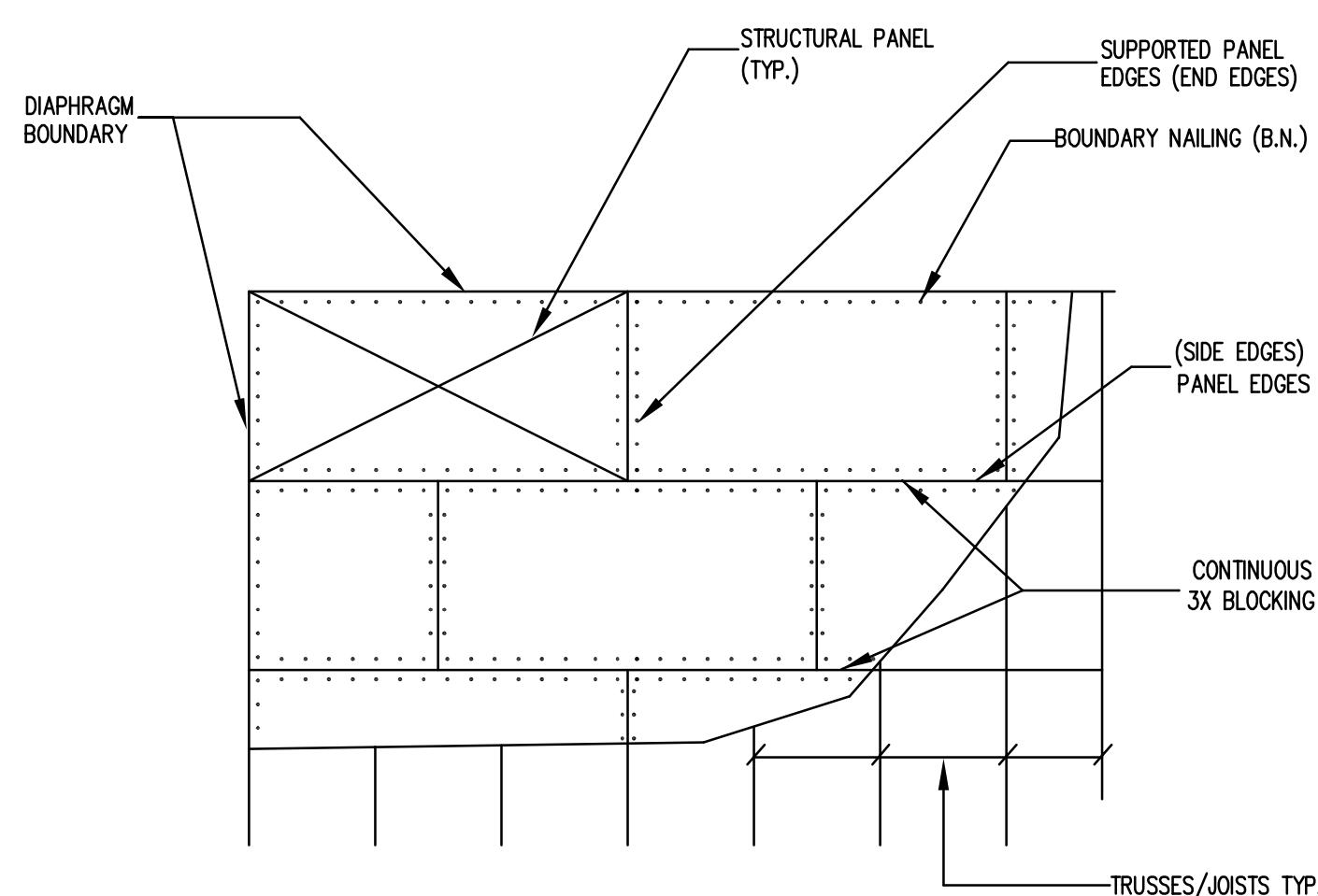
SECTION - PERP. FRAMING



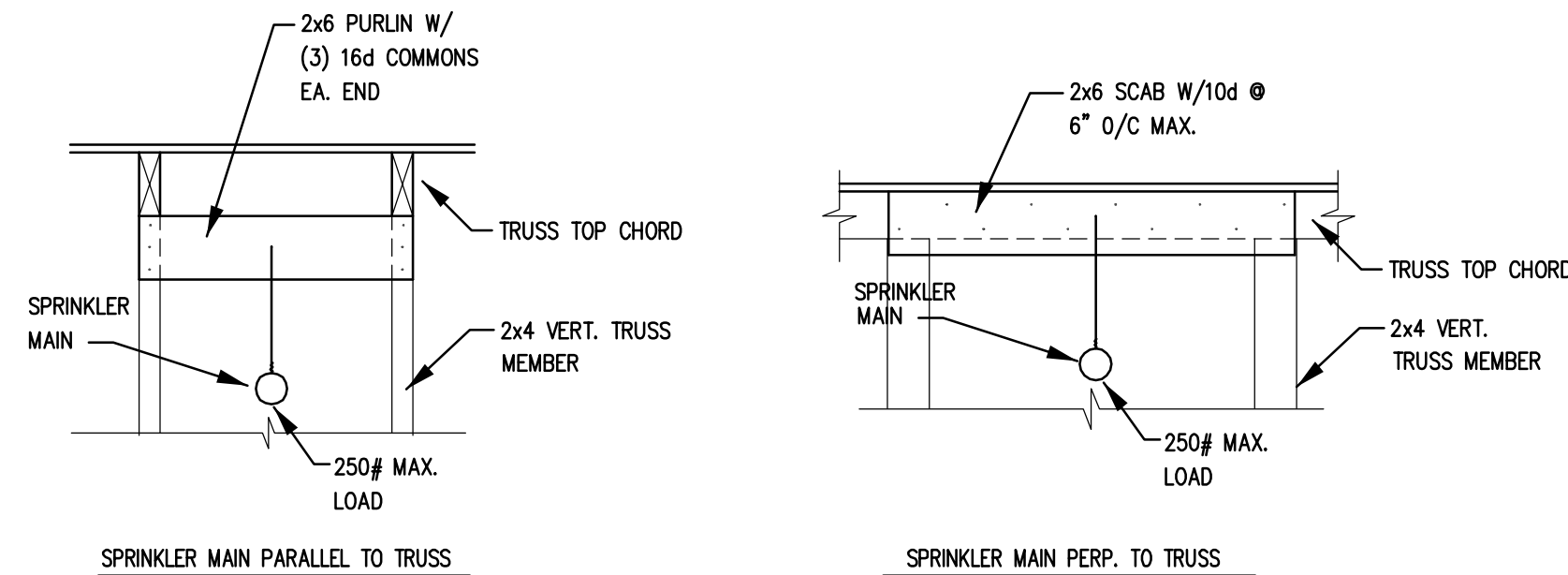
SECTION - PARALLEL FRAMING

5 MECH. EQUIP. SUPPORT / ANCHORAGE NTS

6 TYP. UNBLOCKED DIAPHRAGM *****PLYWOOD CLIPS ACCEPTABLE AT EDGES***** NTS



7 BLOCKED DIAPHRAGM - SEE SHEARWALL PLAN WHERE OCCURS NTS

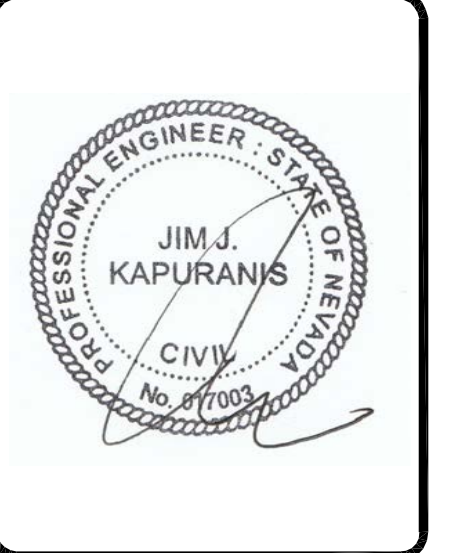


8 SPRINKLER MAIN SUPPORT DETAILS

T&G OR PLYWOOD CLIPS ARE NOT A SUBSTITUTION FOR BLOCKED DIAPHRAGM.

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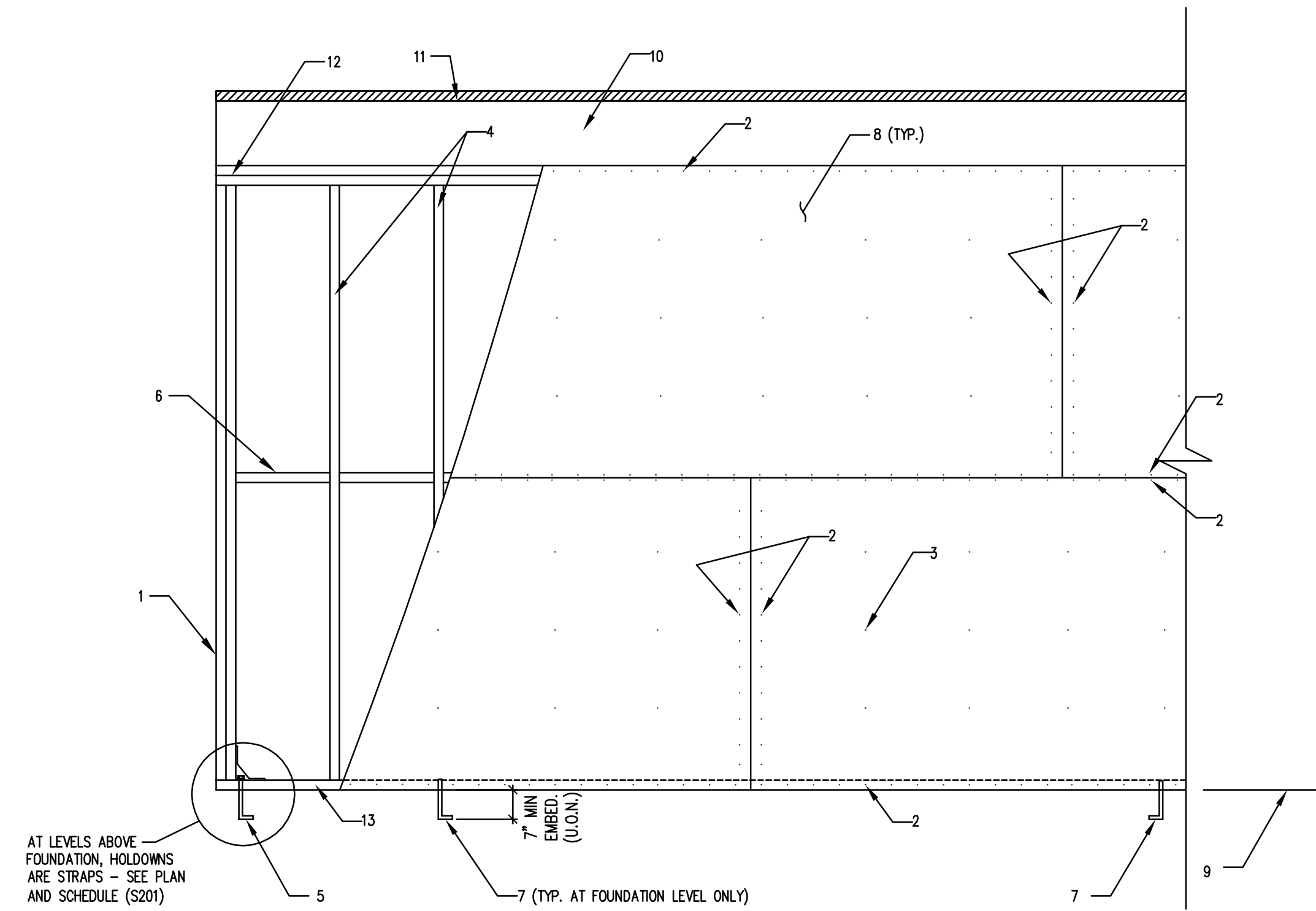
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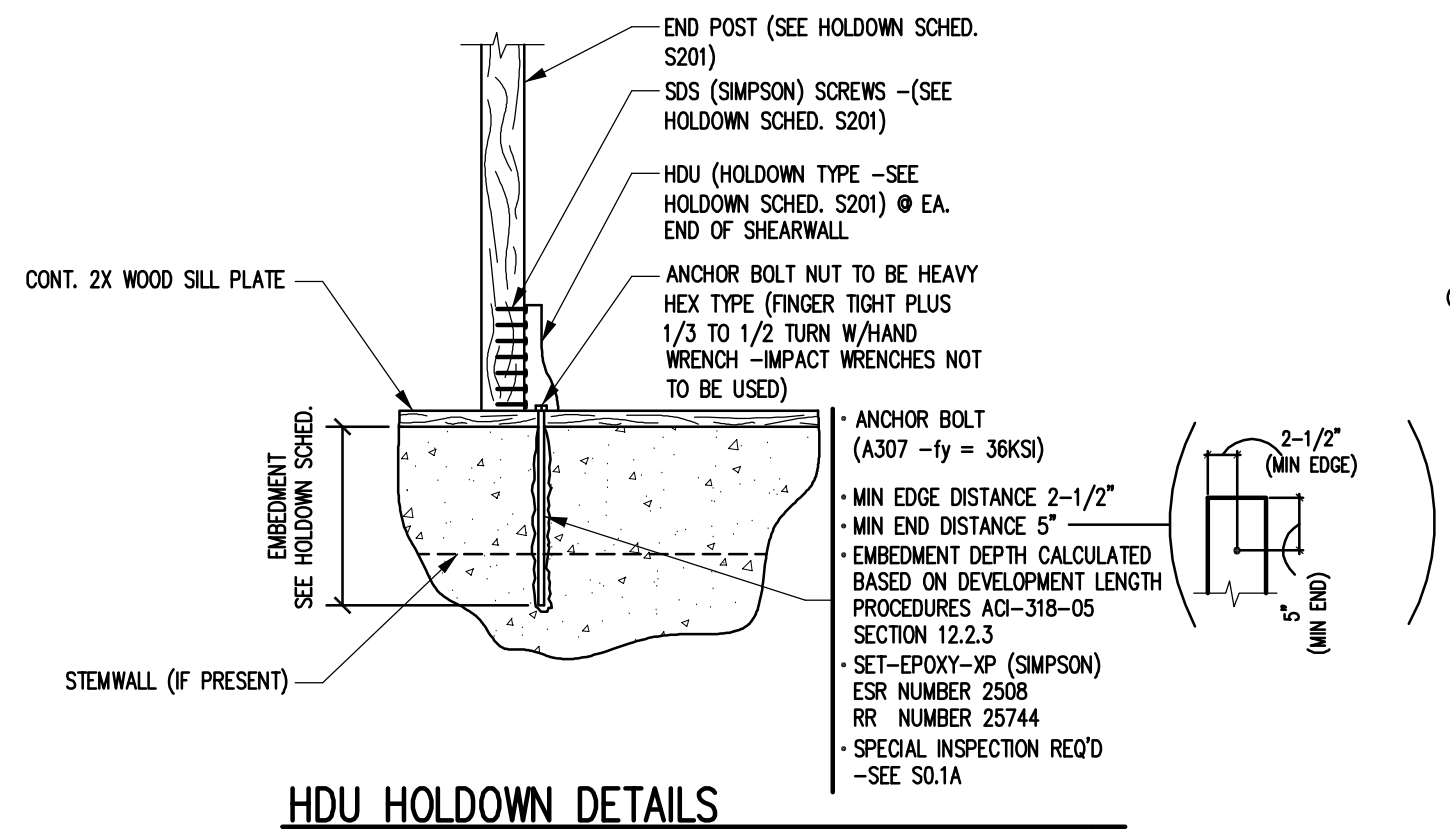
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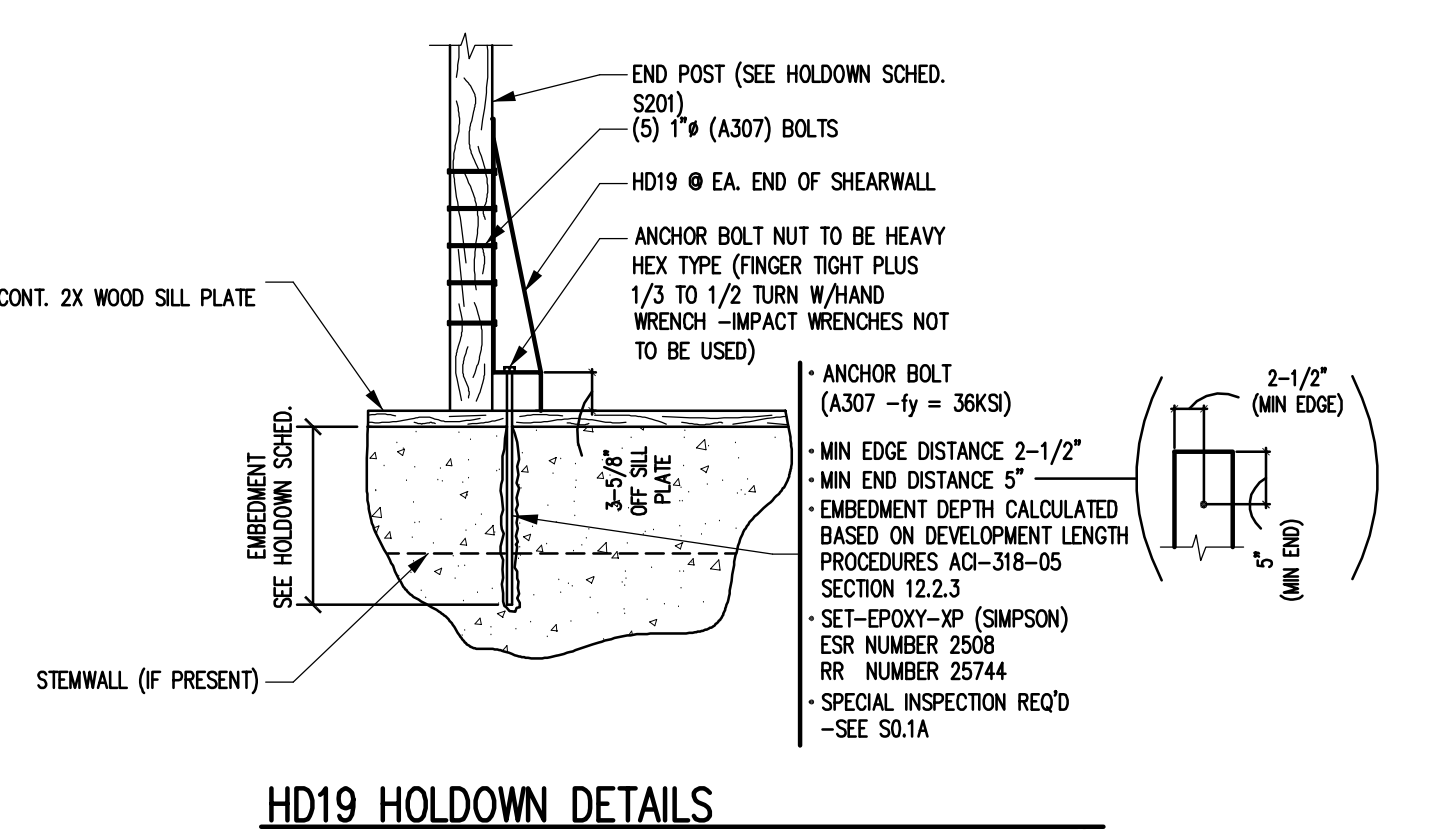
- NOTES**
- DOUBLE STUDS AT EACH END OF PANEL - TYPICAL U.O.N. ON HOLDOWN SCHED. (S201)
 - E.N. (EDGE NAILING) - SEE SHEARWALL SCHED. (S201)
 - FIELD NAILING @ 12" O.C.
 - WOOD STUDS @ 16" O.C. MAX.
 - HOLDOWN AT EA. END OF SHEARWALL ONLY - SEE HOLDOWN SCHED. (S201)
 - 2X or 3X BLOCKING AT UNSUPPORTED SHEATHING PANEL JOINTS - SEE SHEARWALL SCHED. (S201)
 - ANCHOR BOLTS OR DRILLED AND EPOXIED - FOR SIZE AND SPACING - SEE SHEARWALL SCHED. (S201)
 - SHEATHING MATERIAL - SEE SHEARWALL SCHED. (S201)
 - FLOOR LINE
 - BLOCKING OR FLOOR/ROOF JOIST OR TRUSS
 - DIAPHRAGM SHEATHING - SEE DIAPHRAGM SCHED. (S201)
 - 2 - 2X TOP PLATES
 - 2X OR 3X BTM. PLATE - IF 3X BTM PLATE IS REQ'D AT LEVELS ABOVE FOUNDATION, PROVIDE FASTENERS W/ PENETRATION OF 2" INTO CONT. BLOCKING BELOW USING SPACING PER SHEARWALL SCHED. (S201)

TYP. SHEARWALL ELEVATION (AWAY FROM OPENINGS)

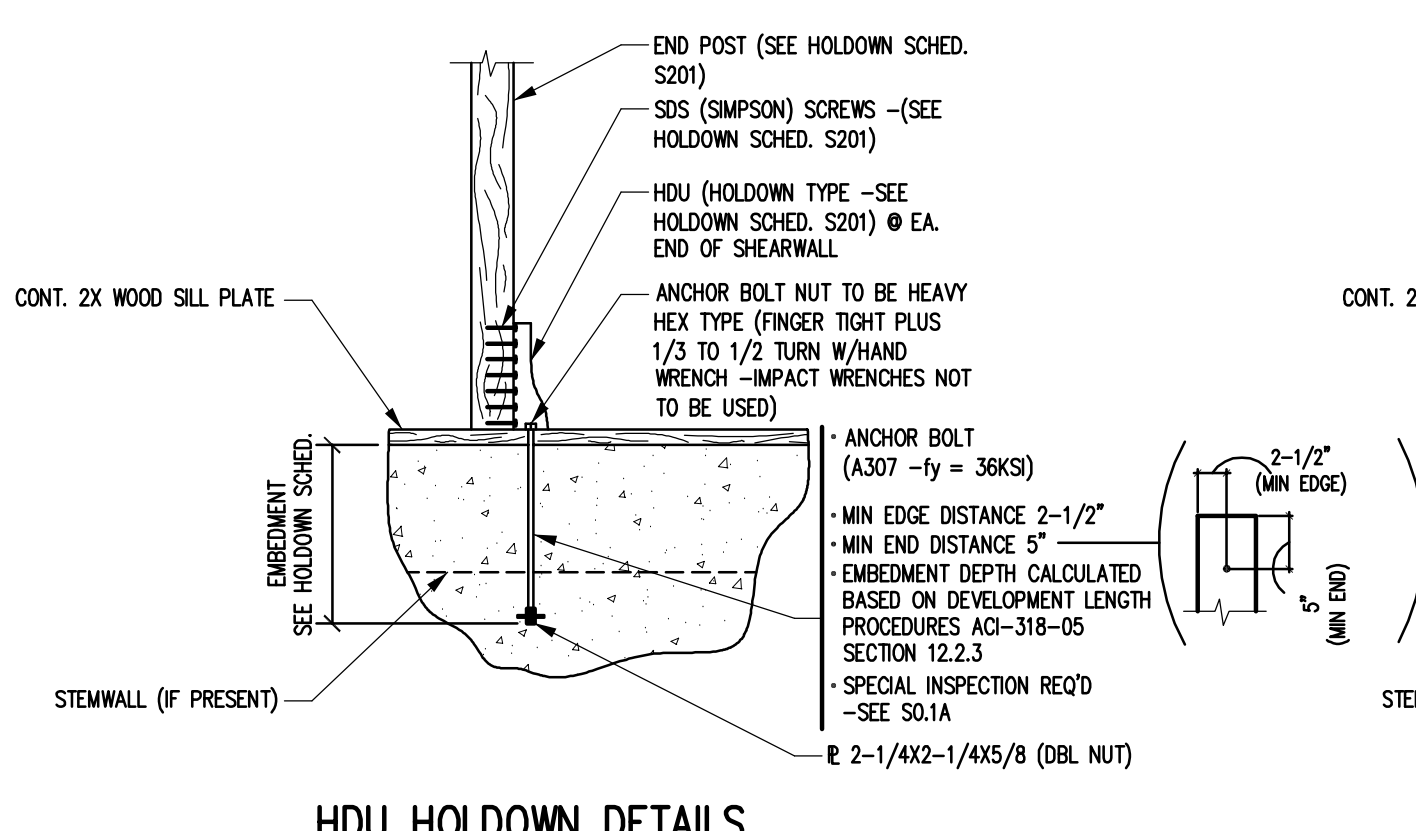
1 TYPICAL SHEARWALL ELEVATIONS AND NOTINGS



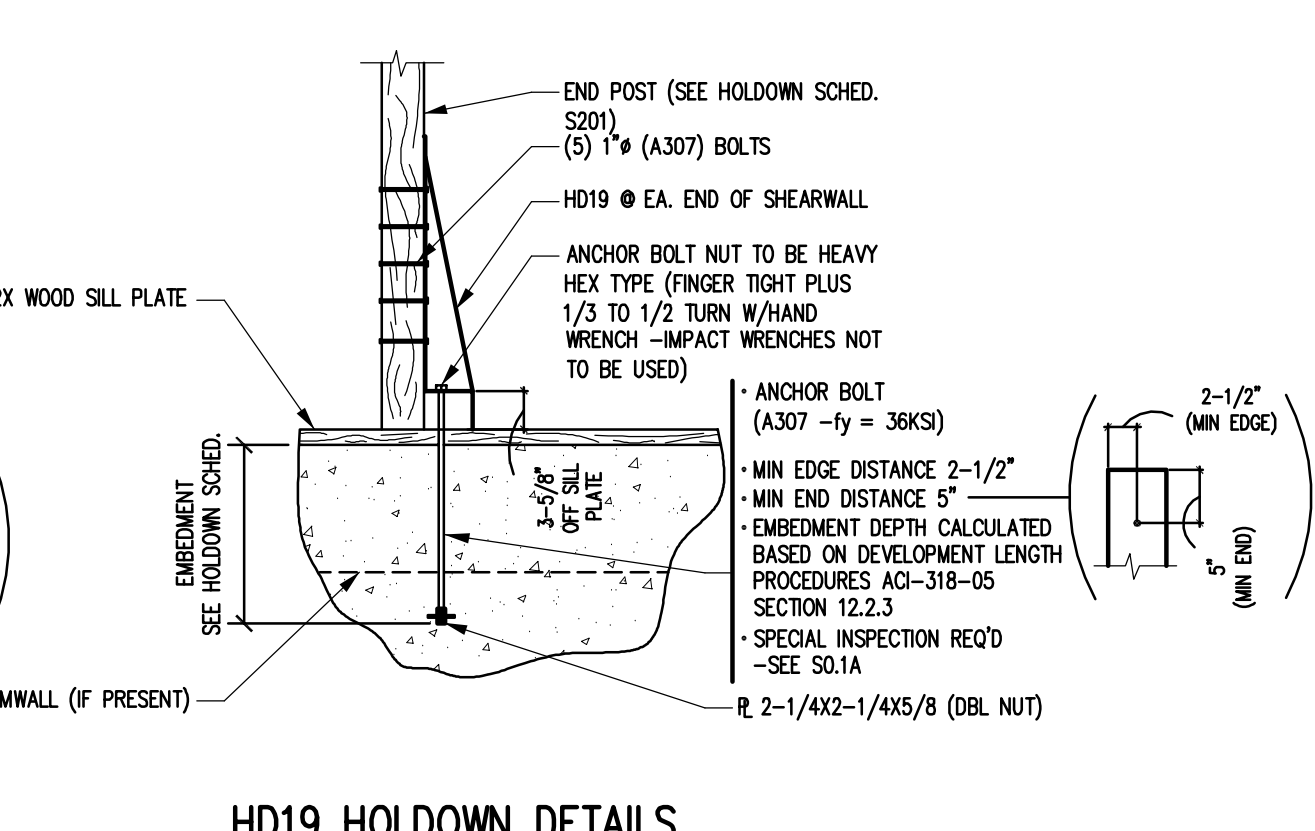
HDU HOLDOWN DETAILS
DRILLED AND EPOXIED HOLDOWN ANCHOR SYSTEM IN CONCRETE



HD19 HOLDOWN DETAILS

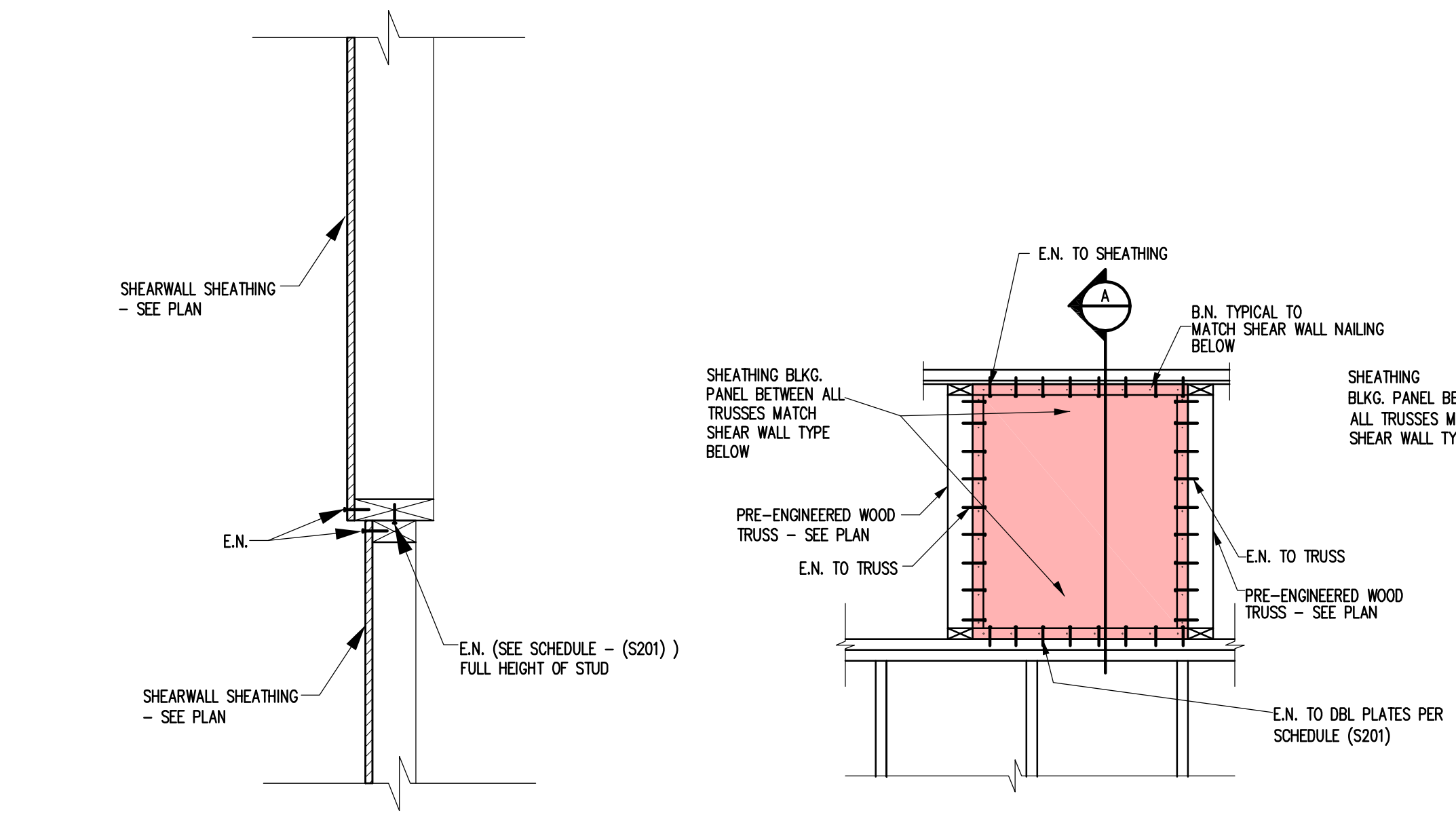


HDU HOLDOWN DETAILS

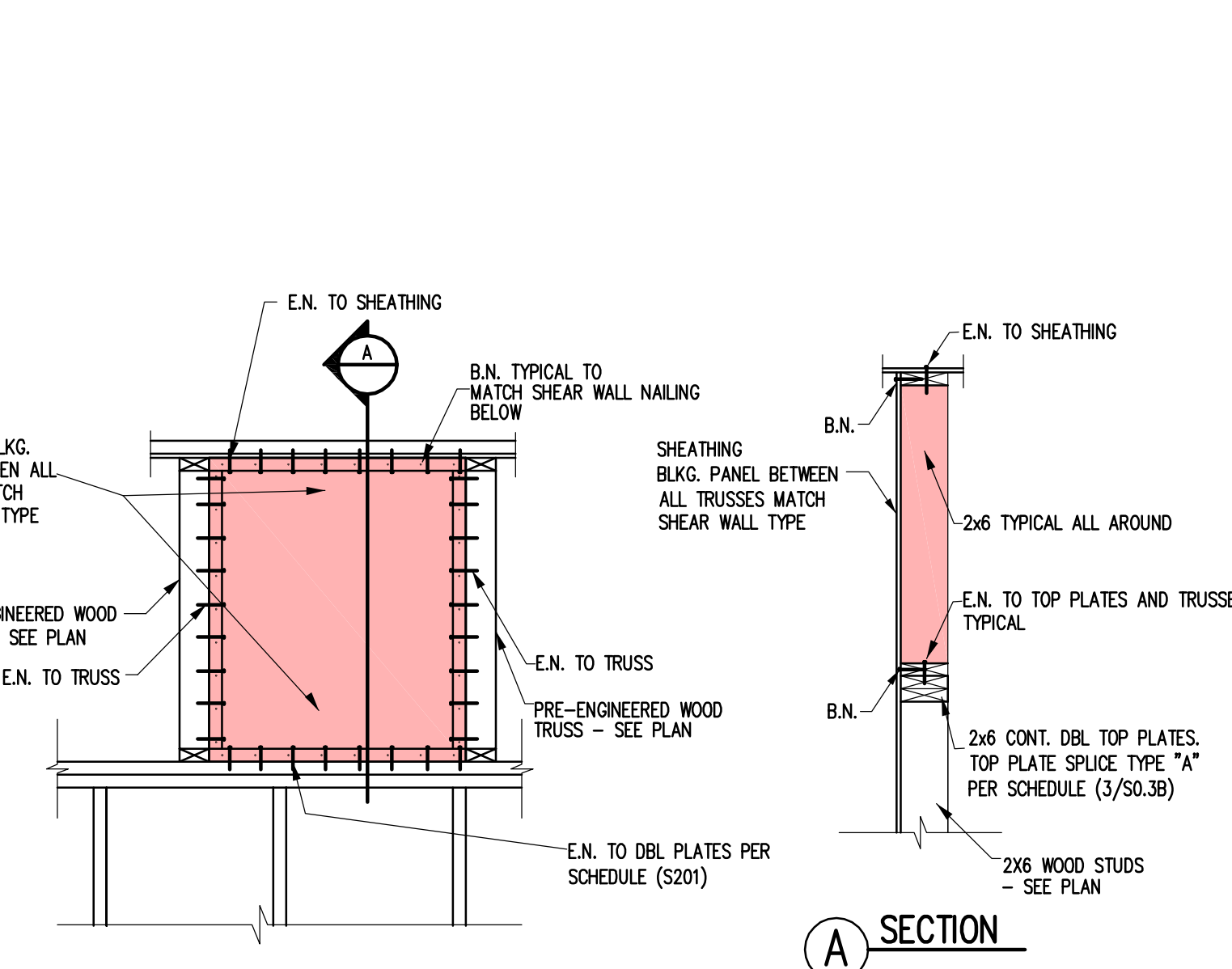


HD19 HOLDOWN DETAILS

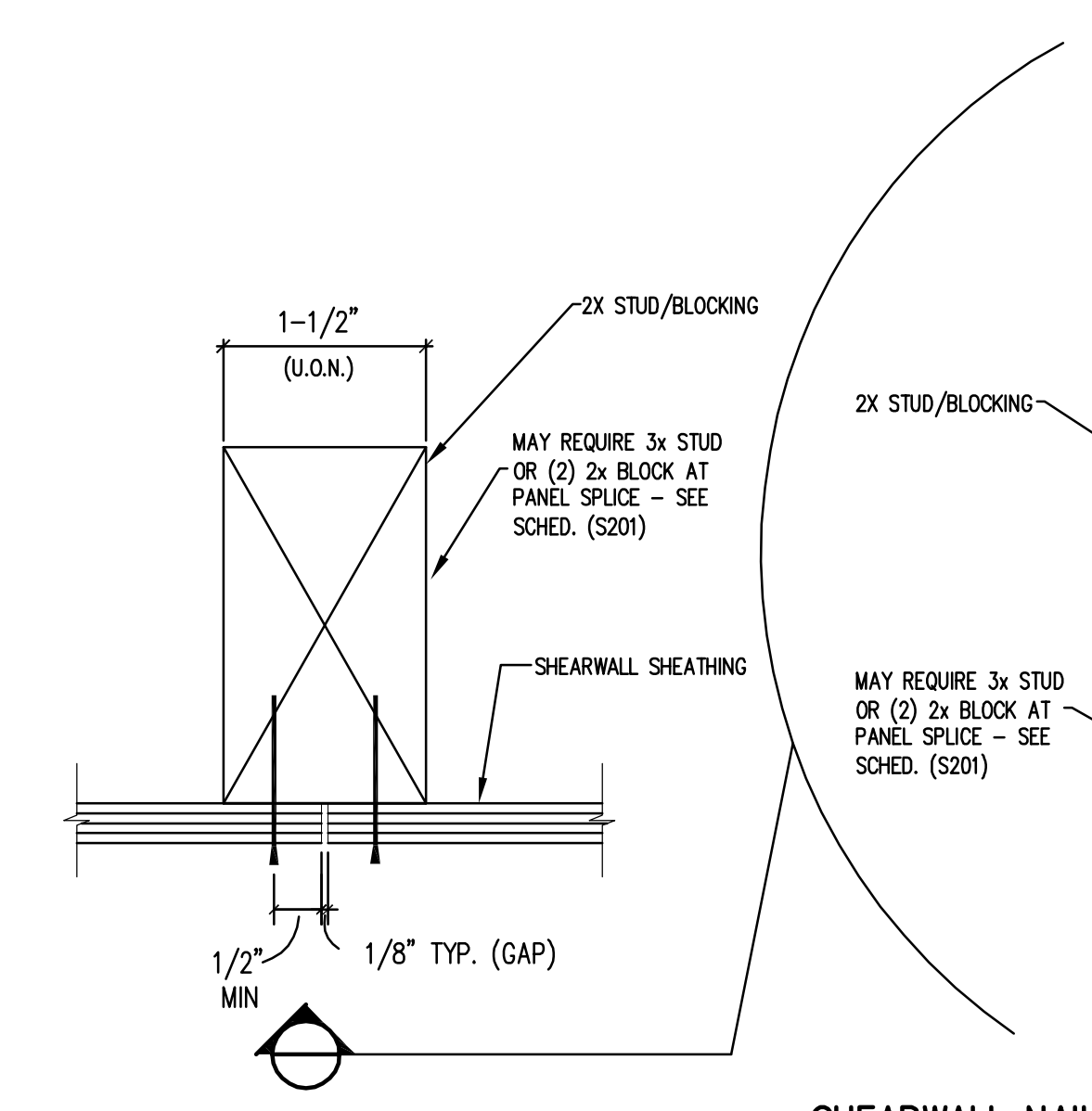
CAST-IN-PLACE HOLDOWN ANCHOR OPTION IN CONCRETE



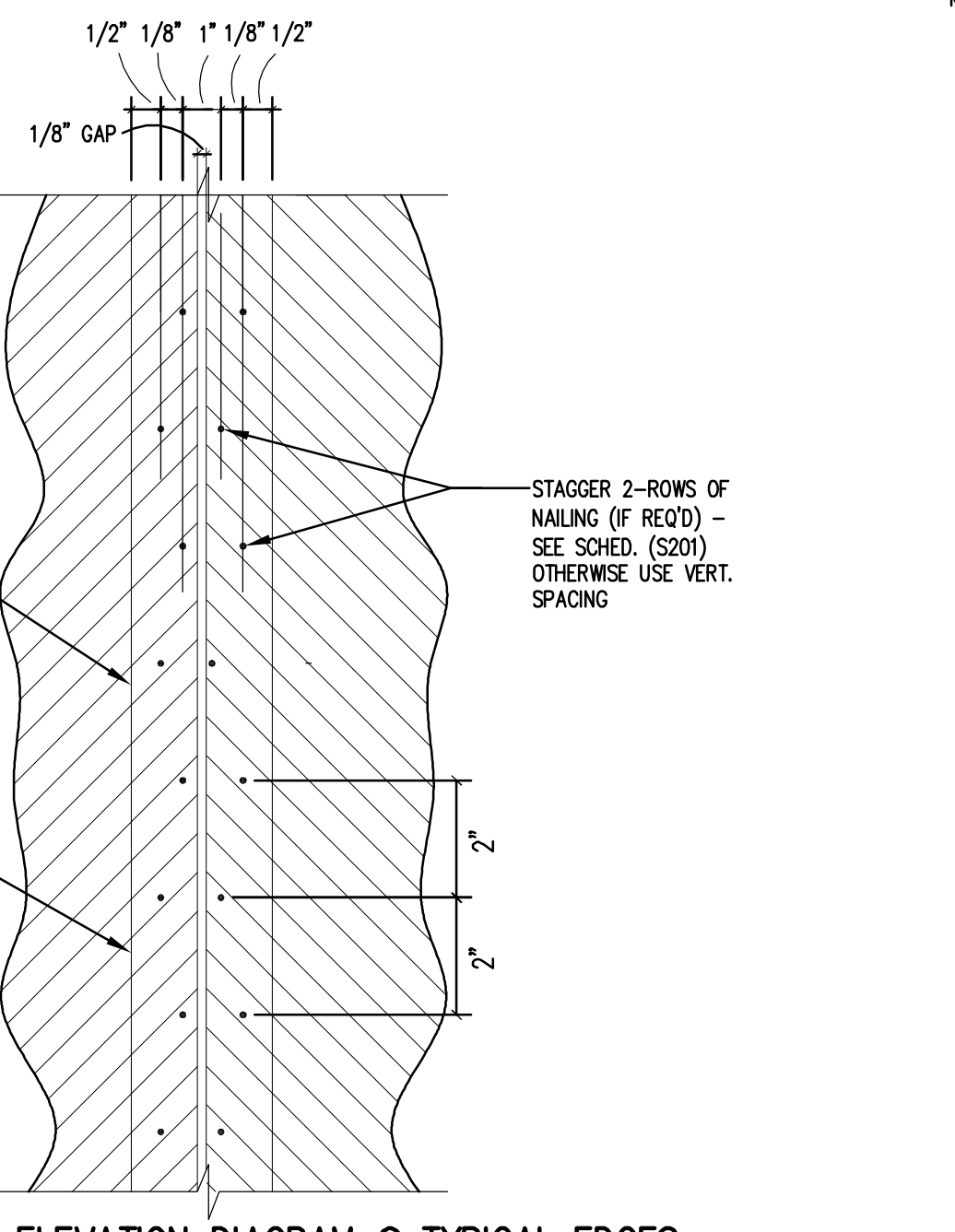
SHEARWALL CONTINUATION @ WALL THICKNESS CHANGE (WHERE OCCURS)



FIELD CONSTRUCTED BLOCKING PANEL (OPTION) (BETWEEN TRUSSES)



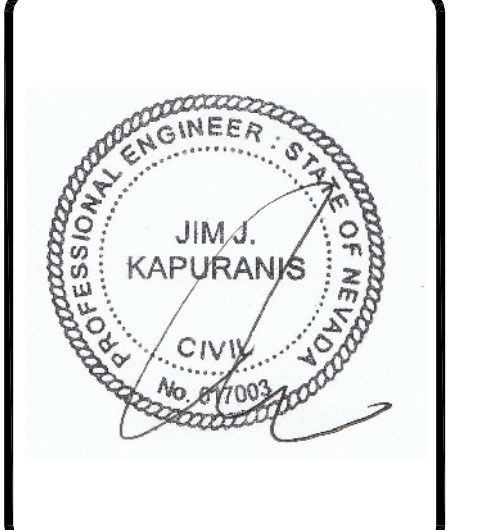
EDGE SPACING



SHEARWALL NAILING ELEVATION DIAGRAM @ TYPICAL EDGES

2 MISC. SHEARWALL DETAILS

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WOOD SHEARWALL GENERAL NOTES

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MASONRY

• CONCRETE MASONRY UNITS SHALL BE HOLLOW, SUITABLE FOR BEARING WALL CONSTRUCTION. ALL BLOCKS SHALL CONFORM TO GRADE "N" UNITS GIVEN IN ASTM C90 LATEST EDITION, AND IN ADDITION SHALL HAVE A LINEAR SHRINKAGE OF 0.08% MAXIMUM FROM SATURATED TO THE OVEN DRY CONDITION. MASONRY UNITS SHALL HAVE CURED FOR NOT LESS THAN (28) DAYS WHEN PLACED IN THE STRUCTURE. PROVIDE ALL BOND BEAM UNITS, LINTELS, ETC., AS NOTED ON PLANS.

• DO NOT USE CHIPPED OR CRACKED BLOCKS. IF ANY SUCH BLOCKS ARE DISCOVERED IN ANY FINISHING WALL, THEY SHALL BE PROMPTLY REMOVED AND REPLACED WITH NEW BLOCKS TO THE APPROVAL OF THE STRUCTURAL ENGINEER.

• THE USE OF ADMIXTURES SHALL NOT BE PERMITTED IN MORTAR OR GROUT UNLESS SUSTAINING DATA HAS BEEN SUBMITTED TO AND REVIEWED BY THE ENGINEER. THE USE OF ADMIXTURES IN MORTAR SHALL NOT BE PERMITTED WITHOUT REDUCING THE LIME CONTENT. THE USE OF UNCONTROLLED FINE CLAY, DIRT AND OTHER DELETERIOUS MATERIALS IS PROHIBITED.

• AGGREGATES, SANDS FOR MORTAR SHALL CONFORM TO ASTM C144 EXCEPT THAT NOT LESS THAN 3% OF THE SAND SHALL PASS THE NUMBER 100 SIEVE. SAND AND PEA GRAVEL FOR GROUT SHALL CONFORM TO ASTM C404, TABLE 1, COURSE AGGREGATE, EXCEPT WHEN OTHER GRADINGS ARE SPECIFICALLY APPROVED BY THE ENGINEER.

• QUICKLIME SHALL CONFORM TO ASTM C5.

• MASONRY REBAR LAP LENGTHS SHALL BE PER LAP SCHEDULE UNLESS NOTED OTHERWISE ON THESE PLANS.

• FOR PROPER MIXING PLACE THE SAND, CEMENT AND WATER IN THE MIXER IN THAT ORDER FOR EACH BATCH OF MORTAR OR GROUT AND MIX FOR A PERIOD OF AT LEAST (2) MINUTES. ADD THE LIME AND CONTINUE MIXING FOR AS LONG AS NEEDED TO SECURE A UNIFORM MASS BUT NOT IN NO CASE LESS THAN (10) MINUTES. USE MIXERS TO SECURE A UNIFORM CAPACITY. BATCHES REQUIRING FRACTIONAL SACKS WILL NOT BE PERMITTED UNLESS CEMENT IS WEIGHED FOR EACH SUCH BATCH. RETEMPER MORTAR ONLY BY ADDING WATER INTO A BATCH MADE WITH THE MORTAR AND THEN CAREFULLY WORKING THE WATER INTO THE MORTAR. RETEMPERING THE MORTAR BY DASHING WATER OVER THE MORTAR SHALL NOT BE PERMITTED. ANY MORTAR OR GROUT WHICH IS UNUSED WITHIN (1) HOUR OF THE INITIAL MIXING SHALL BE REMOVED FROM THE WORK. MORTAR SHALL BE MIXED AND MAINTAINED ON THE BOARDS TO A SLUMP OF (2-5/4") TO PLUS OR MINUS (1/4").

• WATER USED FOR MORTAR AND GROUT SHALL BE CLEAN AND FREE FROM DELETERIOUS AMOUNTS OF ACID, SALTS, ALKALI AND ORGANIC MATERIALS.

• WHEN GROUTING IS STOPPED FOR A PERIOD OF (1) HOUR OR LONGER, FORM HORIZONTAL CONSTRUCTION JOINTS BY STOPPING THE GROUT POUR (1-1/2") MINIMUM BELOW THE UPPER MOST UNIT.

• ALL MASONRY SHALL BE BUILT TO PRESERVE THE UNOBSTRUCTED VERTICAL CONTINUITY OF THE CELLS TO BE FILLED. THE VERTICAL ALIGNMENT SHALL BE SUFFICIENT TO MAINTAIN A CLEAR, UNOBSTRUCTED VERTICAL FLUE MEASURING NOT LESS THAN (3) INCHES, EXCEPT WHERE OPEN END UNITS ARE USED.

• REMOVE CONCRETE SCUM AND GROUT STAINS ON THE WALL IMMEDIATELY. AFTER THE WALL IS CONSTRUCTED, DO NOT SATURATE WITH WATER FOR CURING OR ANY OTHER PURPOSE. CHECK ALL JOINTS FOR TIGHTNESS AND, WHERE CRACKS ARE VISIBLE, CHIP OUT THE MORTAR, TUCK POINT AND TOOL TO MATCH ADJACENT JOINTING.

• GROUT FILL FOR CELLS SHALL CONSIST OF ONE PART PORTLAND CEMENT TO NOT MORE THAN (3) PARTS SAND, TO (2) PARTS PEA GRAVEL (3/8") MAX. SIZE COURSE AGGREGATE. GROUT FILL USING COURSER AGGREGATE MAY BE USED IF THE MIX IS PROPERLY DESIGNED AND APPROVED BY THE ENGINEER. THE MAXIMUM SIZE OF AGGREGATE USED SHALL NOT EXCEED (1/3) THE LEAST LATERAL DIMENSION OF THE CELL TO BE FILLED. GROUT SHALL DEVELOP A MINIMUM COMPRESSIVE STRENGTH OF 2000 PSI AT (28) DAYS.

• ASSUMED COMPRESSIVE STRENGTH F'_m SHALL BE 1500 PSI UNLESS OTHERWISE NOTED ON THESE PLANS. ULTIMATE COMPRESSIVE STRENGTH BASED ON THE AVERAGE OF (3) UNITS SHALL BE NOT LESS THAN 2000 PSI.

• SPECIAL INSPECTION SHALL NOT BE REQUIRED UNLESS SPECIFICALLY NOTED ON THESE PLANS.

• MASONRY LINTELS SHALL BE SOLID GROUTED FOR THE REQUIRED DEPTH. HORIZONTAL REINFORCING SHALL EXTEND (24") BEYOND THE OPENING ON EACH SIDE.

• MAXIMUM HEIGHT OF ANY GROUT POUR SHALL NOT BE GREATER THAN (4') UNLESS PROPER HIGH-LIFT METHODS ARE USED.

• MORTAR SHALL CONFORM TO ASTM C1329, TYPE S WITH A COMPRESSIVE STRENGTH OF 1800 (MIN SLUMP OF 9") PSI AT 28 DAYS UNLESS NOTED OTHERWISE. TYPE M WITH MINIMUM COMPRESSIVE STRENGTH OF 2500 PSI AT 28 DAYS SHALL BE USED WHERE MASONRY IS BELOW GRADE OR IN CONTACT WITH EARTH. THE MIX SHALL BE REVIEWED BY THE ENGINEER WHEN SPECIAL INSPECTION IS REQUIRED.

• PLACE MORTAR IN HORIZONTAL JOINTS, COMPLETELY COVER THE FACE SHELLS OF THE UNITS WITH MORTAR. SOLID FILL ALL HEAD JOINTS. LAY ALL MASONRY WITH COMMON OR RUNNING BOND. HOLD RAKING TO A MINIMUM.

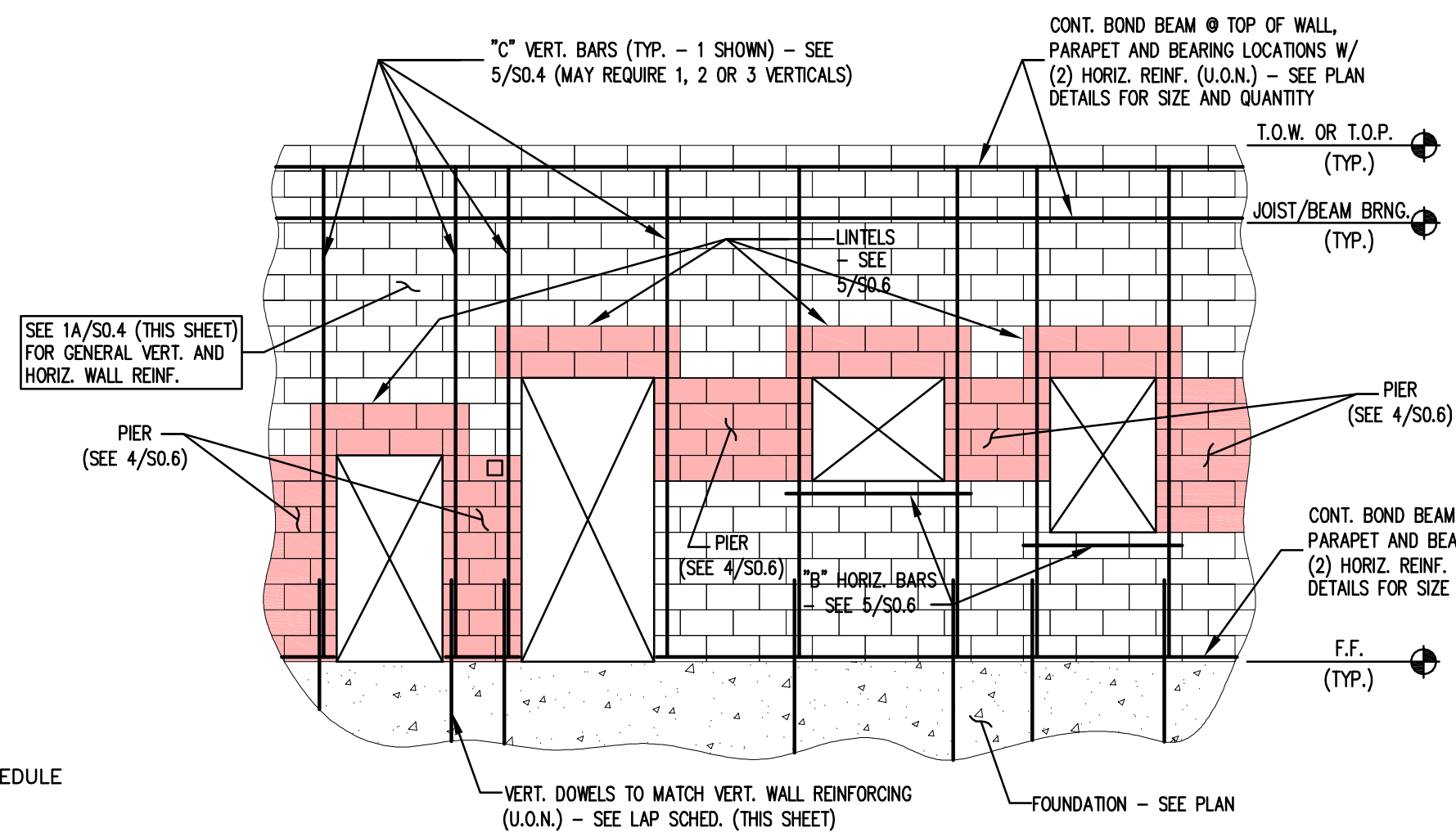
• CEMENT SHALL BE PORTLAND CEMENT CONFORMING TO ASTM C150, TYPE I OR TYPE II AND SHOULD BE ENTIRELY OF ONE MANUFACTURER.

• BOND BEAM HORIZONTAL REINFORCEMENT SHALL BE SOLIDLY ENCASED IN GROUT. WIRE MESH SHALL BE USED IN EACH CELL BELOW BOND BEAM TO PREVENT THE FLOW OF GROUT INTO UNGROUTED CELLS.

• GROUT ALL CELLS CONTAINING VERTICAL REINFORCEMENT, ANCHOR BOLTS OR EMBEDDED ITEMS. PROVIDE (2") MINIMUM COVER TO EMBEDDED ITEMS.

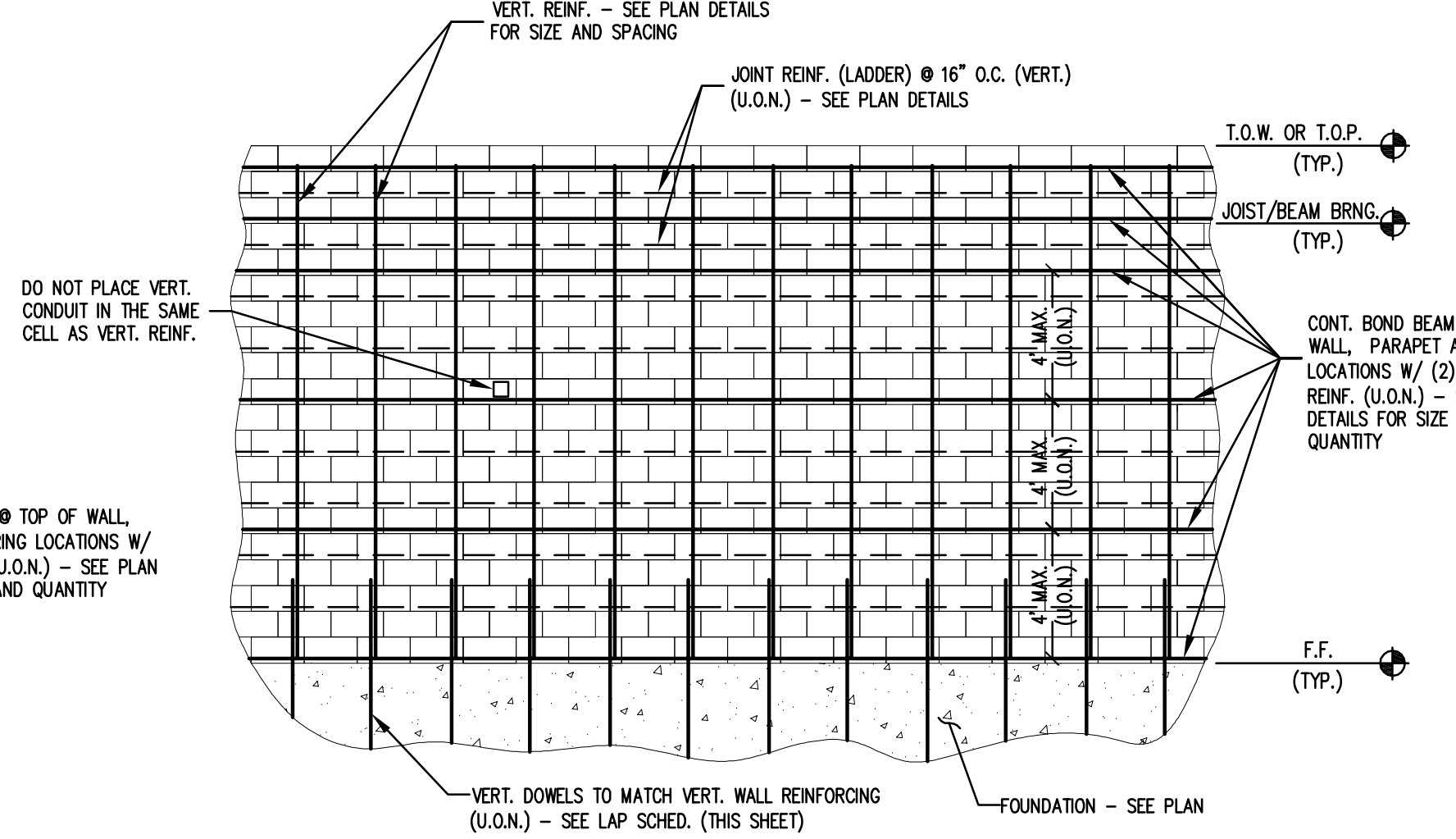
• ALL VERTICAL WALL REINFORCEMENT SHALL HAVE DOWELS EQUAL IN SIZE EMBEDDED INTO FOOTING UNLESS NOTED OTHERWISE IN THESE PLANS.

• REINFORCING COVER SHALL BE (2") MINIMUM THROUGHOUT. POSITIONING DEVICES SHALL BE USED TO INSURE THE CORRECT PLACEMENT OF THE REINFORCEMENT.



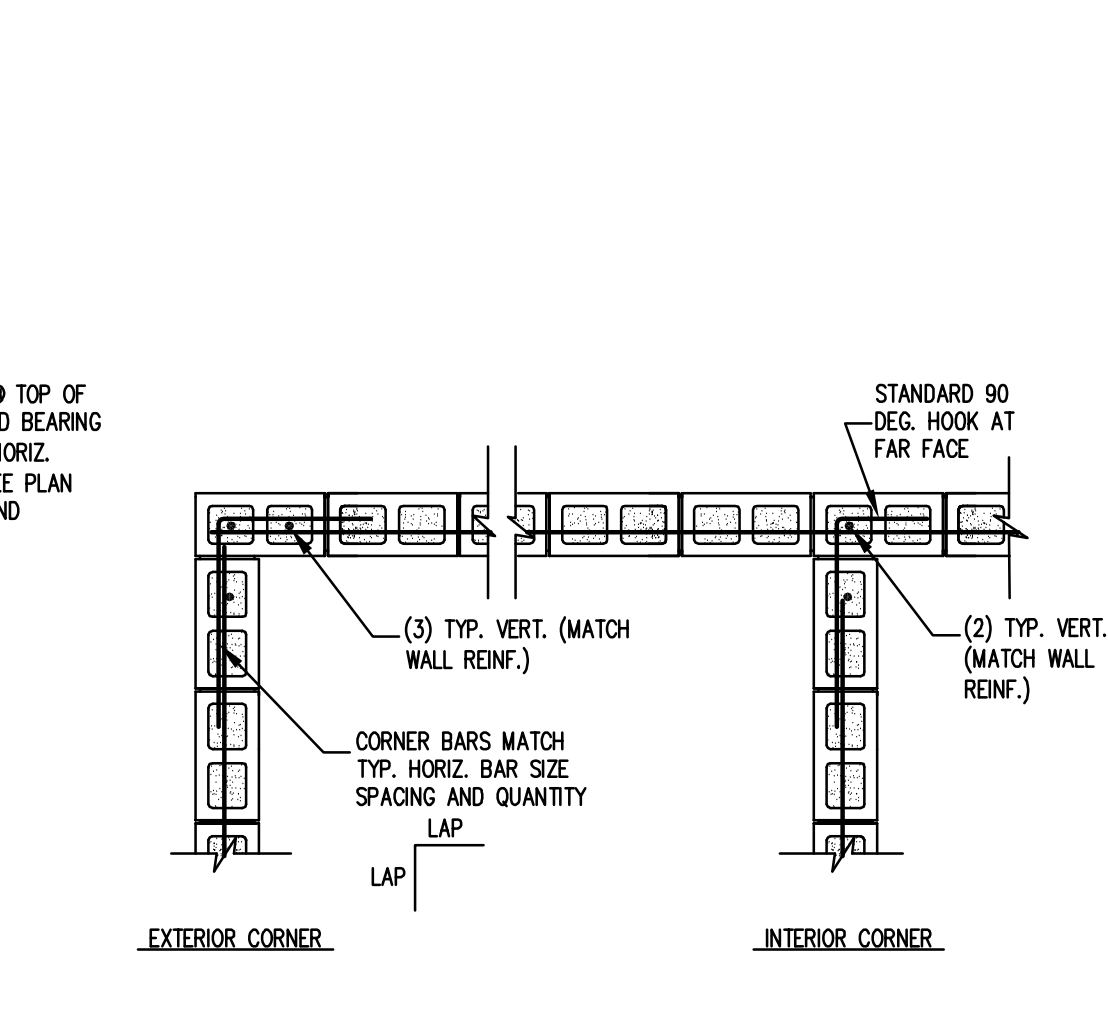
- NOTES:
1. LAP VERTICAL REINFORCING WITH WALL DOWELS AND ALL OTHER VERT. BARS PER 6/50.6 (SCHED.).
 2. STAGGER SPLICES IN ADJACENT HORIZONTAL BARS IN THE SAME COURSE BY 24".
 3. PROVIDE DOWEL BARS IN FOUNDATION TO MATCH ALL VERTICAL REINFORCING (U.O.N.).
 4. FOR ADDED REINFORCING AT WALL INTERSECTIONS AND CORNERS, SEE DETAIL 2/50.6.
 5. GROUT EACH SIDE OF OPENING AS NOTED - SEE TYPICAL OPENING REINFORCING DETAIL 5/50.6.

1 S0.4 REQUIRED MASONRY WALL REINFORCEMENT AT OPENINGS NTS



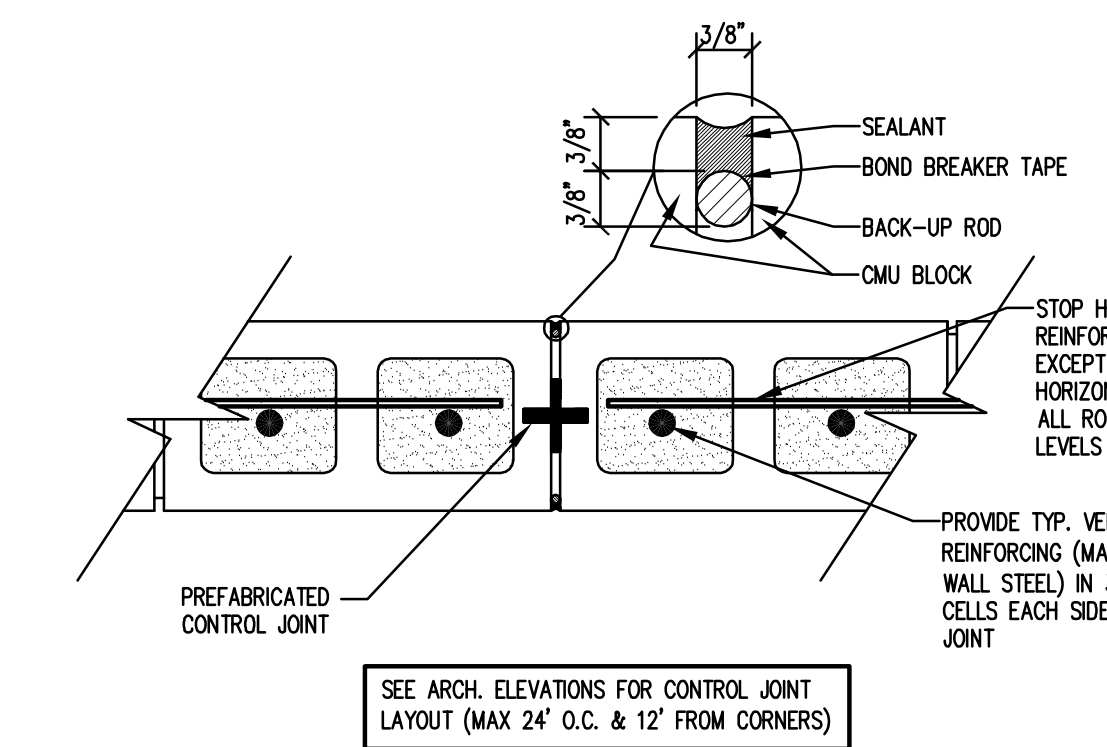
- NOTES:
1. LAP VERTICAL REINFORCING WITH WALL DOWELS AND ALL OTHER VERT. BARS PER 6/50.6 (SCHED.).
 2. STAGGER SPLICES IN ADJACENT HORIZONTAL BARS IN THE SAME COURSE BY 24".
 3. PROVIDE DOWEL BARS IN FOUNDATION TO MATCH ALL VERTICAL REINFORCING (U.O.N.).
 4. FOR ADDED REINFORCING AT WALL INTERSECTIONS AND CORNERS, SEE DETAIL 2/50.6.

1A S0.4 TYPICAL MASONRY WALL REINFORCEMENT AWAY FROM OPENINGS NTS

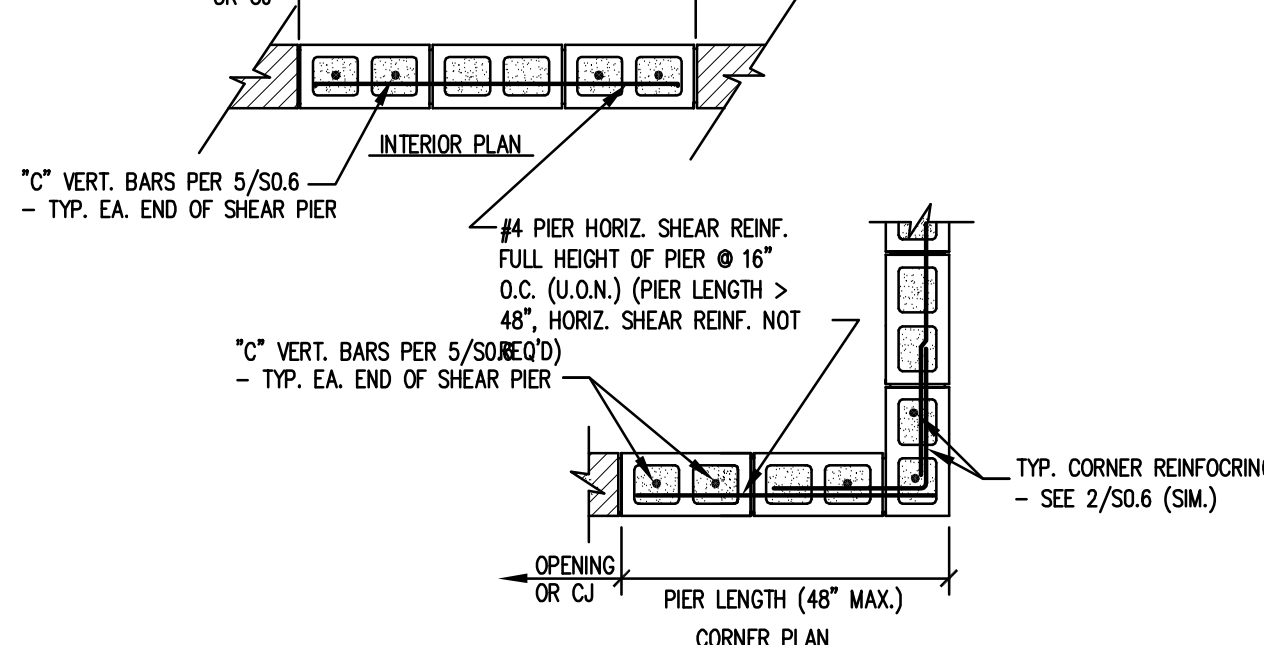


- NOTES:
1. LAP = 48 BAR DIAMETERS OR 24" MINIMUM (U.O.N.).
 2. PROVIDE A STANDARD 90 DEG. HOOK EACH END ON ANY HORIZONTAL BAR BETWEEN OPENINGS, CONTROL JOINTS, OR CORNER BARS LESS THAN 6'-0" IN LENGTH.
 3. PROVIDE CORNER BARS FOR ALL BOND BEAM AND LINTEL REINFORCING.

2 S0.4 CMU WALL CORNERS NTS

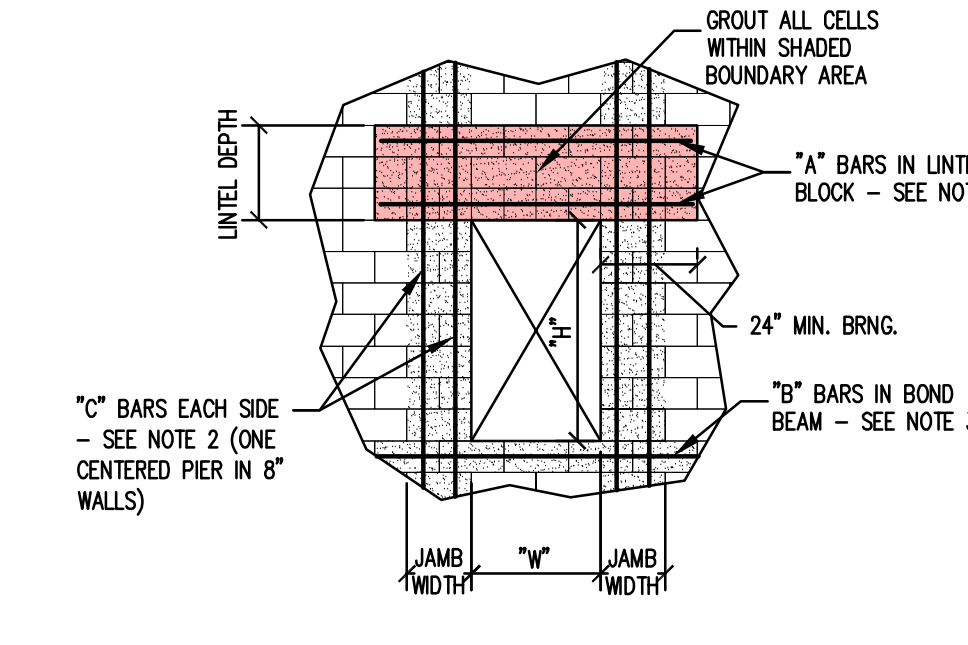


3 S0.4 TYPICAL CMU CONTROL JOINT NTS



- NOTES:
1. GROUT PIERS FULL WALL HEIGHT.
 2. WHERE PIERS ARE CENTERED BELOW BEAM BEARING, REDUCE TIE SPACING TO 8".

4 S0.4 CMU WALL PIER BETWEEN OPENINGS NTS



CMU OPENING REINFORCING SCHEDULE

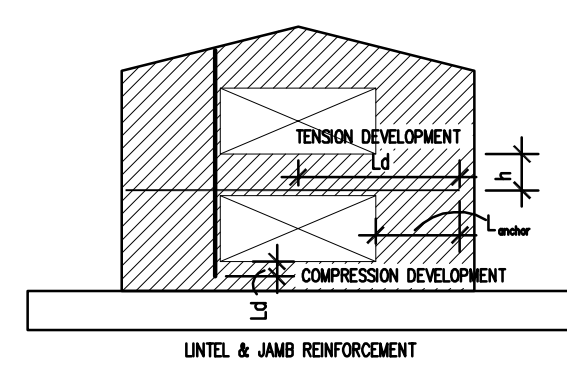
W	LINTEL DEPTH	8" WALLS			JAMB WIDTH
		"A" BARS	"B" BARS	"C" BARS	
< 2'-8"	8"	(1) #5	(1)	(1)	8"
2'-8" ≤ 4'-0"	16"	(2) #5	(1)	(1)	8"
4'-0" ≤ 6'-0"	24"	(2) #6	(1)	(2)	16"
6'-0" ≤ 8'-0"	32"	(2) #6	(2)	(3)	24"
8'-0" ≤ 10'-0"	48"	(2) #6	(2)	(3)	24"
UP TO 13'-4"	48"	(2) #6	(2)	(3)	24"

- NOTES:
1. USE BAR QUANTITIES AND SIZES GIVEN IN LINTEL SCHEDULE UNLESS OTHERWISE NOTED ON THE DRAWINGS.
 2. EXTEND "C" BARS 48 BAR DIAMETERS OR 24" MINIMUM BEYOND TOP AND BOTTOM OF OPENING EXCEPT THAT WHEN "H" OR "W" EXCEEDS 24". "C" BARS SHALL EXTEND FULL HEIGHT, WHERE THERE IS LESS THAN 8" BETWEEN ADJACENT OPENINGS, EXTEND REINFORCING CONTINUOUS TO 32" BEYOND FURTHEST OPENING.
 3. "A" AND "B" BARS SHALL EXTEND 48 BAR DIAMETERS OR 24" MINIMUM EACH SIDE OF THE OPENINGS.
 4. FOR BAR SIZES, MATCH TYPICAL WALL REINFORCING AS SHOWN ON THE BUILDING WALL SECTIONS, U.O.N.

5 S0.4 CMU OPENING REINFORCING NTS

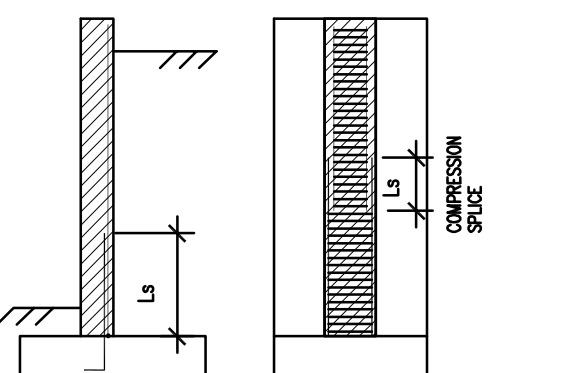
• TENSION & COMPRESSION SPLICE

BAR SIZE	L_d VALUES (IN)					
	1000	1500	2000	2500	3000	3500
#3	25	24	24	24	24	24
#4	36	36	36	36	36	36
#5	45	45	45	45	45	45
#6	54	54	54	54	54	54
#7	73	63	63	63	63	63
#8	112	91	79	72	72	72



• TENSION & COMPRESSION DEVELOPMENT

BAR SIZE	L_d VALUES (IN)					
	1000	1500	2000	2500	3000	3500
#3	25	24	24	24	24	24
#4	25	20	17	16	14	13
#5	31	25	22	20	18	16
#6	52	43	37	33	30	28
#7	73	59	51	46	42	39
#8	112	91	79	71	64	60

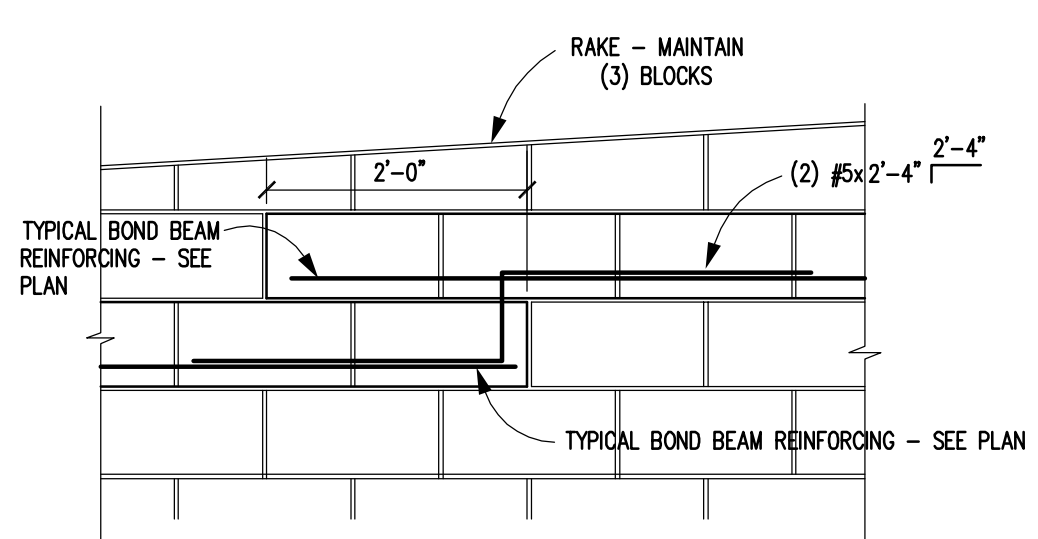


• ANCHORAGE OF FLEXURAL REINFORCEMENT

BAR SIZE	L_{dev} VALUES (IN)					
	1000	1500	2000	2500	3000	3500
ANY						

$L_{dev} = \text{MAXIMUM VALUE OF } (h \text{ OR } 12d_b) \text{ WHERE } h = \text{SEE ELEV. THIS SHEET } d_b = \text{REIN. BAR DIA.}$

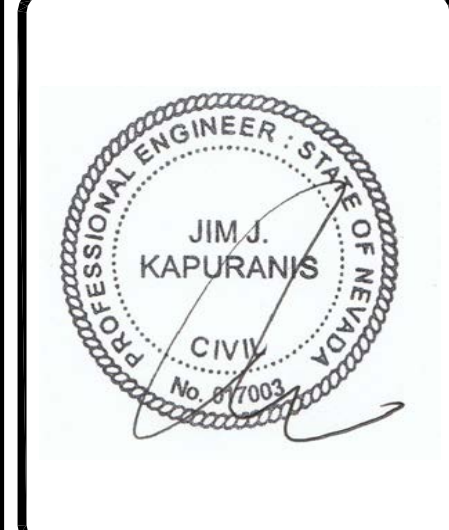
6 S0.4 REINFORCING DEVELOPMENT LENGTHS / SPLICES (WHERE APPLICABLE) NTS



7 S0.4 BOND BEAM STEP DETAIL NTS

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MASONRY GENERAL NOTES AND DETAILS

REV.#	DATE	COMMENTS

REVIEWS

INITIALS	DATE	REVISION

CCAS
PROJECT NO.: 1107
DRAWN:
DATE: 7/23/15
S0.4
OF

JJK Group, Inc.
Consulting Structural Engineers

3240 Juan Tabo Blvd. C Albuquerque, New Mexico 87111 Tel: 505.296.5706 Fax: 505.296.9172 www.jjkgroup.com

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Plot Date: July 20, 2015 - 4:41 PM
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STRUCTURAL STEEL

- STRUCTURAL STEEL SHALL BE SHOP FABRICATED IN ACCORDANCE WITH AISC "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES" LATEST EDITION, WELDING SHALL COMPLY WITH AWS 1.1 "STRUCTURAL STEEL WELDING".
- CONTRACTOR TO VERIFY ALL MEASUREMENTS AT JOB SITE.
- PROVIDE ALL LUGS, CLIPS, ANGLES AND MISCELLANEOUS FASTENERS NECESSARY FOR THE COMPLETE ASSEMBLY AND INSTALLATION.
- DO ALL GROUTING OF BASE PLATES AND SIMILAR ITEMS WITH NON-SHRINK GROUT.
- PROTECT ALL DISSIMILAR METALS FROM GALVANIC CORROSION.
- PROVIDE WASHERS ON ALL HEADS AND NUTS BEARING ON WOOD. DRAW ALL NUTS TIGHT AND UPSET THREADS OF PERMANENT CONNECTIONS TO PREVENT LOOSENING. USE BEVELED WASHERS WHERE BEARING IS ON SLOPED SURFACES.
- THOROUGHLY CLEAN ALL MILL SCALE, RUST, DIRT, GREASE AND OTHER FOREIGN MATTER FROM FERROUS METAL PRIOR TO PAINTING.
- AFTER MATERIAL HAS BEEN PROPERLY CLEANED AND TREATED, APPLY PRIME COAT OF PAINT TO ALL SURFACES EXCEPT THOSE ENCASED IN CONCRETE OR MASONRY. APPLY ALL PAINT AS PER MANUFACTURER'S DIRECTIONS. SPOT PAINT ALL ABRASIONS AND FIELD CONNECTIONS AFTER ASSEMBLY. SHOP COAT SHALL BE DRY PRIOR TO SHIPMENT TO JOB SITE.
- REFER TO SHEET S0.1 FOR ALL "ER" REQUIRED REPORTS.
- WELDING FOR STRUCTURAL STEEL SHALL BE IN ACCORDANCE WITH AWS CODE P1.1. WELDS SHALL BE MADE ONLY BY OPERATORS EXPERIENCED IN PERFORMING THE TYPE OF WORK INDICATED. WELDS NORMALLY EXPOSED TO VIEW IN THE FINISHED WORK SHALL BE UNIFORMLY MADE AND GROUND SMOOTH. WHERE WELDING IS DONE IN PROXIMITY TO GLASS OR FINISHED SURFACES, SUCH SURFACES SHALL BE PROTECTED FROM DAMAGE DUE TO WELD SPARKS, SPATTER OR TRAMP METAL.
- INDIVIDUAL WELDERS FOR STRUCTURAL METAL WORK SHALL BE QUALIFIED FOR THE WELDS BEING PERFORMED.
- WELDING ELECTRODES TO BE E70 SERIES UNLESS OTHERWISE NOTED.
- ALL STRUCTURAL MISCELLANEOUS STEEL SHALL BE FABRICATED AND ERECTED IN ACCORDANCE WITH THE A.I.S.C SPECIFICATIONS FOR THE DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS, LATEST EDITION.

ASTM MATERIAL REQUIREMENTS:

- ASTM A-992 GRADE 50 FOR ALL WIDE FLANGE W SECTIONS AND T SECTIONS.
- ASTM A-36 FOR C, MC, BARS, ANGLES, AND PLATES.
- ASTM A-53 GRADE B OR A-501 FOR STEEL PIPES.
- ASTM A-500 GRADE B, F_y=46 KSI FOR TS/HSS TUBE STEEL FOR SIZES UP TO 5/8" THICK.
- ASTM A-501 GRADE B, F_y=36 KSI FOR TS/HSS TUBE STEEL FOR OTHERS UP TO 1" THICK.
- ASTM A-307 PLAIN BOLTS.
- ASTM F-1554 GRADE 36, A-307 OR A-36 PLAIN ANCHOR BOLTS.
- ASTM A-325-N HIGH STRENGTH BOLTS; TESTED BY A CALIBRATED WRENCH UNLESS LOAD INDICATOR BOLTS ARE USED. HIGH STRENGTH BOLTS SHALL BE TIGHTENED TO "SHUG TIGHT" CONDITION PER AISC UNLESS SPECIFICALLY CALLED OUT TO BE "TORQUED AND TESTED" ON PLANS BY A CALIBRATED TORQUE WRENCH.

- CLEARLY IDENTIFY EACH PIECE OF STRUCTURAL STEEL WITH A PIECE MARK AND THE CORRESPONDING STRUCTURAL ASTM GRADE AND YIELD STRESS.
- SEE MECHANICAL DRAWINGS FOR MECHANICAL EQUIPMENT SUPPORT FRAMING AND SPREADERS. SEE MECHANICAL & ARCHITECTURAL DRAWINGS FOR EXACT QUANTITY, SIZE, LOCATIONS, AND WEIGHT OF MECHANICAL UNITS.

STRUCTURAL METAL DECK

- THE MANUFACTURING, DETAILING, AND ERECTION OF METAL DECK SHALL BE IN ACCORDANCE WITH THE "STEEL DECK INSTITUTE SPECIFICATIONS", LATEST EDITION.
- ALL METAL DECK SHALL BE FABRICATED FROM STEEL HAVING A MINIMUM YIELD STRENGTH OF 33,000 PSI.
- REFER TO PLANS AND DETAILS FOR DECK SIZE.
- PROVIDE DETAILED AND CHECKED SHOP DRAWINGS INDICATING LOCATION, GAGE, AND SIZE OF EACH PIECE OF DECKING AND RELATED DECKING ACCESSORY. THE DRAWINGS SHALL CLEARLY SHOW WELDING DETAILS TO STRUCTURAL FRAMING ELEMENTS, SIDE LAP CONNECTION DETAILS, DECK OPENINGS, EDGE CLOSURES, AND WHERE REQUIRED, SUPPLEMENTARY DECK AND/OR CLOSURE REINFORCING.
- ALL DECKING SHALL BE WELDED TO STRUCTURAL STEEL BY QUALIFIED WELDERS USING PRE-QUALIFIED PROCEDURES. THE TECHNICAL SPECIFICATIONS ESTABLISH A PROCEDURE FOR PRE-QUALIFICATION OF THE PLUG WELDING OF THE STEEL DECKING TO THE STRUCTURAL STEEL FOR THE PARTICULAR GAGES USED. PRIOR TO THE START OF ERECTION OF THE STEEL DECK, EACH WELDER SHALL BE QUALIFIED USING THIS PROCEDURE AS WITNESSED BY THE OWNER'S STRUCTURAL STEEL TESTING LABORATORY.
- THE METAL DECK SHALL BE DESIGNED TO BE UNSHORED AND CONTINUOUS OVER A MINIMUM OF THREE (3) SPANS IN THE DIRECTION INDICATED. METAL DECKING FOR SINGLE AND DOUBLE SPANS, IF REQUIRED, SHALL ALSO SATISFY THE SPECIFIED LOAD AND DEFLECTION REQUIREMENTS.
- THE DECK SHALL BE DESIGNED FOR AN ASSUMED SUITABLE CONSTRUCTION LIVE LOAD. THE ASSUMED CONSTRUCTION LIVE LOAD SHALL NOT BE LESS THAN 20 PSF. HOWEVER, CONTRACTOR SHALL FOLLOW ALL APPLICABLE CITY, LOCAL, STEEL DECK INSTITUTE, AND AISC REQUIREMENTS FOR TEMPORARY CONSTRUCTION LOADINGS.
- DO NOT HANG LOADS FROM METAL DECK. HANG ALL DUCTWORK, PIPING, ETC. DIRECTLY FROM STRUCTURAL STEEL FRAMING OR SUPPLEMENTARY MEMBERS. ALL HANGING LOAD DETAILS SHALL BE SUBMITTED FOR REVIEW. LATERAL ROOF LOADS SHALL BE TRANSFERRED THROUGH DIAPHRAGM ACTION PROVIDED BY THE METAL ROOF DECK.
- PROVIDE CONTINUOUS SHEET METAL CLOSURES AT ALL SLAB OPENINGS AND SLAB EDGES AND CONTINUOUS DECK CLOSURES AT ALL DECK ENDS.
- ALL METAL DECK SHALL BE WELDED TO THE SUPPORTING STEEL PER SCHEDULE (THIS SHEET). PUDDLE WELDS AT 12 INCHES ON CENTER (6" O.C. AT PERIMETER). THE USE OF WELDING WASHERS FOR ALL METAL DECK WELDING SHALL BE IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS. PROVIDE WELDED SIDE LAP FASTENERS AT 36" MAXIMUM ON CENTER OR #10 TEK FASTENERS AT A MINIMUM OF 2 PER SPAN, UNLESS OTHERWISE NOTED. REFER TO ATTACHMENT PATTERN SCHEDULE THIS SHEET.
- PROVIDE, AS REQUIRED, ALL RIDGE AND VALLEY PLATES, COLUMN CLOSURES, CANT STRIPS, SUMP PLATES AT PIPING PENETRATIONS, AND RECESSED SUMP PANS AT ROOF DRAINS. PROVIDE SUPPLEMENTAL FRAMING, INCLUDING REINFORCING PLATES, AT OPENINGS AS REQUIRED FOR SUPPORT OF THE METAL DECK. COORDINATE ALL OPENINGS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS.

COMPOSITE METAL DECK

- ALL SLAB COMPOSITE METAL DECK SHALL HAVE WIDE RIBS SUITABLE FOR SHEAR STUD PLACEMENT WHERE STUDS ARE REQUIRED. THE CONFIGURATION OF THE METAL DECK SHALL BE SUCH AS TO DEVELOP THE FULL SHEAR VALUE OF THE STUD FOR THE PARTICULAR WEIGHTS OF THE CONCRETE AS LISTED IN THE AISC SPECIFICATIONS, LATEST EDITION. THE CONTRACTOR SHALL PROVIDE VERIFICATION OF THE STUD VALUES FOR THE SPECIFIC DECK TYPE AND STUD SPACING AND PROVIDE ADDITIONAL STUDS AS REQUIRED FOR SPECIFIC DECK TYPES UTILIZED.
- PROVIDE 3/4" DIA. x 3-1/2" HEADED STUDS AS DESIGNATED ON STEEL MEMBERS WHICH SUPPORT METAL DECK SLABS. OMIT STUDS WHERE NOT SHOWN ON PLANS.
- THE CONTRACTOR SHALL PROVIDE CHECKED SHOP DRAWINGS INDICATING EXACT LAYOUT OF STUDS FOR EACH BEAM, SPAN, AND DECK LAYOUT.
- SHEAR STUDS SHALL BE WELDED THROUGH THE METAL DECK BY PRE-QUALIFIED METHODS. IF THROUGH-DECK WELDING IS UNFEASIBLE, THE STUDS SHALL BE INSTALLED IN PRE-PUNCHED HOLES IN THE DECK.

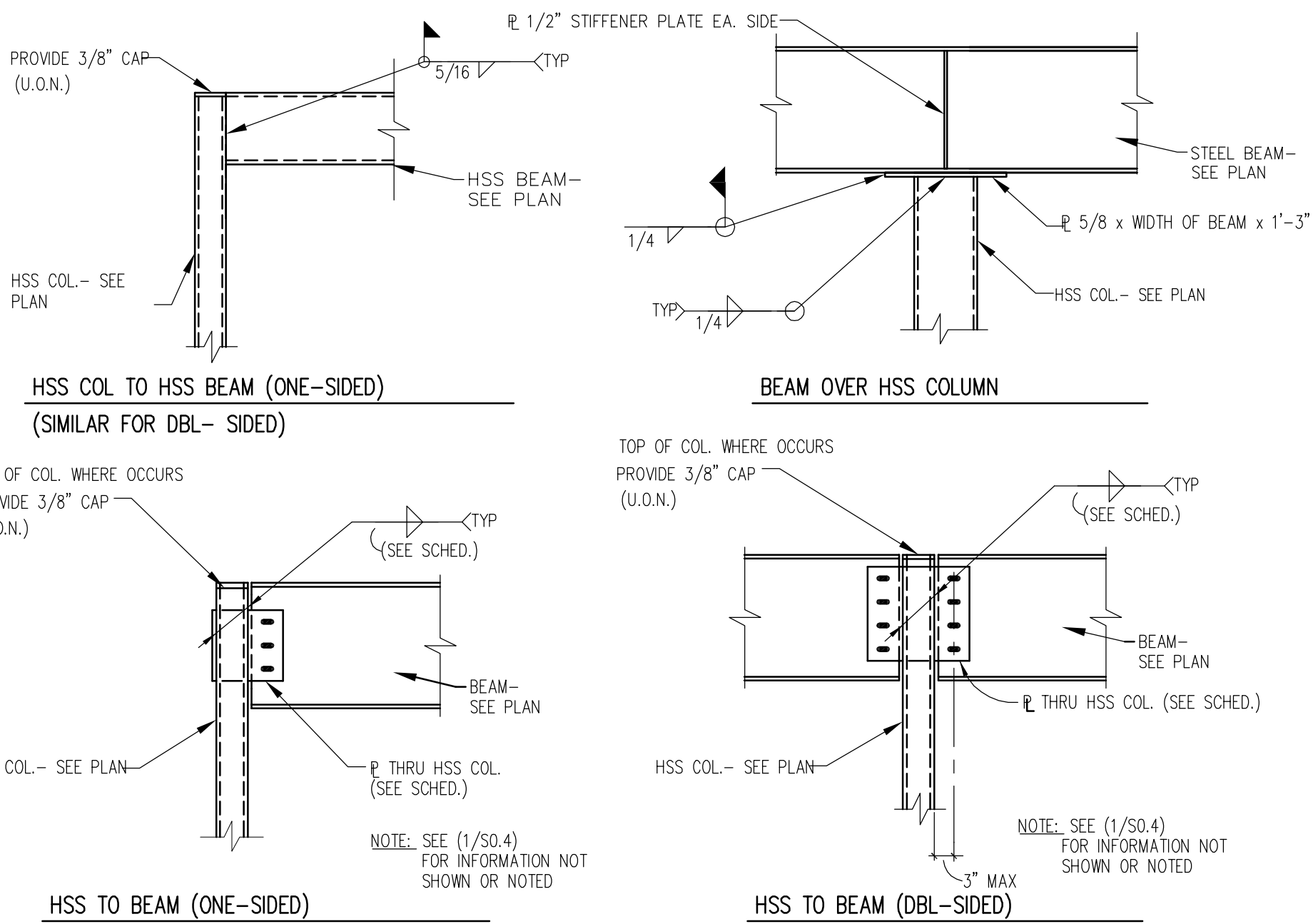
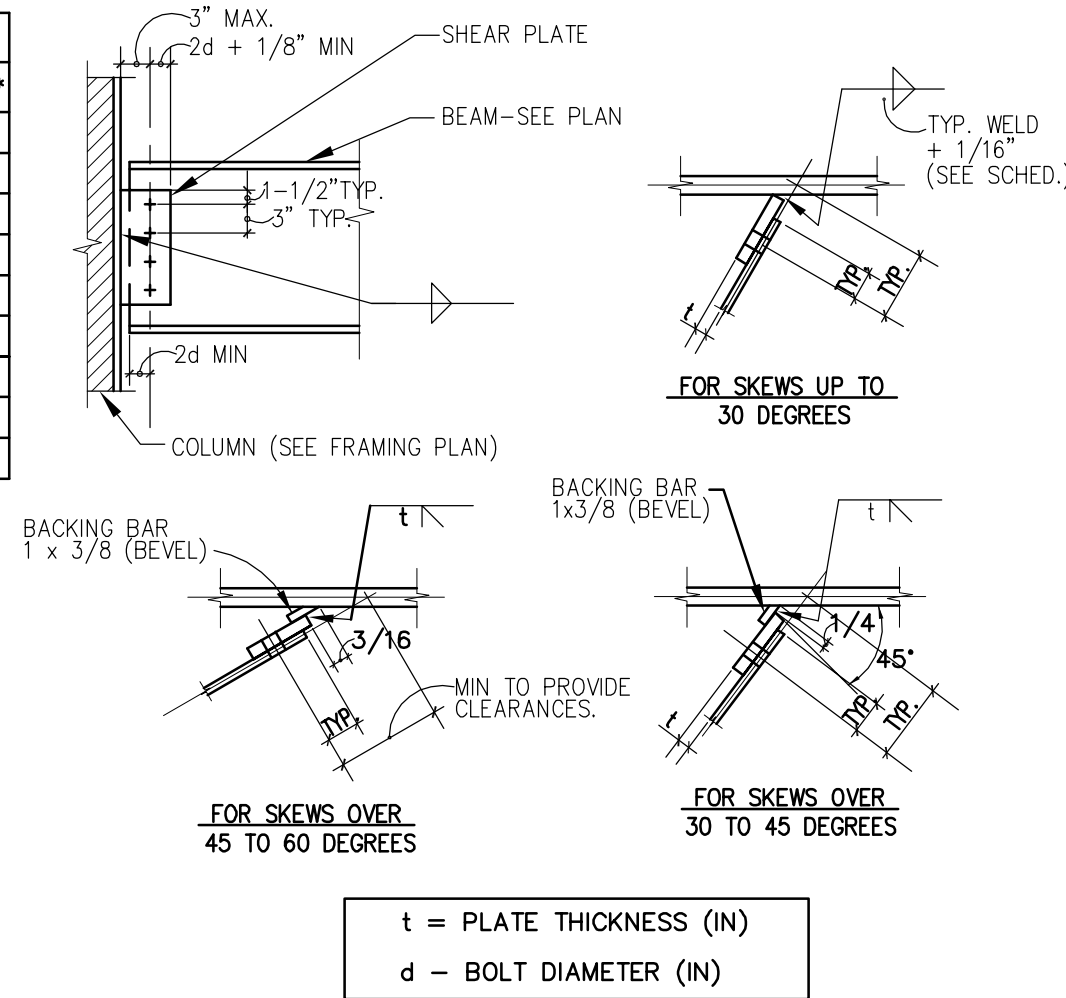
CONNECTION SCHEDULE				
BEAM SIZE	NO. OF 7/8" BOLTS	SHEAR PLATE THICKNESS	WELD SIZE	CAP (KIPS)*
W10x	2	3/8"	1/4" (3)	8.2

*HIGHER CAPACITIES MAY BE REQUIRED - SEE PLANS.

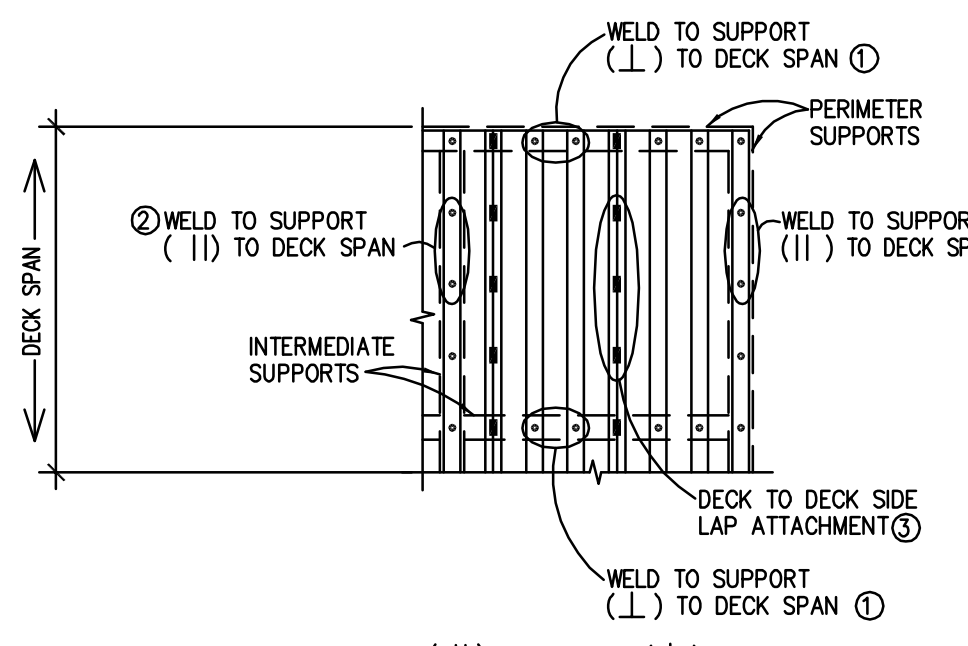
NOTES:

- ALL BOLTS ARE INSTALLED IN STANDARD OR SHORT-SLOTTED (HORIZONTAL ORIENTATION OF SLOT) HOLES AS NOTED IN SPECIFIC DETAILS. REFER TO DETAILS-THIS SHEET FOR SIMPLE BEAM TO COLUMN CONNECTIONS.
- PROVIDE BOLTS IN TWO VERTICAL ROWS WHEN SCHEDULED BOLTS CANNOT FIT IN ONE ROW.
- UNLESS LARGER SIZE WELD IS REQUIRED BY AISC SPECIFICATIONS FOR WELDING TO THICKER PLATES.
- ALL BOLTS ARE A325 BOLTS (SLIP CRITICAL) - U.O.N.
- ALL BOLTED CONNECTIONS SHALL BE TYPE(N) BOLTS.
- FABRICATOR STANDARD CONNECTION DETAILS MAY BE SUBMITTED FOR APPROVAL.
- CHANNEL SHAPES "C" SHALL HAVE SAME CONNECTION AS "W" SHAPES OF EQUAL DEPTH.

1 CONNECTION SCHEDULE AND REQUIREMENTS (WHERE APPLICABLE - SEE PLANS)

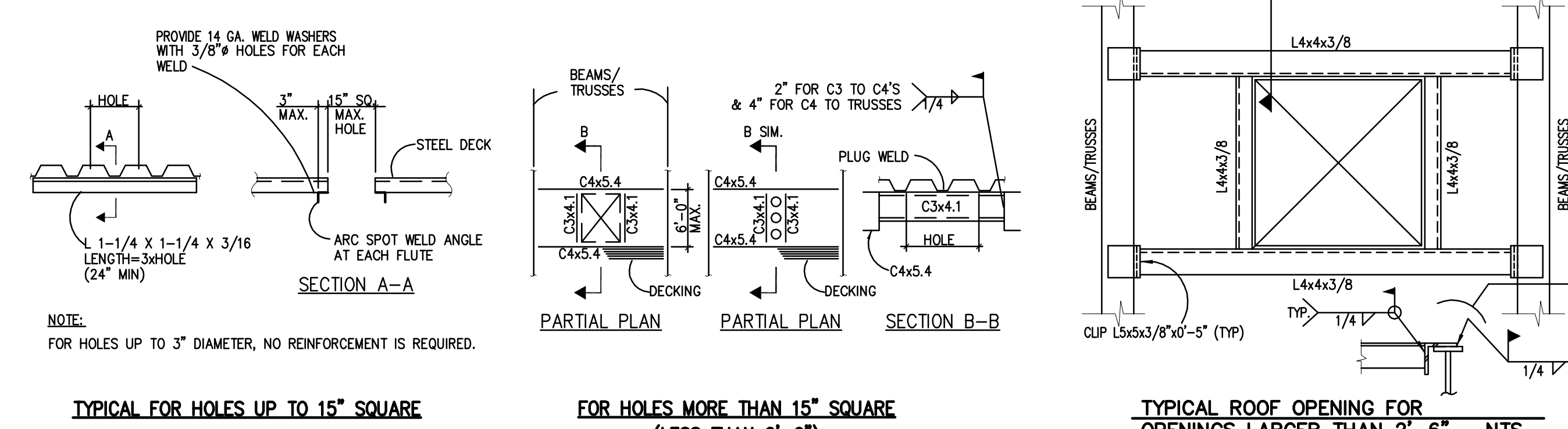


2 TYPICAL SIMPLE SHEAR CONNECTIONS (WHERE APPLICABLE)



DECK TYPE	ATTACHMENT PATTERN			DIAPHRAGM SHEAR	DECK TYPE	DEPTH (INCHES)	GAUGE	CONFIGURATION	I min (IN ⁴ /FT)	S _x min (IN ³ /FT)	F _y (KSI)
	TO SUPPORT PERPENDICULAR TO DECK RIBS ①	TO SUPPORT PARALLEL TO DECK RIBS ②	DECK TO DECK SIDE LAPS ③								
3N	5/8" PUDDLE WELD AT ALL FLUTES	5/8" PUDDLE WELD @ 10" O.C.	(5) FASTENERS PER SPAN #10 TEK	471 LB/FT ON 6'-0" SPAN	B	1-1/2"	18		.292	.327	33
1.5B	5/8" PUDDLE WELD AT ALL FLUTES	5/8" PUDDLE WELD @ 10" O.C.	(7) FASTENERS PER SPAN #10 TEK	455 LB/FT ON 6'-0" SPAN							

- NOTES:
- DIAPHRAGM SHEAR IS IBC ALLOWABLE DIAPHRAGM SHEAR FOR DECK INSTALLED IN ACCORDANCE WITH CONTRACT DOCUMENTS.
 - THE DETAILS SHOWN ON THIS SHEET SHALL BE INCORPORATED INTO THE PROJECT AT ALL LOCATIONS WHERE METAL DECK IS USED, WHETHER SPECIFICALLY CALLED OUT OR NOT.
 - DECK SHALL BE WELDED TO ALL BEAMS AND SUPPORTS.



3 METAL DECK DETAILS

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BDA

PROFESSIONAL ENGINEER, STATE OF NEVADA
JIM J. KAPURANIS
CIVIL
No. 71053

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Carson City, Nevada 89701
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STRUCTURAL STEEL & METAL DECK & DETAILS	
REV.#	DATE

REVIEWS

INITIALS	BDA DSGN. REV.
	BDA TECH. REV.

CCAS

PROJECT NO.: 1107
DRAWN:
DATE: 7/23/15

S0.4A
OF

JKJ JJK Group, Inc.
Consulting Structural Engineers

3240 Juan Tabo Blvd. C. Albuquerque, New Mexico 87111 Tel. 505.296.5700 Fax 505.296.4672 www.jkjgroup.com

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EARTHWORK NOTES

CRITERIA :

ALL INFORMATION BELOW IS A SUMMARY PROVIDED BY THE GEOTECHNICAL ENGINEER SOILS REPORT BY : **RESOURCE CONCEPTS, INC DRAFT**
JAN 9, 2015

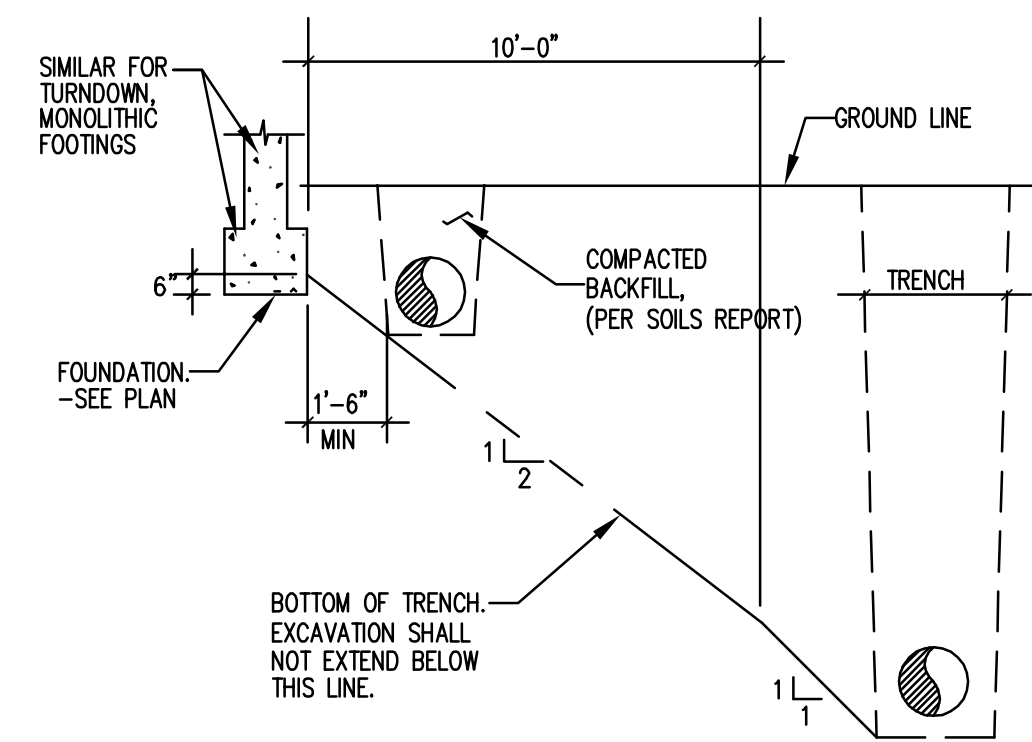
THE FOLLOWING INFORMATION BELOW BY JJK GROUP, INC. DOES NOT RELIEVE THE CONTRACTOR FROM REVIEWING THE SOILS REPORT AND FOLLOWING ALL REQUIRED SUBGRADE PREPARATIONS.

GENERAL :

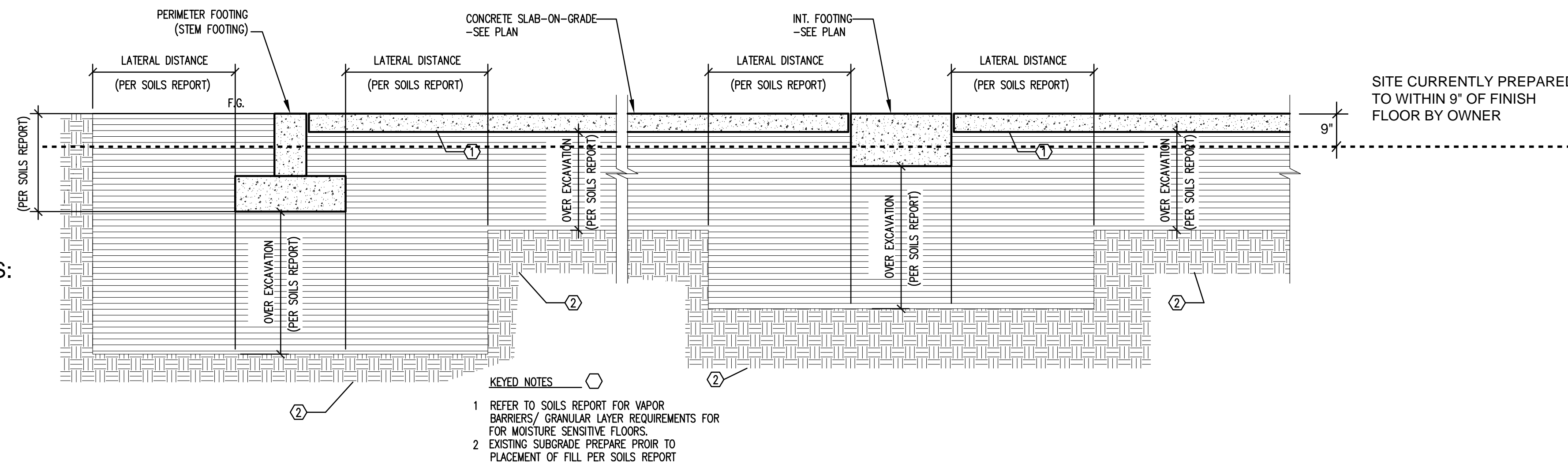
- THE GEOTECHNICAL ENGINEER SHALL ACT AS THE OWNER'S REPRESENTATIVE AND SHALL MAKE OBSERVATIONS AND TESTS AS CONSIDERED NECESSARY FOR QUALITY CONTROL WHERE FOUNDATIONS OR OTHER CRITICAL ELEMENTS ARE TO BE SUPPORTED ON ENGINEERED FILL. CONTINUOUS OBSERVATIONS AND TESTS OF GRADING OPERATIONS SHALL BE MADE BY THE GEOTECHNICAL ENGINEER. ALL TESTS SHALL BE PERFORMED IN ACCORDANCE WITH PROCEDURES SET FORTH IN THE CURRENT BOOK OF STANDARDS OF THE AMERICAN SOCIETY OF TESTING AND MATERIALS (ASTM).
- FILL OR BACKFILL CONSISTING OF SOIL APPROVED BY THE GEOTECHNICAL ENGINEER, SHALL BE PLACED IN CONTROLLED COMPACTED LAYERS WITH APPROVED COMPACTION EQUIPMENT. CONTROLLED COMPACTED LAYERS WITH APPROVED COMPACTION EQUIPMENT. ALL COMPACTION SHALL BE TO A MINIMUM OF 95 PERCENT (U.O.N. IN SOILS REPORT) OF THE MAXIMUM DRY DENSITY DETERMINED IN ACCORDANCE WITH ASTM D-1557 A TEST METHOD. SOILS MOISTURE CONTENT DURING COMPACTION SHALL BE AT OPTIMUM MOISTURE CONTENT PLUS OR MINUS 2% (U.O.N. IN SOILS REPORT).
- TESTS FOR DEGREE OF COMPACTION SHALL BE DETERMINED BY THE ASTM D-1556 OR ASTM D-2922 TEST METHODS. OBSERVATION AND FIELD TESTS SHALL BE CARRIED ON DURING FILL AND BACKFILL PLACEMENT BY THE GEOTECHNICAL ENGINEER TO ASSIST THE CONTRACTOR IN OBTAINING THE REQUIRED DEGREE OF COMPACTION. IF LESS THAN 95 (U.O.N. IN SOILS REPORT) PERCENT IS INDICATED, ADDITIONAL COMPACTION EFFORT SHALL BE MADE WITH ADJUSTMENT OF THE MOISTURE CONTENT AS REQUIRED COMPACTION IS OBTAINED.
- WHEREVER, IN THE OPINION OF THE GEOTECHNICAL ENGINEER, AN UNSTABLE CONDITION IS BEING CREATED, EITHER BY CUTTING OR FILLING, THE WORK SHALL NOT PROCEED IN THAT AREA UNTIL AN INVESTIGATION HAS BEEN MADE AND THE GRADING PLAN REVISED.

GENERAL DRAINAGE, MAINTENANCE AND PRECAUTIONS:

- PRECAUTIONS SHALL BE TAKEN DURING AND AFTER CONSTRUCTION TO MINIMIZE SATURATION OF THE FOUNDATION SOILS. POSITIVE DRAINAGE SHALL BE ESTABLISHED AWAY FROM THE EXTERIOR WALLS OF THE STRUCTURE (FOUR PERCENT GRADIENT WITHIN TEN FEET OF THE STRUCTURE). ALL UTILITY TRENCHES LEADING INTO THE BUILDING SHALL BE BACKFILLED WITH COMPACTED FILL. SPECIAL CARE SHALL BE TAKEN DURING INSTALLATION OF WATER LINES TO REDUCE THE POSSIBILITY OF FUTURE SUBSURFACE SATURATION.
- DEPRESSIONS AND EXCAVATIONS SHOULD BE BACKFILLED WITH COMPACTED, NON-SWELLING, RELATIVELY-IMPERVIOUS SOILS SUCH AS CLAYEY SANDS.
- GUTTERS AND DOWN SPOUTS SHOULD BE INSTALLED TO CONTROL ROOF DRAINAGE. DOWNSPOUTS SHOULD DISCHARGE A MINIMUM OF TEN FEET AWAY FROM STRUCTURES. AREA DRAINS MAY BE INSTALLED AROUND STRUCTURES TO IMPROVE DRAINAGE. DISCHARGE PIPES SHOULD SLOPE A MINIMUM OF 1/8 TH INCH VERTICAL PER FOOT OF HORIZONTAL PIPE. DRAINAGE SEWERS AND DISCHARGE CHANNELS SHOULD BE KEPT FREE OF DEBRIS.
- EXPANSION JOINTS WITHIN EXTERIOR CONCRETE FLATWORK SHOULD BE FILLED WITH A FLEXIBLE JOINT SEALER TO MINIMIZE WATER INFILTRATION.
- SOME MINOR CRACKING OF NEW CONCRETE FOUNDATIONS, CONCRETE FLATWORK, AND INTERIOR DRY WALL IS NORMAL. THIS IS A RESULT OF CONCRETE SHRINKAGE AS IT CURES. "SETTLING IN" OF THE NEW STRUCTURE, DRYING OF TIMBERS USED IN CONSTRUCTION, ETC. NORMALLY THE MAJORITY OF THIS MOVEMENT SHOULD CEASE WITHIN THE FIRST YEAR FOLLOWING CONSTRUCTION. HOWEVER, DEPENDING ON THE STRUCTURE AND SITE CONDITIONS, MOVEMENT MAY CONTINUE AT A SLOW RATE FOR SEVERAL YEARS.
- ROOF GUTTERS AND DOWNSPOUTS SHOULD BE UTILIZED. DOWN SPOUTS SHOULD DISCHARGE DOWN SLOPE AND WELL AWAY FROM THE STRUCTURES. SPLASH BLOCKS SHOULD BE UTILIZED BELOW DOWN SPOUTS. SURFACE WATER SHOULD RUN OFF RAPIDLY.
- LANDSCAPING ADJACENT TO STRUCTURES SHOULD BE DESIGNED AND CONSTRUCTED TO MINIMIZE THE POTENTIAL FOR WETTING OF SOILS SUPPORTING THE PROPOSED FACILITIES. IT IS NOT RECOMMENDED THAT LANDSCAPING REQUIRING IRRIGATION BE INSTALLED CLOSER THAN FIVE FEET OF STRUCTURES. TREES AND SHRUBS SHOULD BE HAND WATERED OR WATERED USING CONTROLLED DRIP IRRIGATION. IF DRIP IRRIGATION IS USED, EMITTERS SHOULD DISCHARGE NO MORE THAN ONE GALLON PER HOUR. WATERING SHOULD BE CAREFULLY CONTROLLED TO PREVENT OVER WATERING. GRASSED AREAS ADJACENT TO STRUCTURES SHOULD BE SLOPED SO THAT EXCESS IRRIGATION WATER WILL RUN OFF PROMPTLY. SPRINKLER LINES AND DRIP IRRIGATION MAINS SHOULD BE LOCATED A MINIMUM OF FIVE FEET AWAY FROM FOUNDATIONS. MOWING STRIPS, PLANTERS AND SIDEWALKS SHOULD NOT "DAM" WATER ADJACENT TO STRUCTURES.



1 GENERAL TRENCH REQUIREMENTS PARALLEL TO FOUNDATION

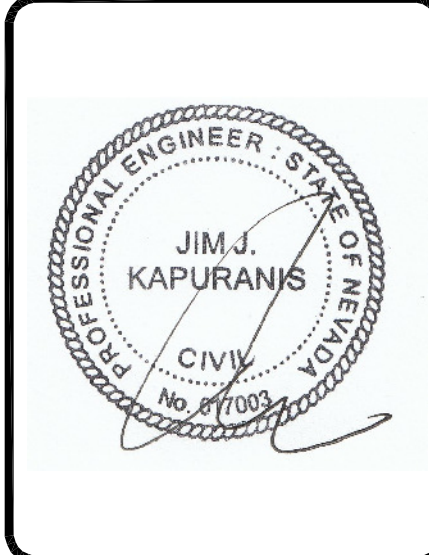


2 EARTHWORK REQUIREMENTS

- THE ABOVE EARTHWORK INFORMATION IS BASED ON THE SOILS REPORT PROVIDED. THIS DOES NOT RELIEVE THE CONTRACTOR FROM THE COMPLETE REVIEW OF THE SOILS REPORT AND ALL REQUIREMENTS/RECOMMENDATIONS OF SUCH REPORT MUST BE FOLLOWED.
- REFER TO CIVIL DRAWINGS FOR ADDITIONAL SUBGRADE PREPARATION

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EARTHWORK GENERAL NOTES AND DETAILS

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OF

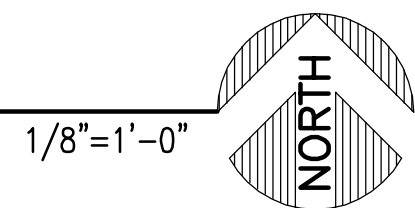
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Consulting Structural Engineers

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July 22, 2015 - 2:39 PM - Yanni Yanni
 Drawing Name: P:\223\2015\682\Drawings\JKK_Drawing\7-22-15.dwg
 Drawing Date: 7/22/15

1 FOUNDATION PLAN

REFER TO ARCH. DIMENSION PLAN FOR ALL DIMENSIONS NOT SHOWN. COORDINATE ALL ARCHITECTURAL DIMENSIONS/SLAB ELEVATIONS PRIOR TO ANY EXCAVATIONS. **DO NOT SCALE DRAWINGS.**



GENERAL NOTES :

- REFER TO GENERAL NOTES SHEETS FOR ALL REQUIREMENTS.
- AT NON-BEARING PARTITION WALLS PROVIDE 0.145" DIAMETER SHOT-PINS OF SUFFICIENT LENGTH FOR 1-3/8" PENETRATION INTO CONCRETE SPACED AT NO MORE THAN 16" O.C. FROM SILL PLATE INTO SLAB AT BASE LEVEL.
- REFER TO MECH./ELEC. FOR ALL INSERTS AND DRAIN LOCATIONS.
- REFER TO ARCH. FOR LOCATION OF FLOOR RECESSES.
- REFER TO CIVIL PLANS FOR FOOTING STEP LOCATIONS
- REFERENCE TYPICAL DETAILS ON SHEET S0.2 AS FOLLOWS:
 STANDARD HOOKS/BAR BENDS
 TYPICAL REINFORCING LAP SPICE SCHEDULE
 TYPICAL ANCHOR BOLT EMBEDMENT SCHEDULE
 REINFORCEMENT MINIMUM COVER/TOLERANCES

KEYED NOTES :

- 4" CONC. SLAB-ON-GRADE W/ #4 @ 18" O.C. E.W. @ MID-DEPTH OVER PREPARED SUBGRADE PER SOILS REPORT.
- TRENCH DRAIN -SEE MECH & 12/S300
- HSS COL TO FOOTING -SEE 10/S300
- SPREAD FOOTING INTEGRAL W/ FOOTING -SEE 3A/S300
- EXTERIOR WOOD COL -SEE 11/S300
- EXTERIOR CONC. SLAB ON GRADE W/ TURNDOWN -SEE S0.2
- 6" CMU LOW WALL -SEE 4A/S300
- WOOD COL ON STEM (IN WALL) PROVIDE SIMPSON "ABA" POST BASE

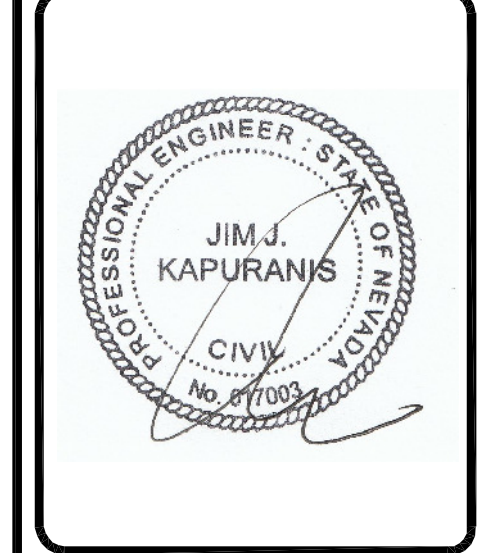
LEGEND

- DRAIN LOCATIONS - SEE ARCH. & MECH.
- CMU WALL
- DEPRESSED STEM @ DOOR OPENINGS
 -SEE 2/S300 FOR CONC. STEM
 -SEE 7/S300 FOR BLOCK STEM
- H.D. HOLD/DOWN SEE SHEARWALL SCHED AND PLAN S201



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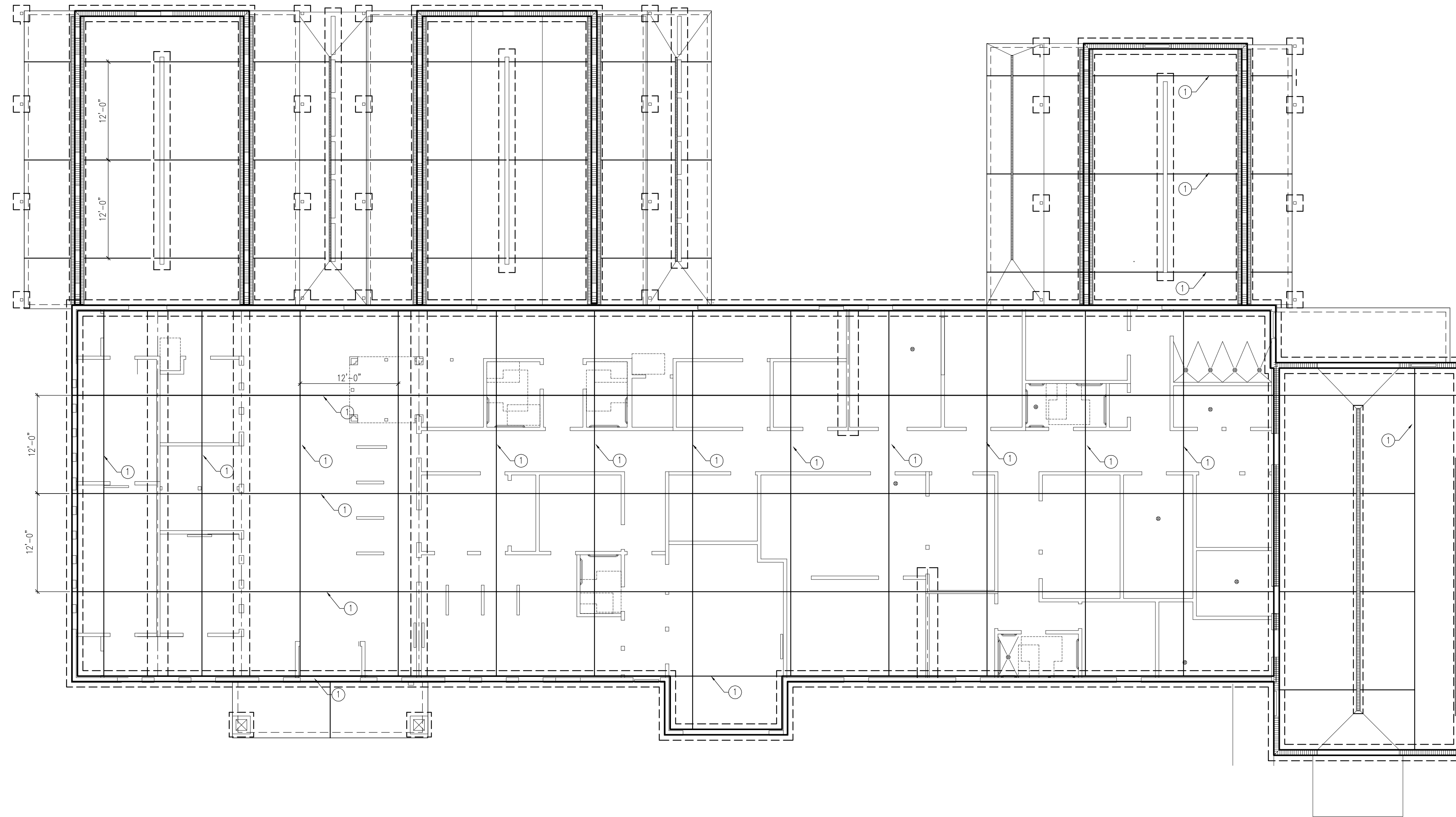
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GENERAL NOTES :

1. REFER TO GENERAL NOTES SHEETS FOR ALL REQUIREMENTS.
2. REFERENCE DETAILS ON SHEET SO.2A AS FOLLOWS: TYPICAL JOINT LAYOUT DETAILS
CONTROL/CONTRACTION JOINT DETAILS ISOLATION/EXPANSION JOINT DETAILS
CONSTRUCTION JOINT DETAILS TYPICAL REINFORCEMENT AT RE-ENTRANT CORNERS
TYPICAL REINFORCING AROUND SLAB-ON-GRADE OPENINGS

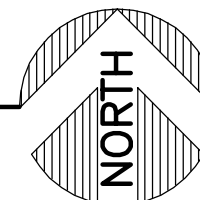
KEYED NOTES : ○

1. CONTROL JOINT – SEE SO.2A FOR DETAILS.



1 FOUNDATION CONTROL JOINT PLAN

1/8"=1'-0"



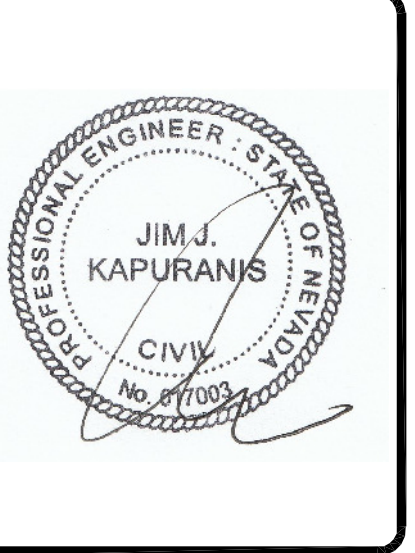
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FOUNDATION CONTROL JOINT PLAN

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GENERAL NOTES :

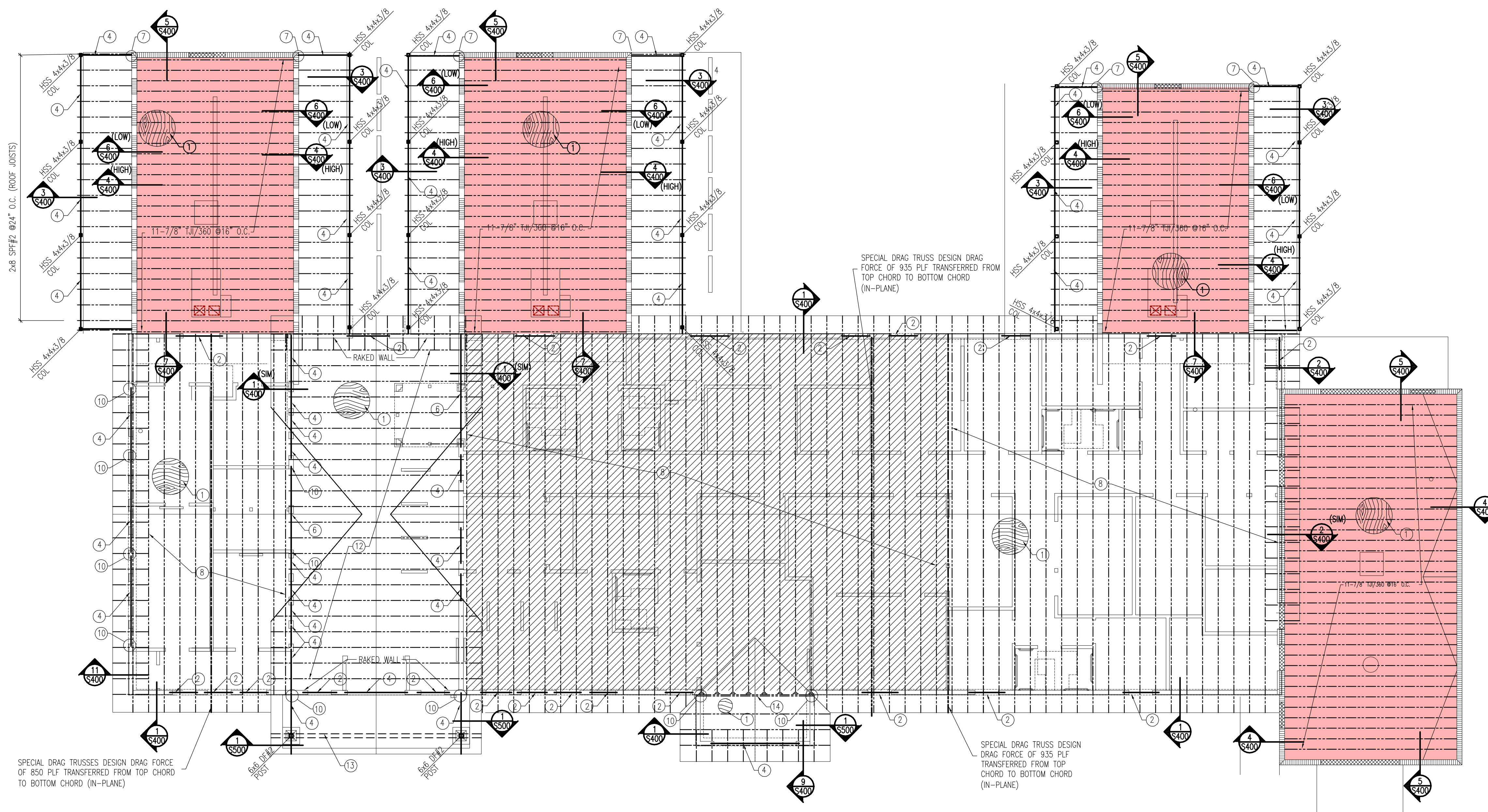
1. REFER TO GENERAL NOTES SHEETS FOR ALL REQUIREMENTS.
2. REFER TO ARCH. FOR ALL BEARING HEIGHTS.
3. CONNECTION HARDWARE TO BE SIMPSON U.O.N.
4. REFER TO SHEET S0.3A FOR WOOD COLUMN POST CAP BASE CONNECTIONS.
5. ALL TRUSSES ARE TO BE PRE-ENGINEERED TRUSSES

KEYED NOTES :

1. 5/8" APA RATED ROOF SHEATHING -NAIL W/10d AT 6" O.C.
2. EDGE/BOUNDARY U.O.N. ON SHEARWALL PLAN S201
3. (2)2x10 DF#2 BUILT-UP WOOD HEADER
4. 5x4x14" PSL BEAM
5. (2)2x12 DF#2 BUILT-UP WOOD HEADER
6. (2)2x8 DF#2 BUILT-UP WOOD HEADER
7. 5x4x11 1/2" PSL BEAM
8. SIMPSON MRHJUS-56 MASONRY HANGER
9. PRE-ENGINEERED WOOD TRUSSES @24" O.C.
10. BUILT-UP WOOD TRUSSES @60" O.C. -SEE S500
11. (3)2x BUILT-UP STUDS (IN WALL)
12. 9-1/2" TJI/210 ROOF JOISTS @24" O.C.
13. PRE-ENGINEERED WOOD SCISSOR TRUSSES @24" O.C.
14. TIMBER TRUSSES- SEE 1/S500 FOR DETAILS
15. PRE-ENGINEERED WOOD GIRDER TRUSS

LEGEND :

- 8" CMU WALL
- MASONRY LINTEL
- FLAT ROOF AREA
- AREA OF BLOCKED DIAPHRAGM -SEE S201

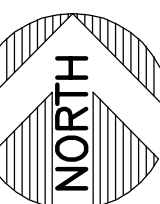


1 ROOF FRAMING PLAN

S200

REFER TO S0.3A FOR ALL WOOD COLUMN TO WOOD BEAM CONNECTIONS

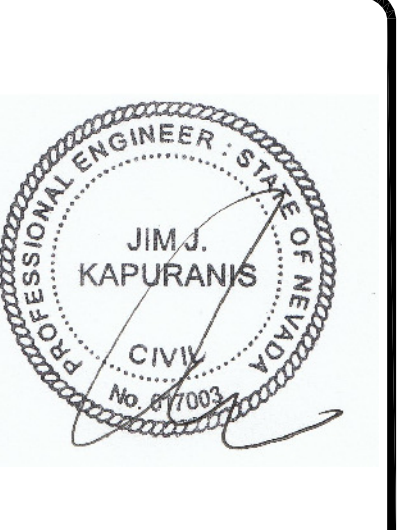
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ROOF FRAMING PLAN

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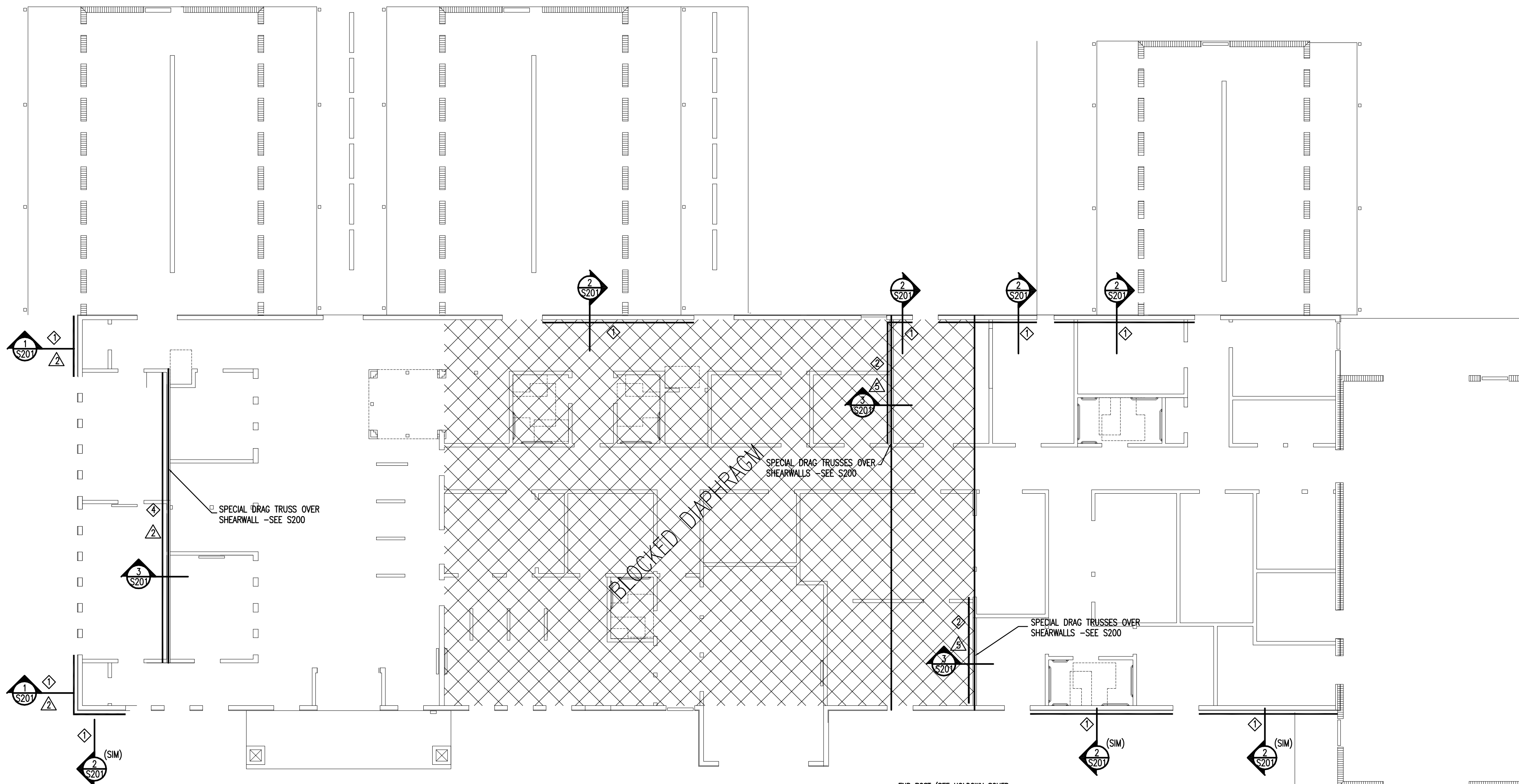
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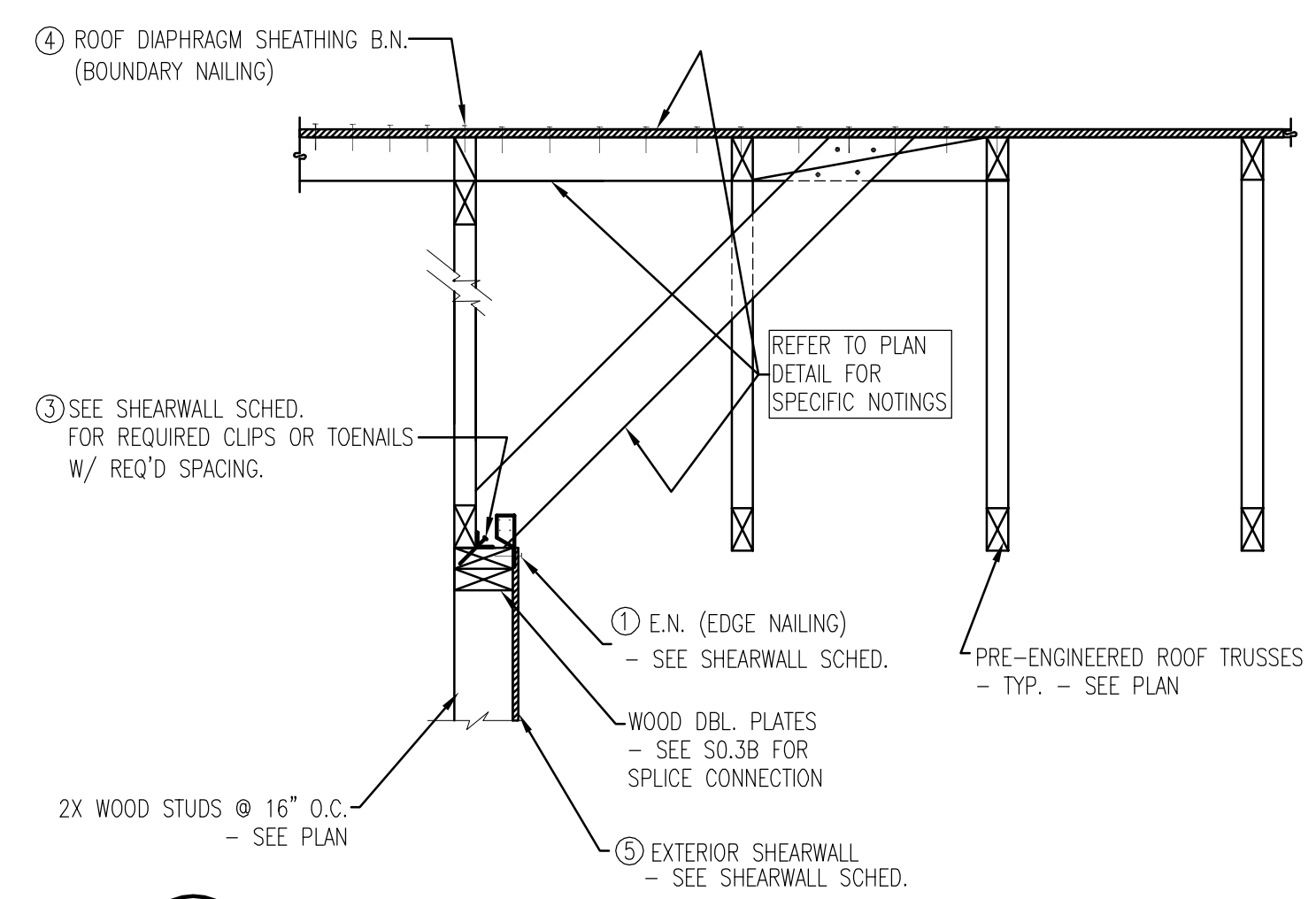
1 SHEARWALL PLAN
S201

GENERAL NOTES :

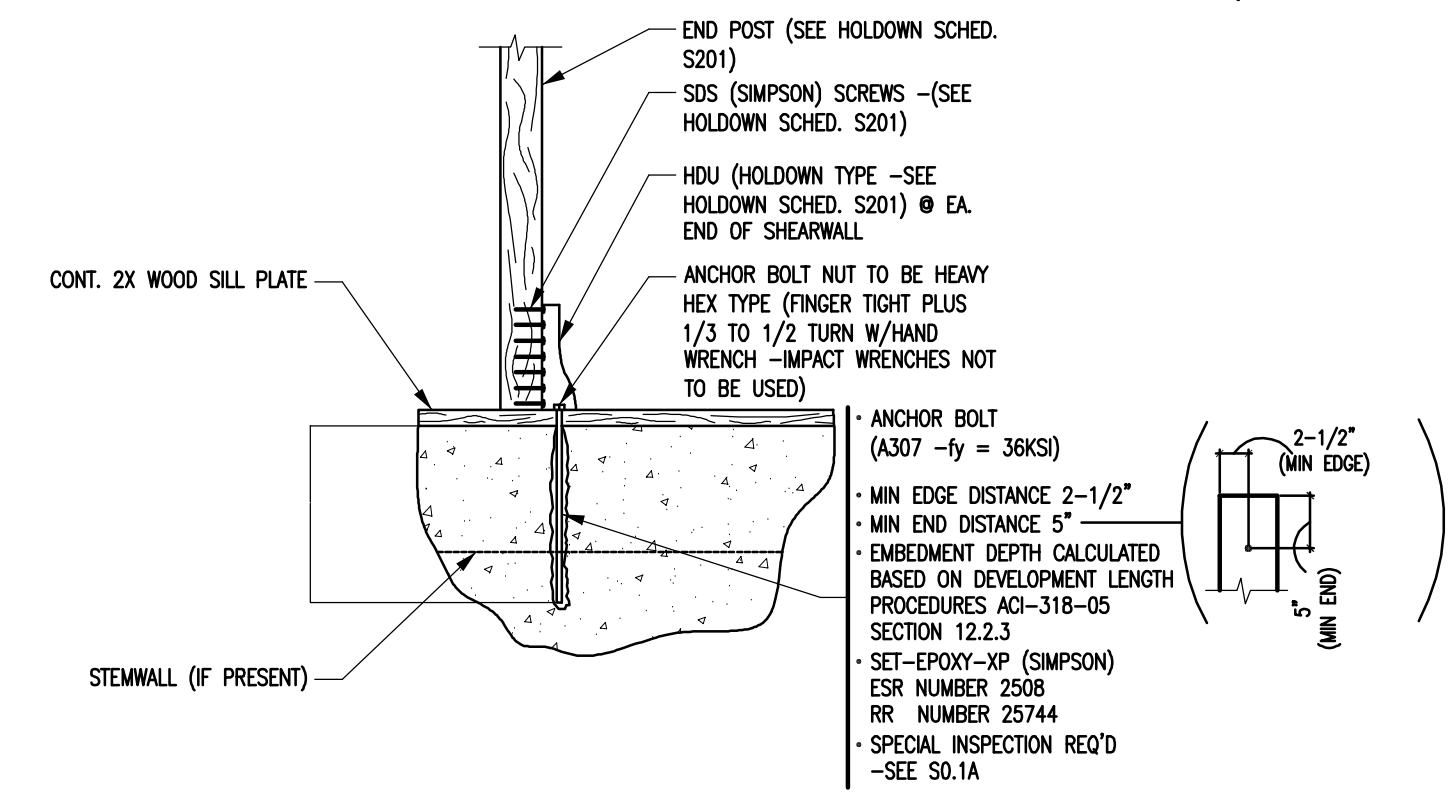
- REFER TO GENERAL NOTES SHEETS FOR ALL REQUIREMENTS.
- REFER TO ARCH. FOR ALL BEARING HEIGHTS.
- CONNECTION HARDWARE TO BE SIMPSON U.O.N.
- REFER TO SHEET S0.3A FOR WOOD COLUMN POST CAP BASE CONNECTIONS.
- ALL TRUSSES ARE TO BE PRE-ENGINEERED TRUSSES

LEGEND :

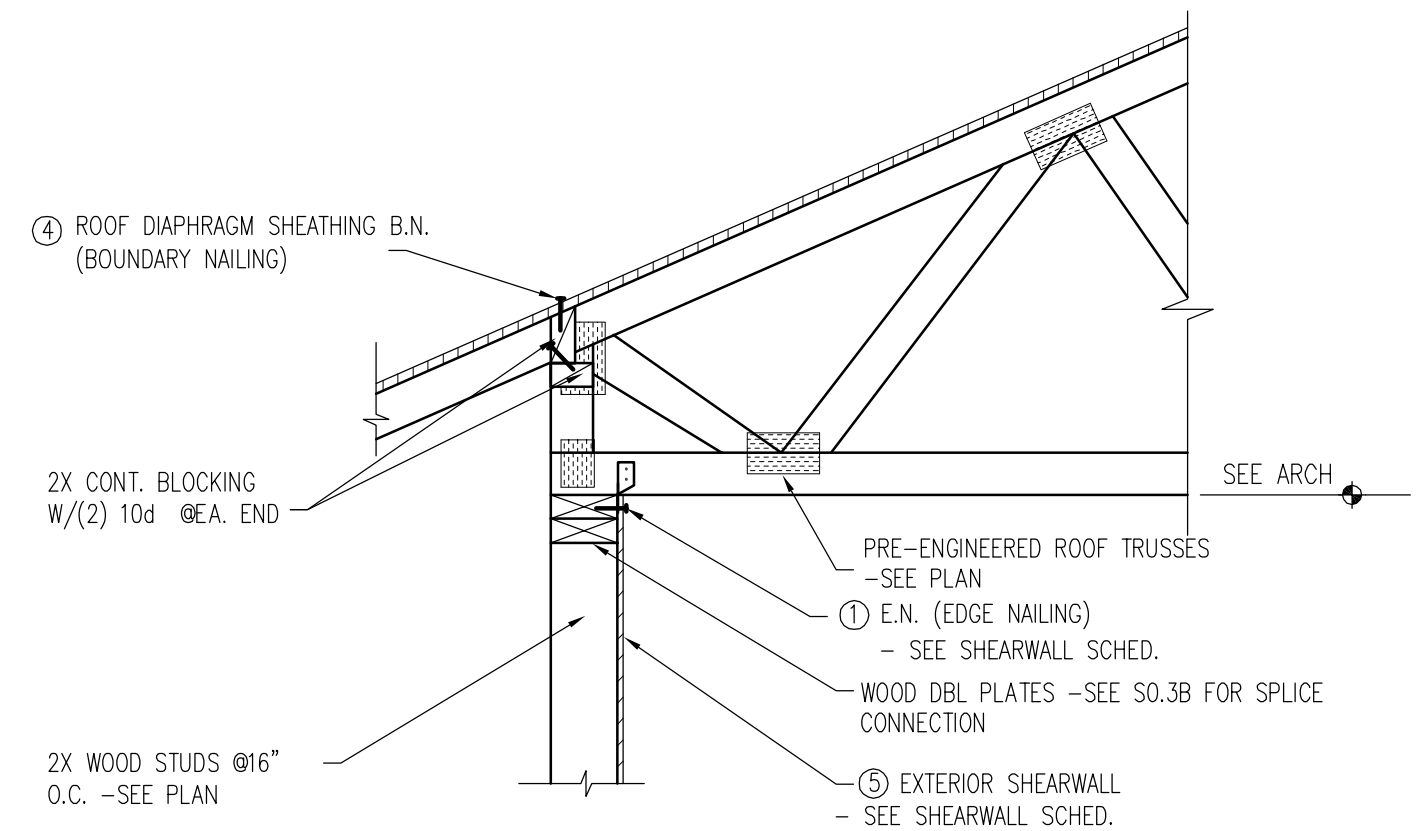
- WOOD STUD (METAL STUD) SHEARWALL SHEATHING/REQUIREMENTS - SEE SCHED. (THIS SHEET) AND SHEET S0.3D FOR GENERAL DETAILS AND REQUIREMENTS
- HOLDOWN TYPE - SEE SCHED. (THIS SHEET) AND SHEET S0.3D FOR DETAILS
- LATERAL FORCE RESISTING ELEMENT LOCATION AND LENGTH



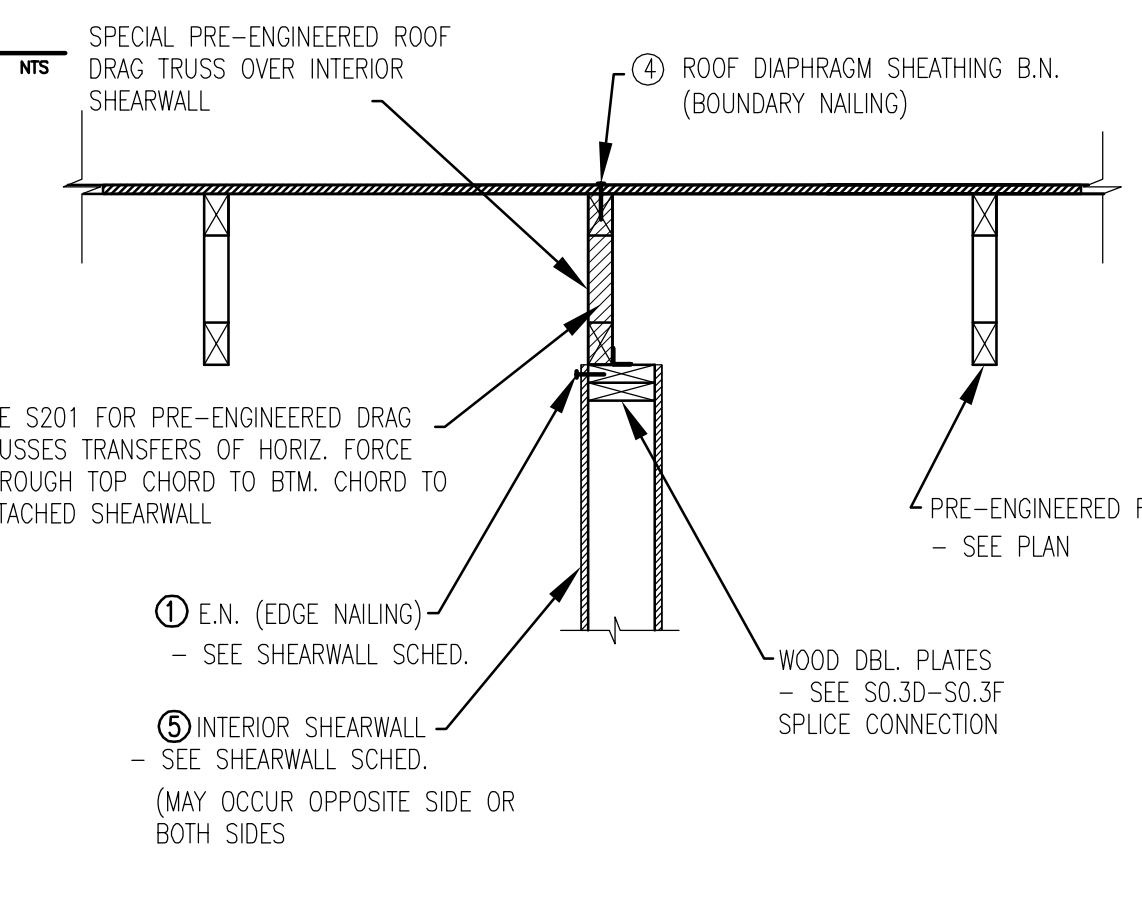
1 SECTION
S201



DRILLED AND EPOXIED HOLDOWN ANCHOR SYSTEM IN CONCRETE



2 SECTION
S201



3 SECTION
S201

(Holdown Schedule)
(Simpson Strong-Tie Holdown to solid concrete foundation)

HD	Type	End Post type	Fastener to Stud	*Anchor Bolts (A307) Simpson "SET" SYSTEM	**Cap (Kips)
1	HDU2	(2) 2x	6-SDS 1/4"x2-1/2"	5/8"Ø EMBED = 12"	3.1
2	HDU4	(2) 2x	10-SDS 1/4"x2-1/2"	5/8"Ø EMBED = 12"	4.6
3	HDU5	(2) 2x	14-SDS 1/4"x2-1/2"	5/8"Ø EMBED = 12"	5.6
4	HDU8	6x	20-SDS 1/4"x2-1/2"	7/8"Ø EMBED = 17"	7.9
5	HDU11	6x	30-SDS 1/4"x2-1/2"	1"Ø EMBED = 20"	9.6

*Set-Epoxy (Simpson Anchor System)
**Capacity based on lesser allowable value of holdown hardware, A307 Anchor Bolt embedment depth (based on development length procedures (ACI-318-12.2.3) or epoxy bond strength)

NOTES :
1. Values are based on DF#2 or Southern Pine species end post type. If other species is used, notify engineer for holdown capacity reductions and re-calculations.
2. f'c = 3,000 psi (MIN)

HOLDOWN SCHEDULE AT FOUNDATION LEVEL

(REFER TO SHEARWALL PLANS)

(Roof Diaphragm Schedule)
(5/8" RATED Sheathing w/ 10d Common)

ID	Diaphragm Case	Bndry (in o.c.)	Edge	Allowable Shear (Klf)	Notes
	BLOCKED	2-1/2	2-1/2	640	1
	UNBLOCKED	6	6	285	1

TABLE LEGEND:
① = TAGGED NOTES ON FRAMING SECTIONS BELOW
Klf = KIPS/FT
o.c. = ON CENTER SPACING

(Diaphragm Notes)
GENERAL NOTES

- Fasteners shall be driven flush with surface of sheathing.
- Space nails @ 12 in o.c. along intermediate framing members.
- Refer to Diaphragm Nailing Plan-This sheet

SPECIAL NOTES (apply to diaphragms specifically noted)
1) APA Rated Sheathing, STURD-I-FLOOR EXP1/EXP2/EXT or C-C/C-D Plywood.

WAFERBOARD AND ORIENTED STRAND BOARD CONFORMING TO NER-108 AND PRODUCT STANDARD 2-92, AND WITH THE SAME EXPOSURE DURABILITY CLASSIFICATION, NOMINAL THICKNESS AND SPAN/INDEX RATIO MAY BE SUBSTITUTED FOR PLYWOOD ONLY IF APPROVED BY THE STRUCTURAL ENGINEER.

WOOD HORIZONTAL DIAPHRAGM SCHEDULE

(REFER TO SHEARWALL PLANS)

ALLOW. SHEAR VALUES ARE ONLY BASED ON SOUTHERN PINE OR DOUG FIR SPECIES. IF OTHER SPECIES OF WOOD IS PROPOSED, THE ENGINEER MUST BE NOTIFIED FOR RECALCULATION FOR REDUCED ALLOW. SHEAR VALUES.

(ShearWall Schedule - 16" O.C. MAX. SPACING OF STUDS)

SW	⑤ SHEARWALL Sheathing (Thick side)	① Nailing (in o.c.)	Anchor Bolts (in o.c.)	② Bottom Plate (in o.c.)	③ Rim/Blk to Blk Top plates (in o.c.)	Allow Shear (Klf)	Notes
1	SURE-BOARD SERIES 200-W	10d @ 2	5/8" Ø 40	16d @ 5	16d @ 3T	657	
2	SURE-BOARD SERIES 200-W BOTH SIDES	10d @ 2	5/8" Ø 16	2-16d @ 3	(2)A35Ø9	0.92	1,2,4

TABLE LEGEND:
= TOENAIL
= TAGGED NOTES ON FRAMING SECTIONS BELOW
Klf = KIPS/FT
o.c. = ON CENTER SPACING

(SHEARWALL Notes)
GENERAL NOTES (apply to all SHEARWALLS)

- Space nails @ 12 in o.c. along intermediate framing members.
- Block all panel edges with minimum 2x blocking (unless 3x blocking is req'd).
- Apply nailing to all studs, top and bottom plates and blocking.
- Framing to be a maximum of 16 in o.c.
- Fasteners shall be driven flush with surface of sheathing.
- Panels may be installed either Horiz. or Vert. for plywood. Horiz. only if "OSB" is used.
- All nails to be common type.
- Oriented Strand Structural Panels (OSD) may be substituted for plywood ONLY conforming to NER-180 product standard 2-92 and ONLY with panels applied with long dimension across studs.
- 1-5/8" min nail penetration to framing members.

SPECIAL NOTES FOR SHEARWALLS (apply to walls specifically noted)

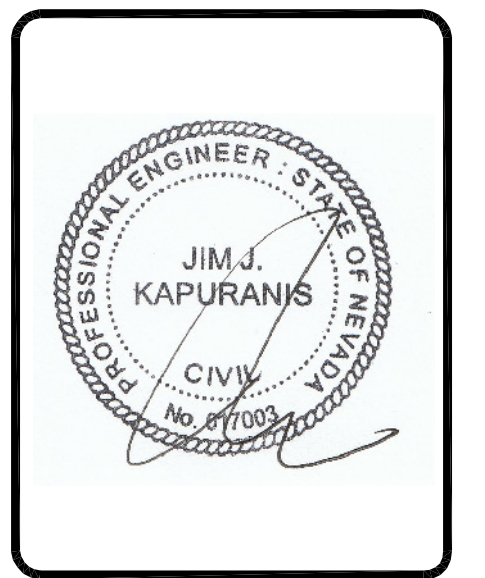
- SURE-BOARD SERIES 200W
- Provide 3x s at adjoining panel edges w/nails staggered or (2) 2x studs edge nailed together per schedule. At foundation level, provide a 3x sill plate or (2) 2x single plates edge nailed together per schedule (staggered).
- Provide 3-inch nominal or thicker blocking beneath bottom plate for full length of shearwall.

WOOD STUD SHEARWALL SCHEDULE

(REFER TO SHEARWALL PLANS)

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SHEARWALL PLAN

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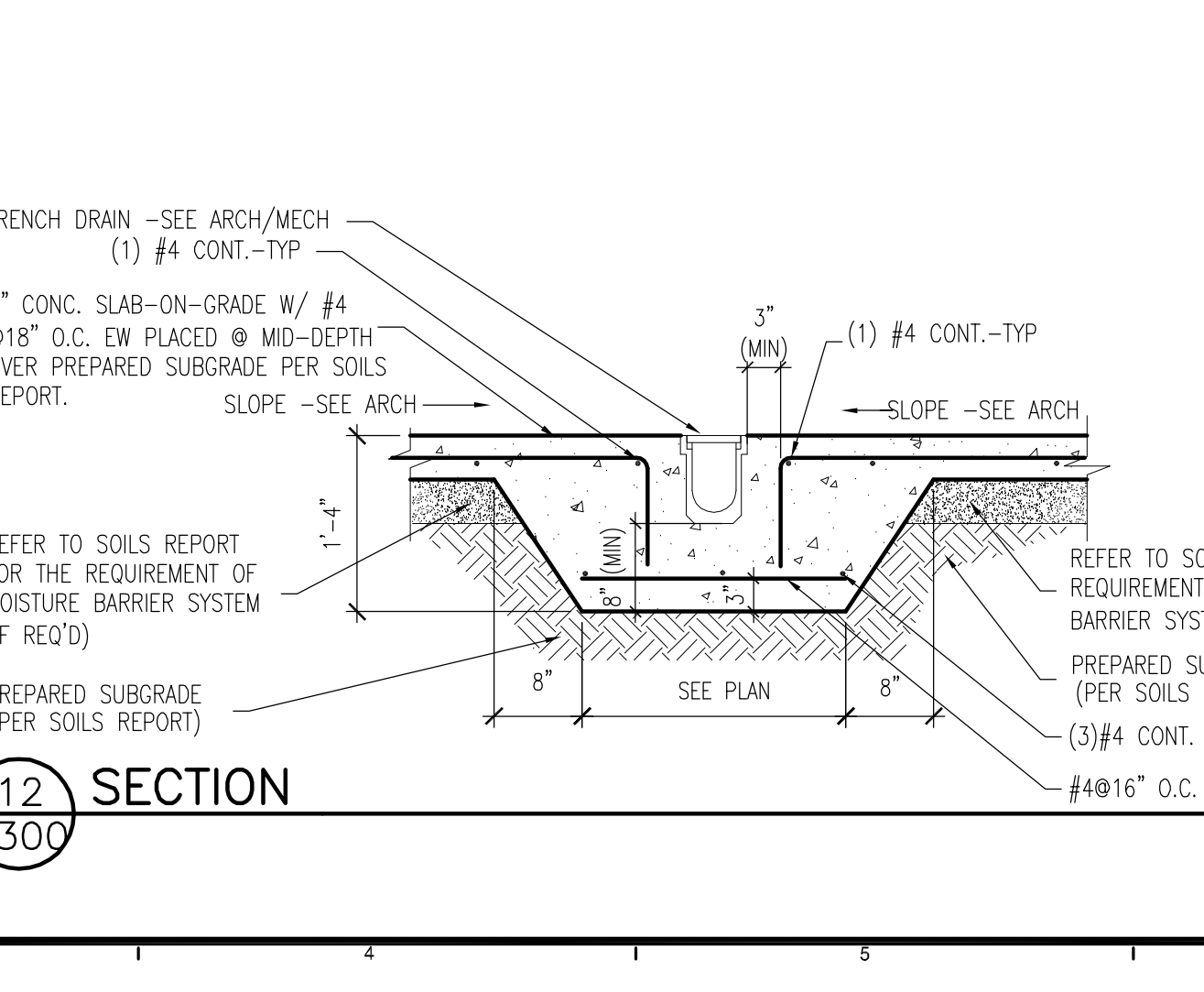
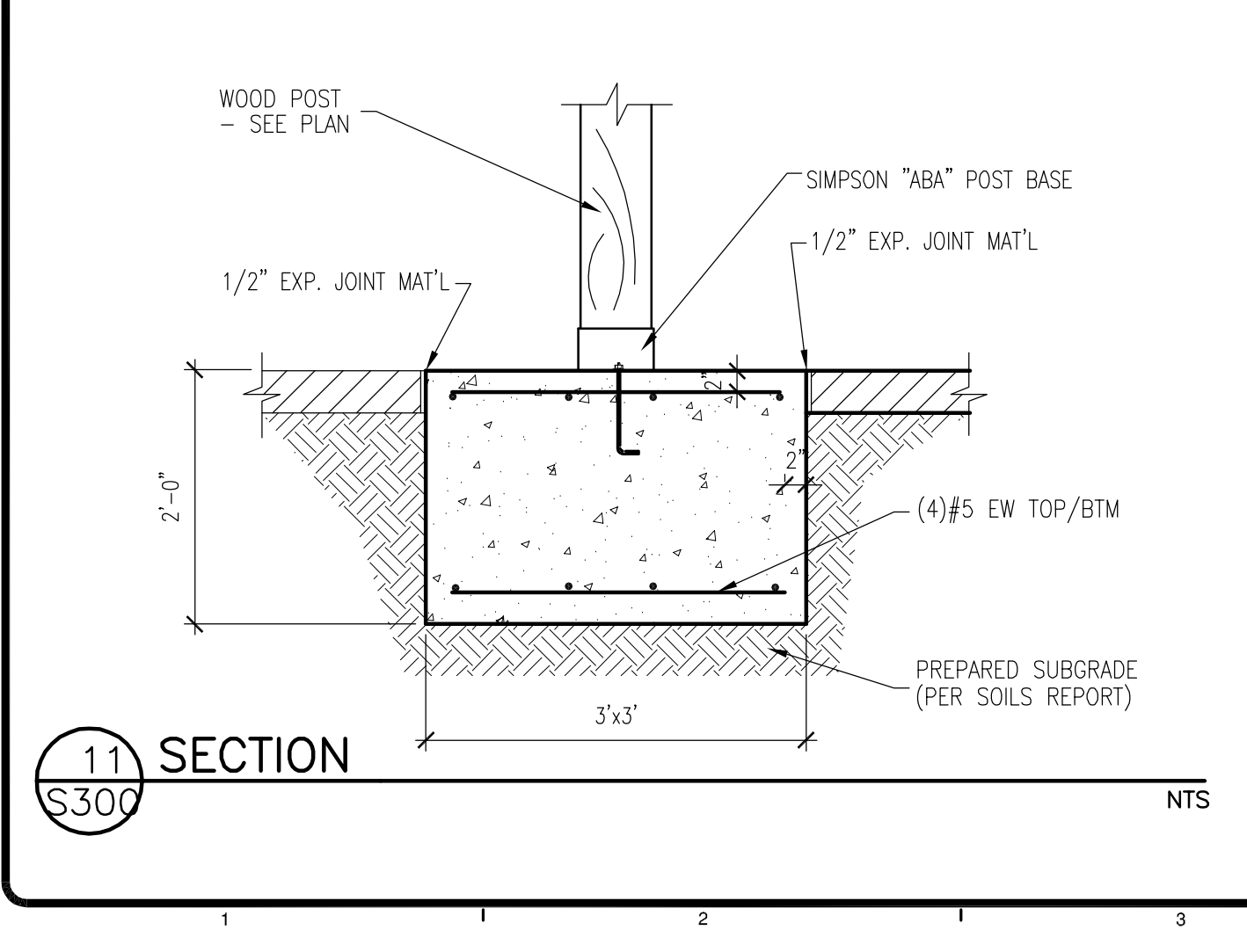
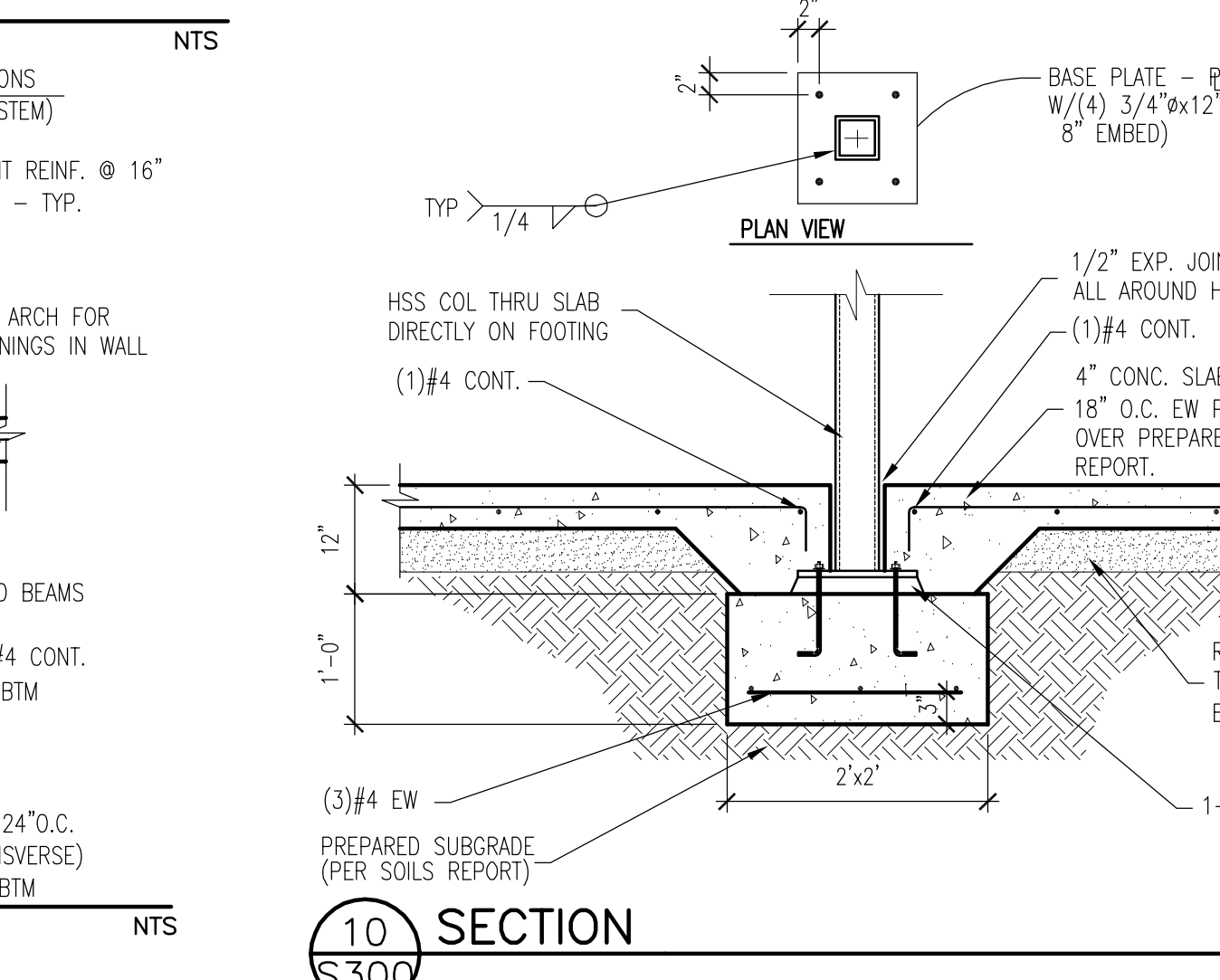
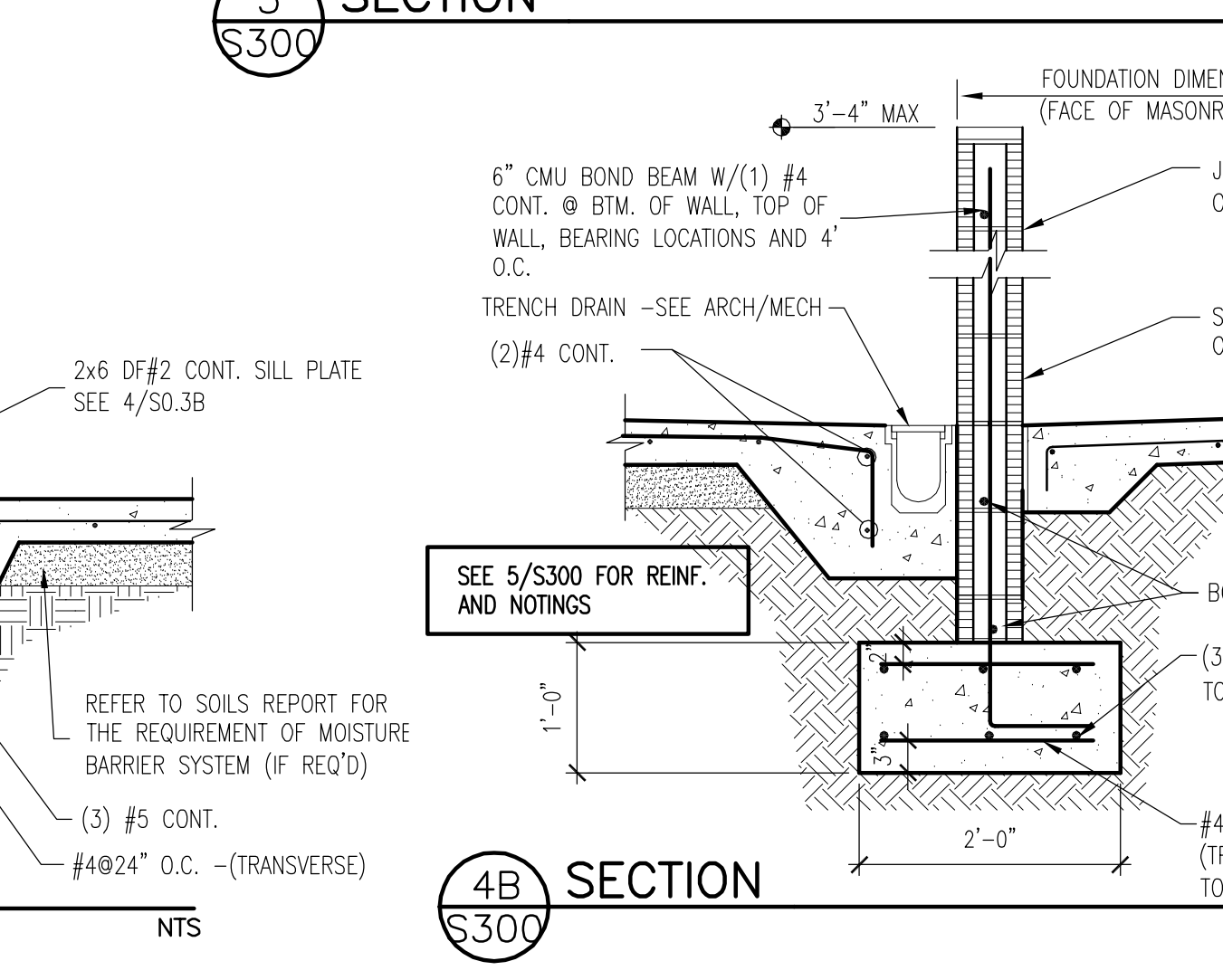
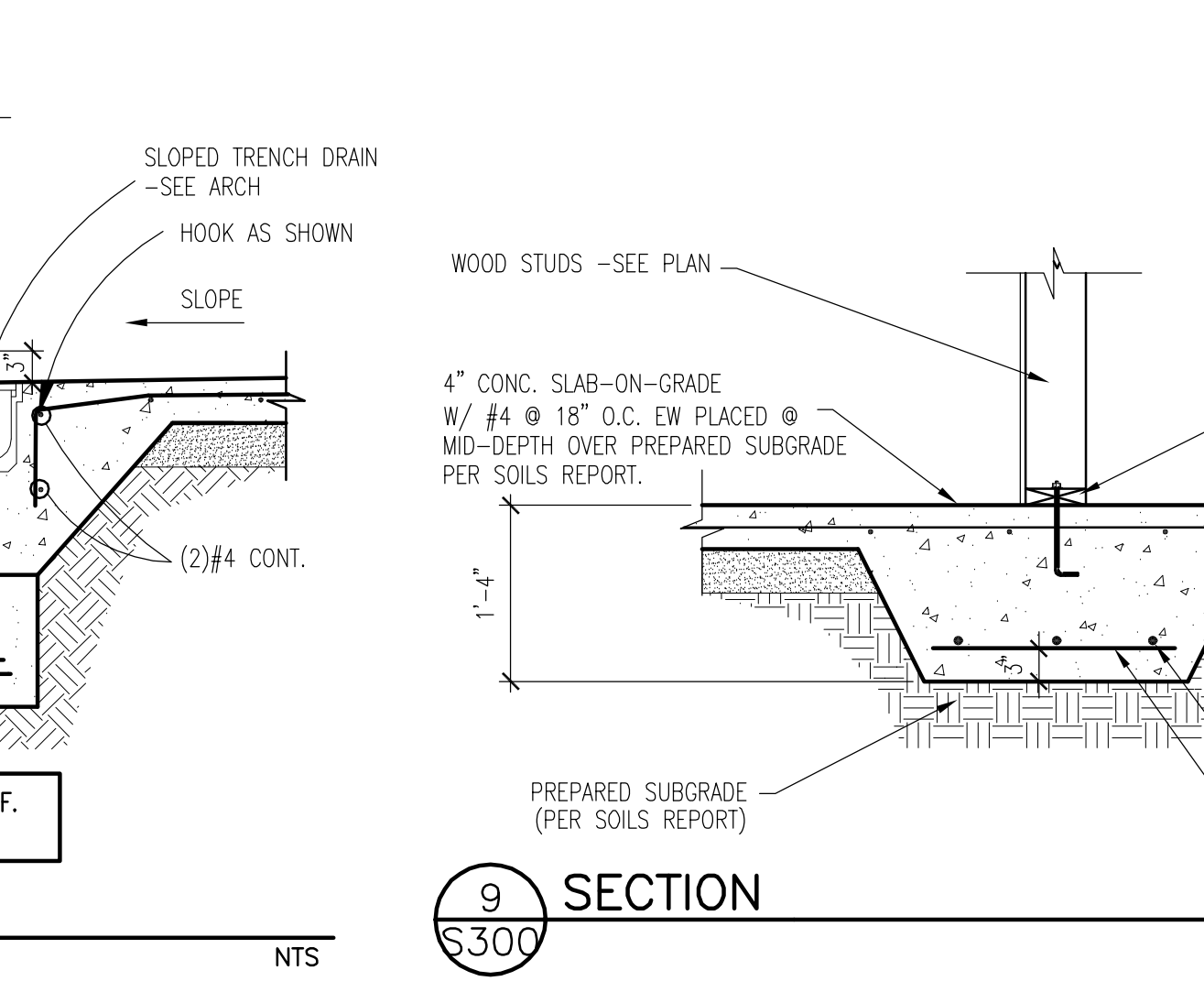
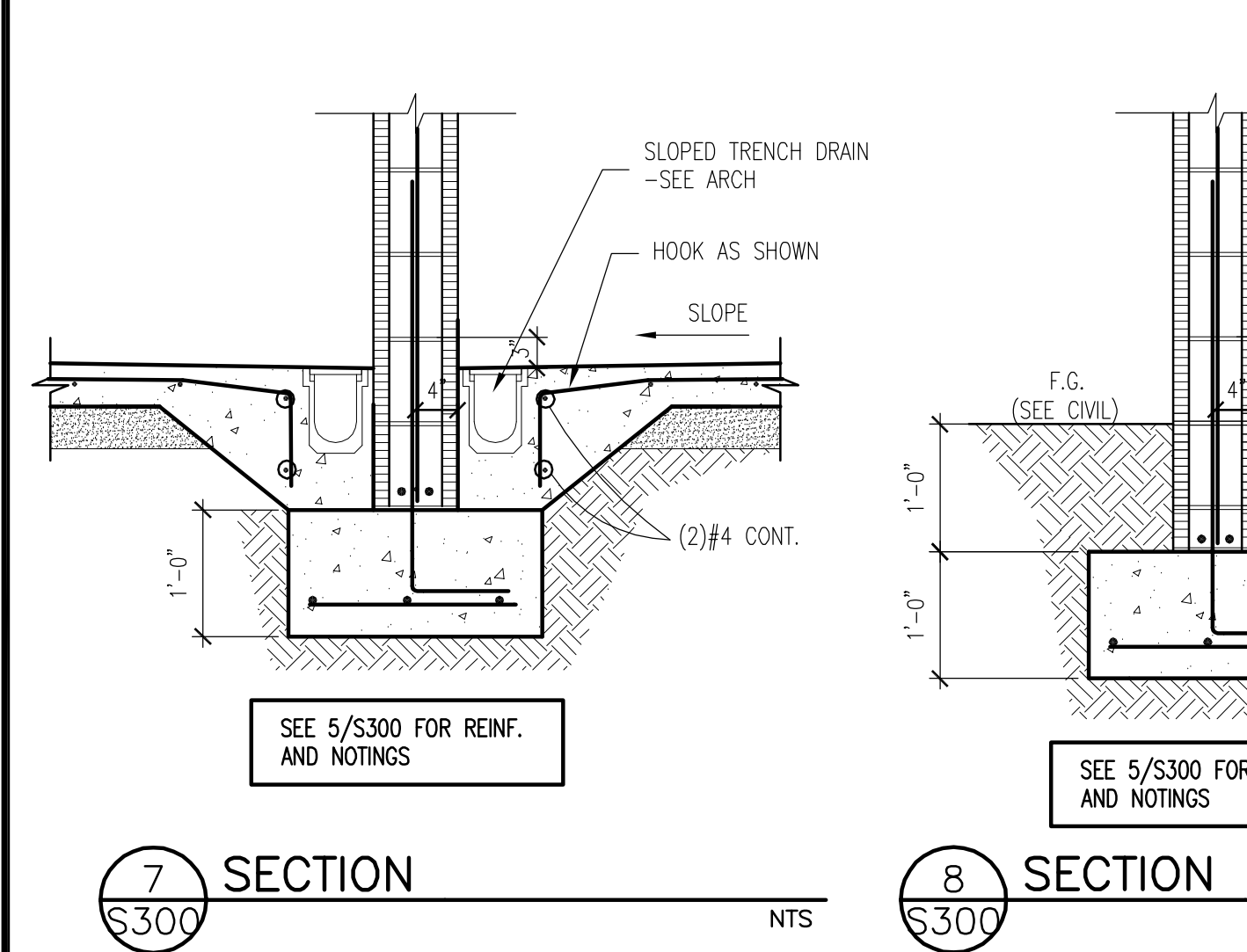
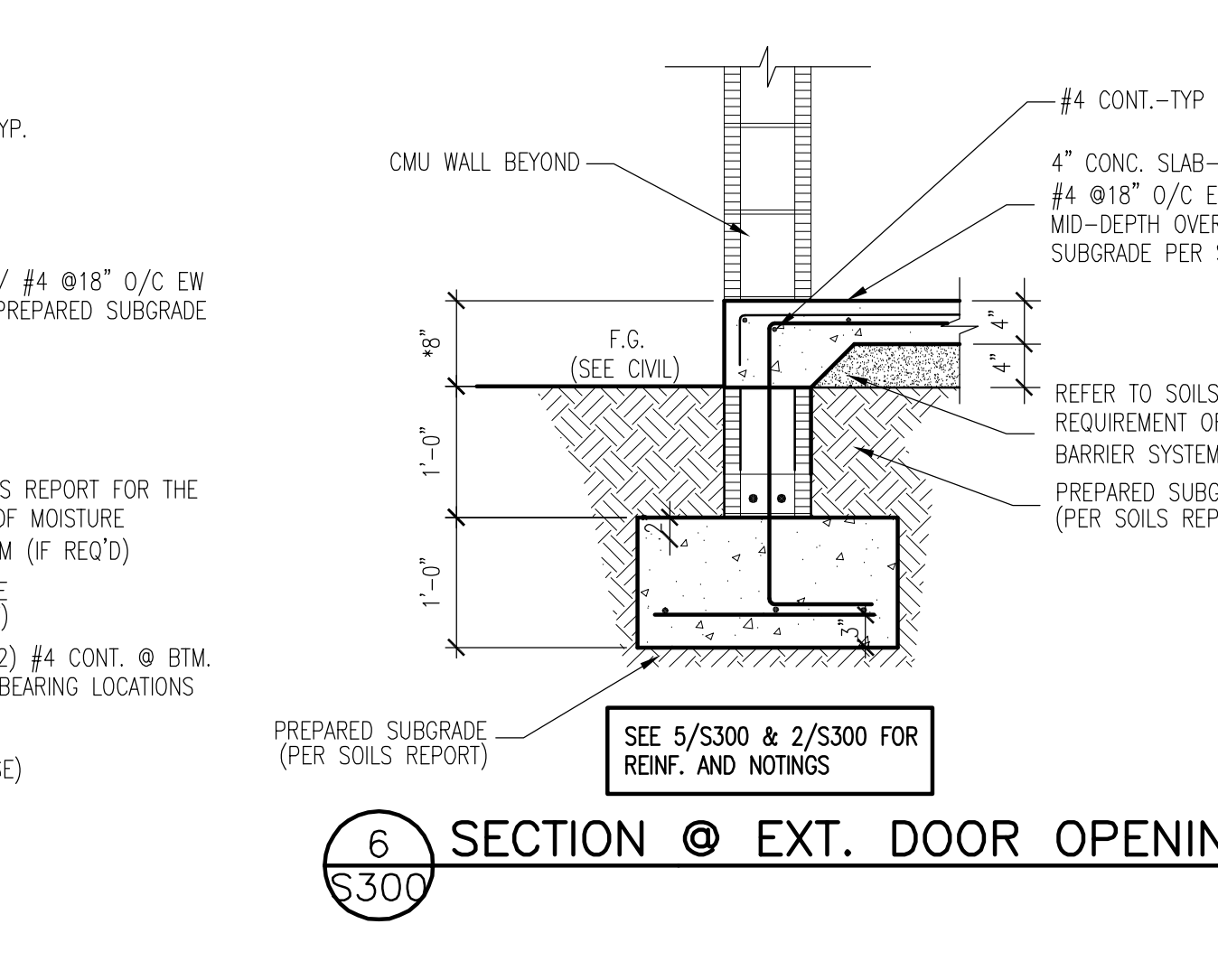
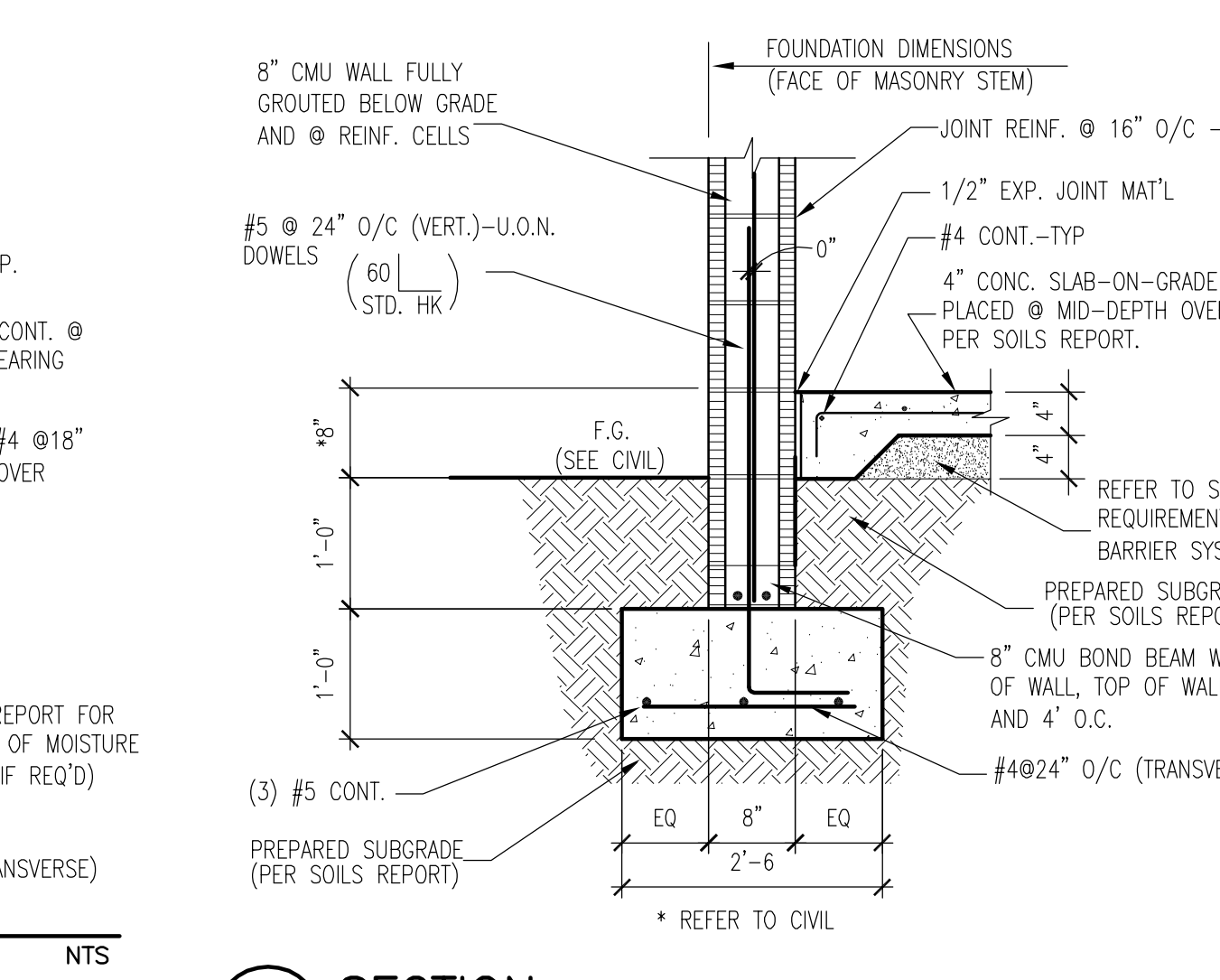
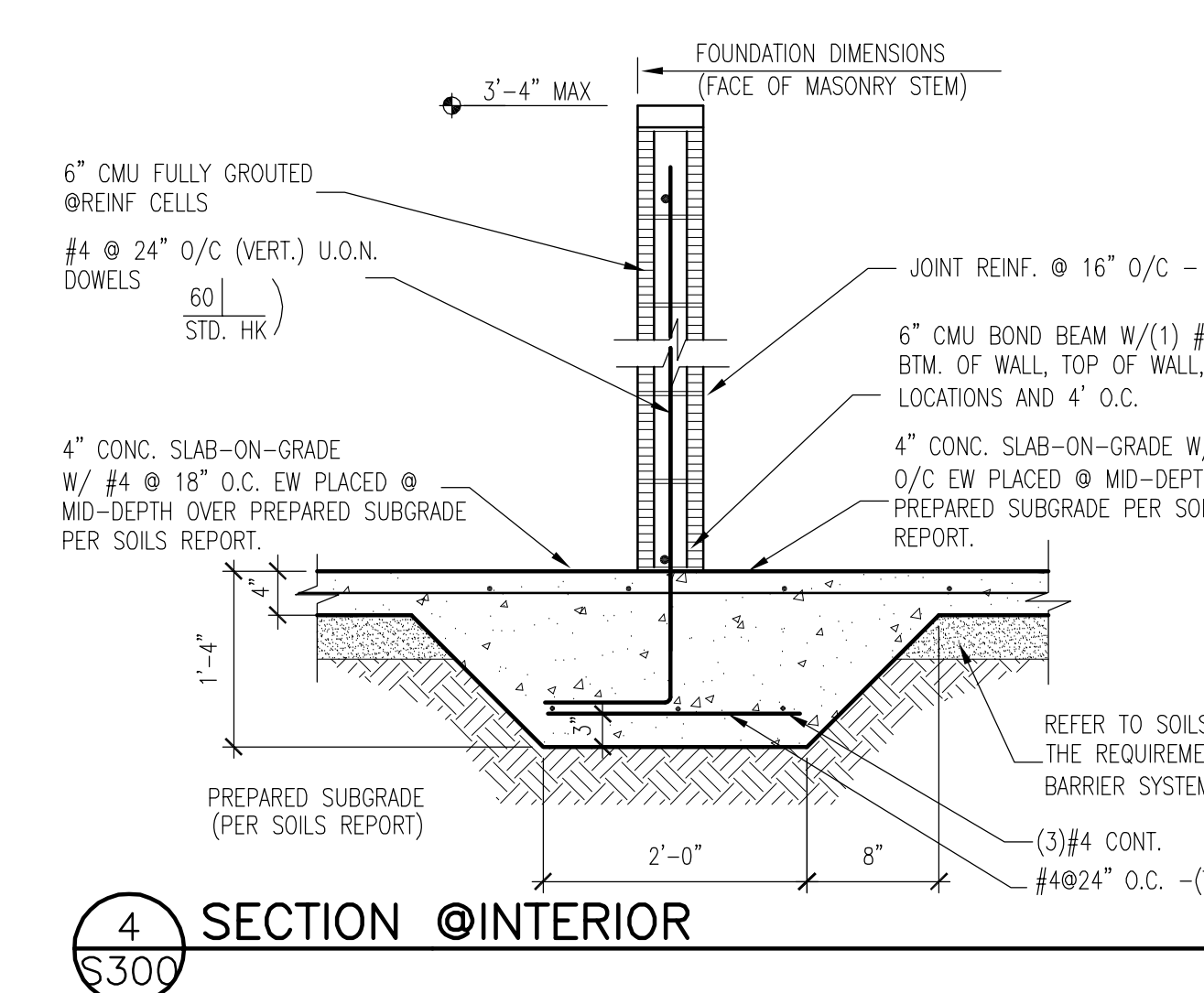
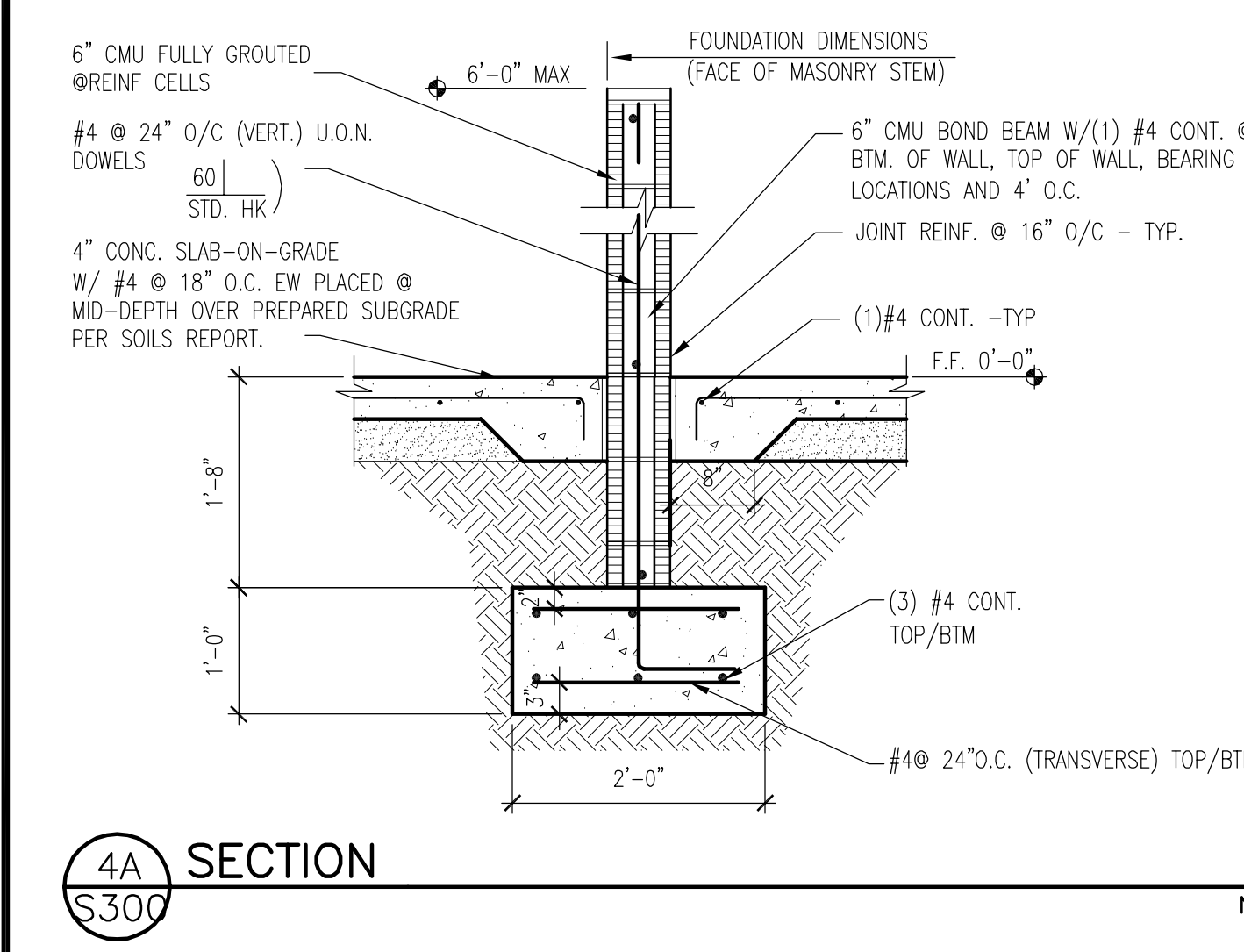
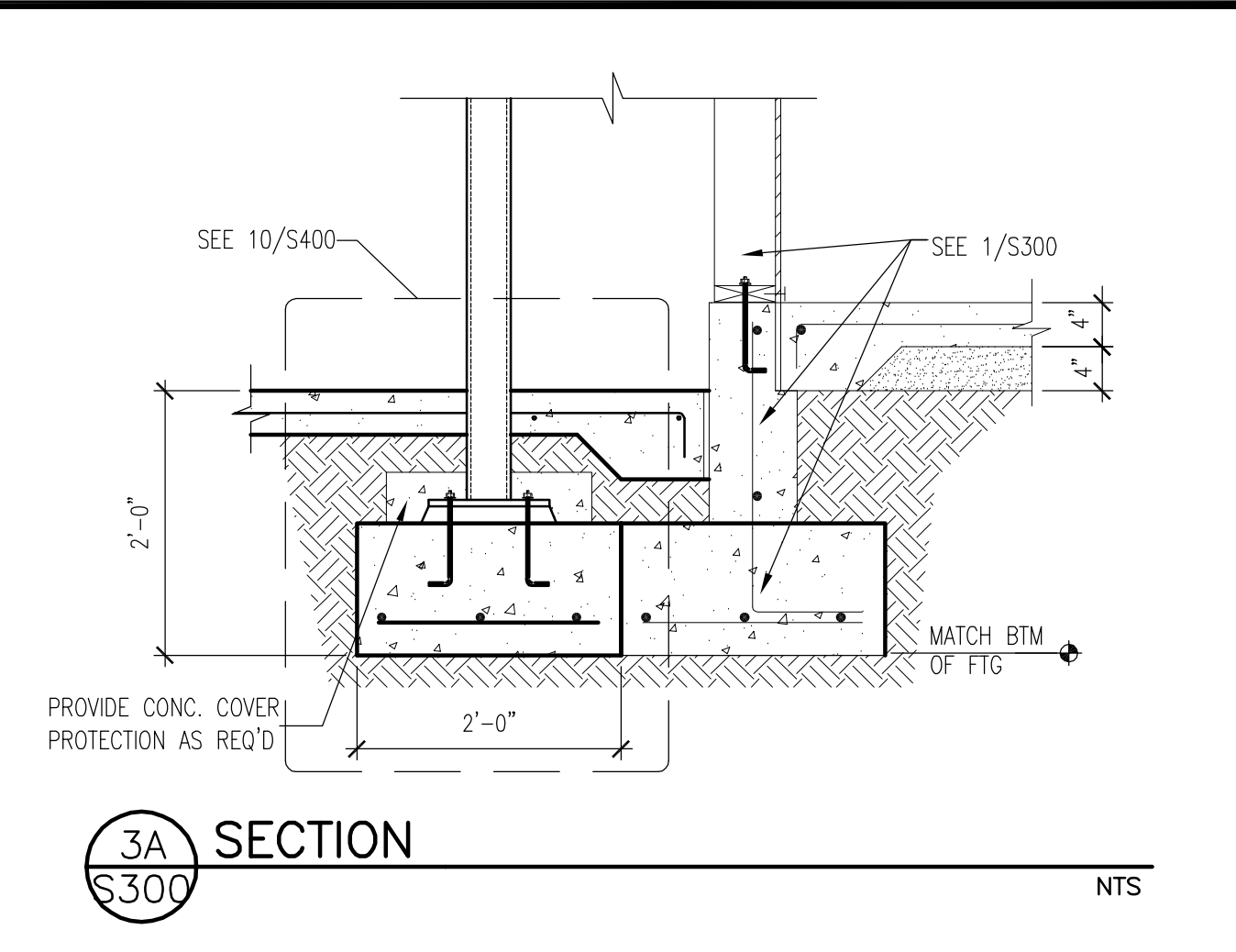
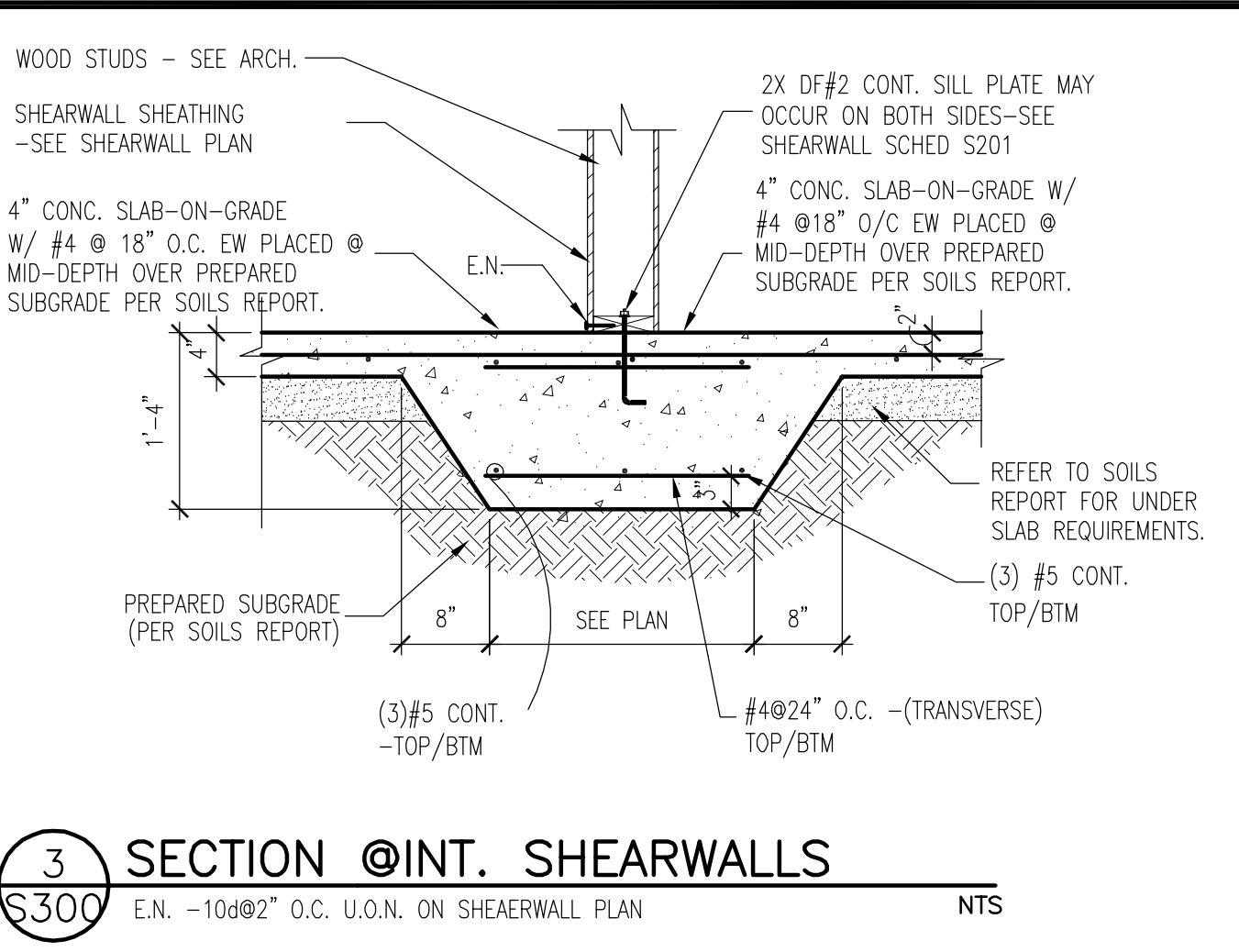
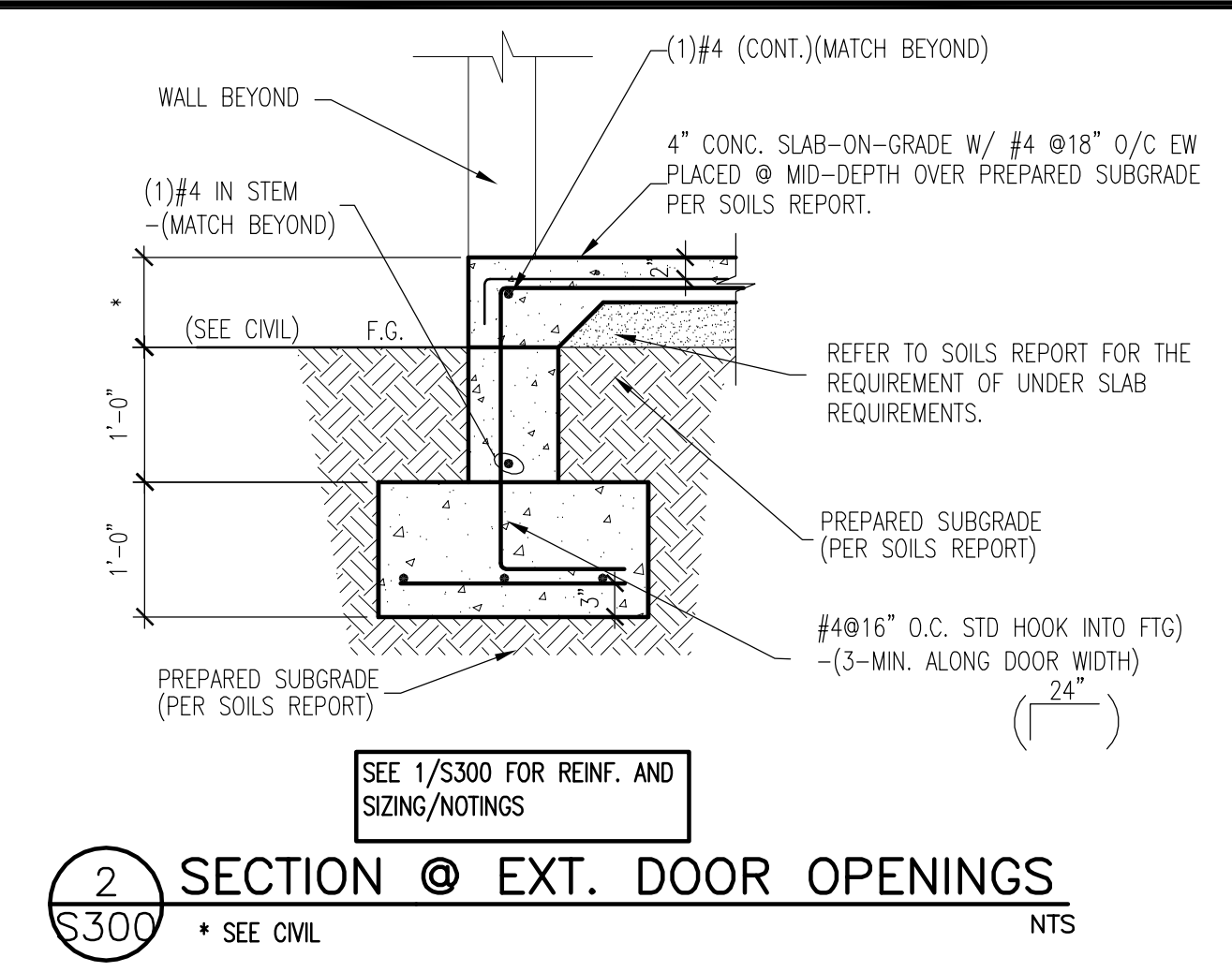
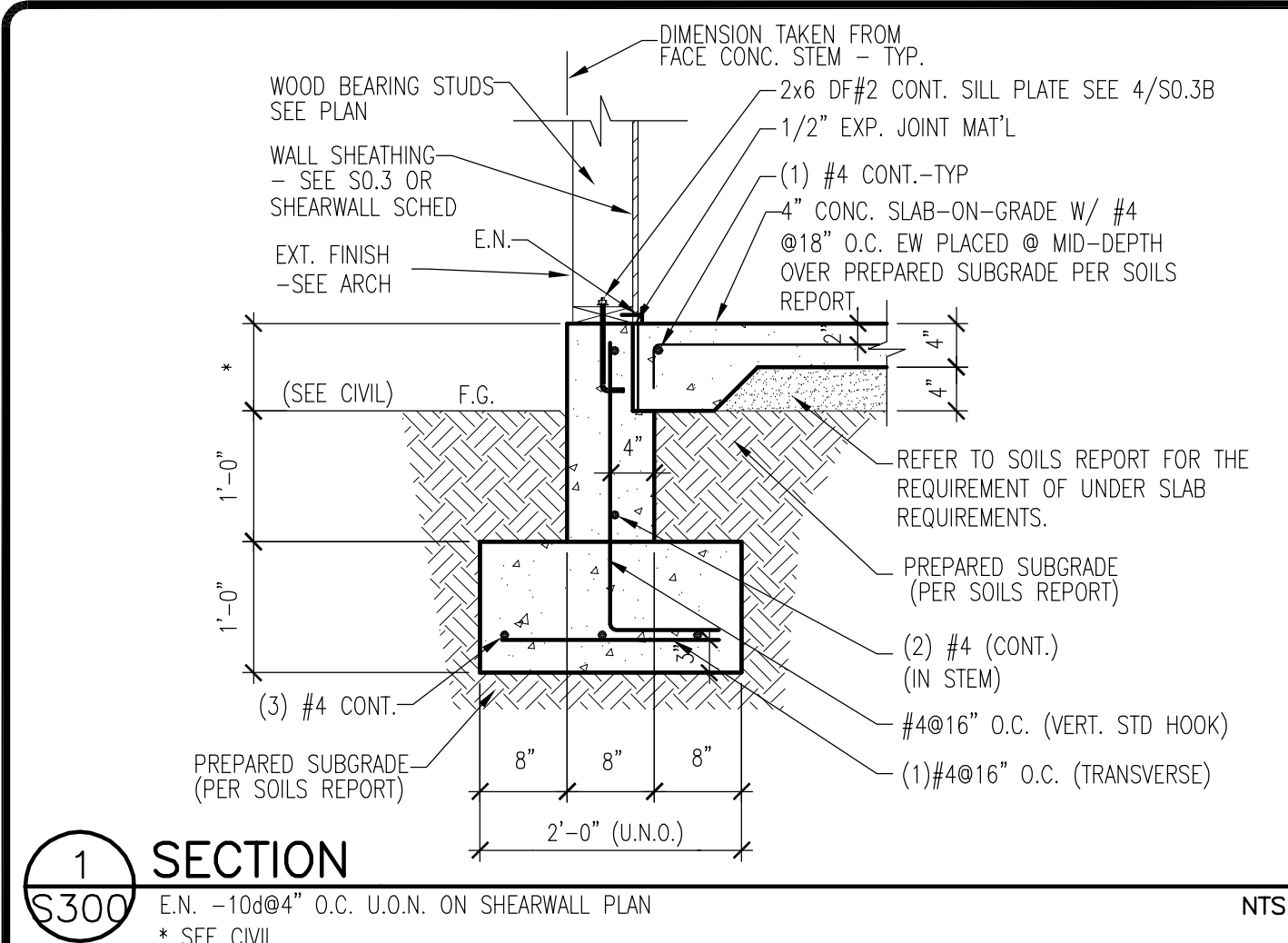
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DATE: 7/23/15
S201
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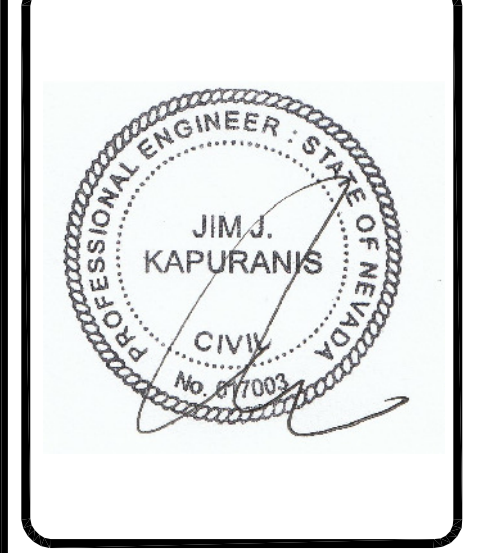
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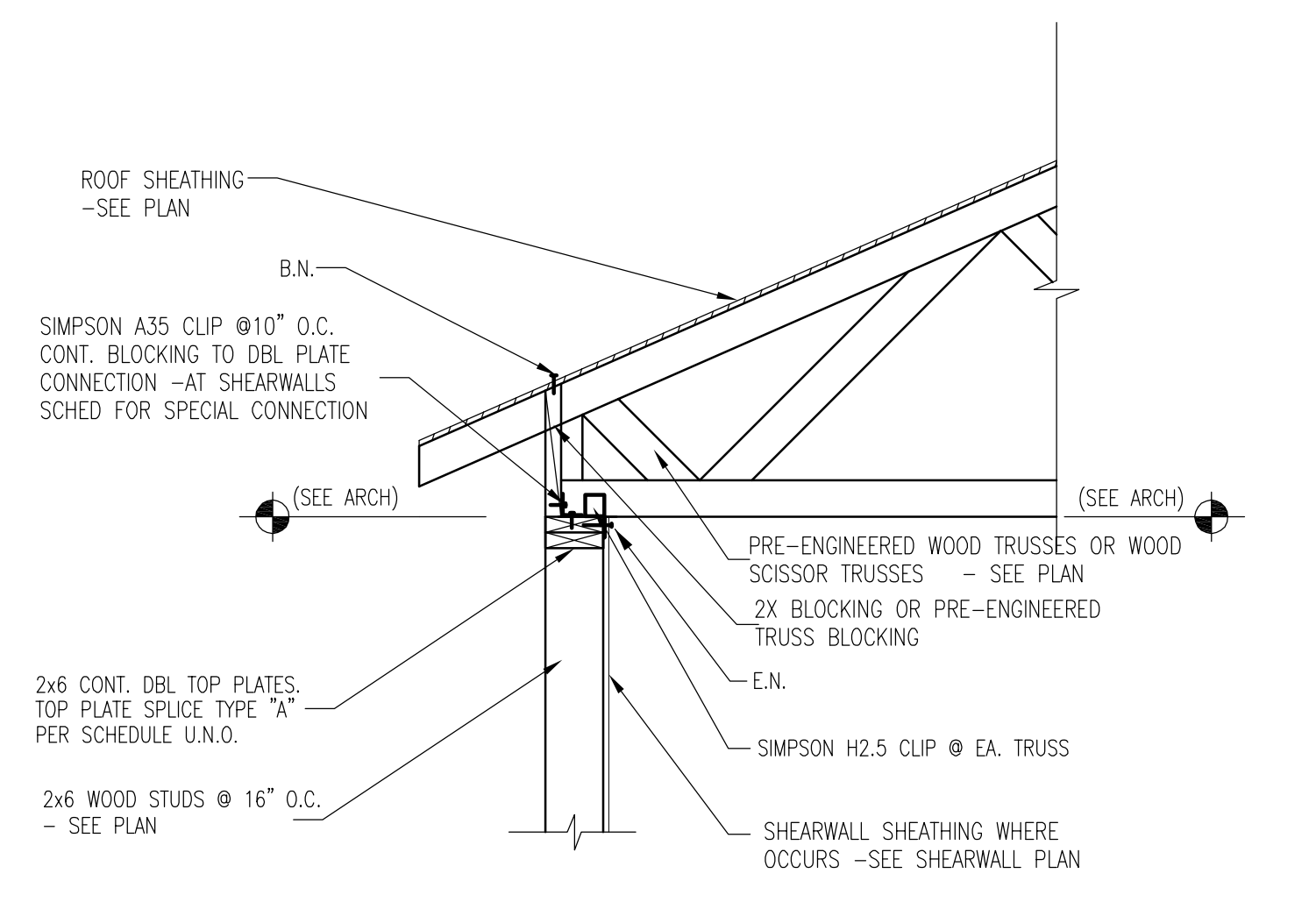
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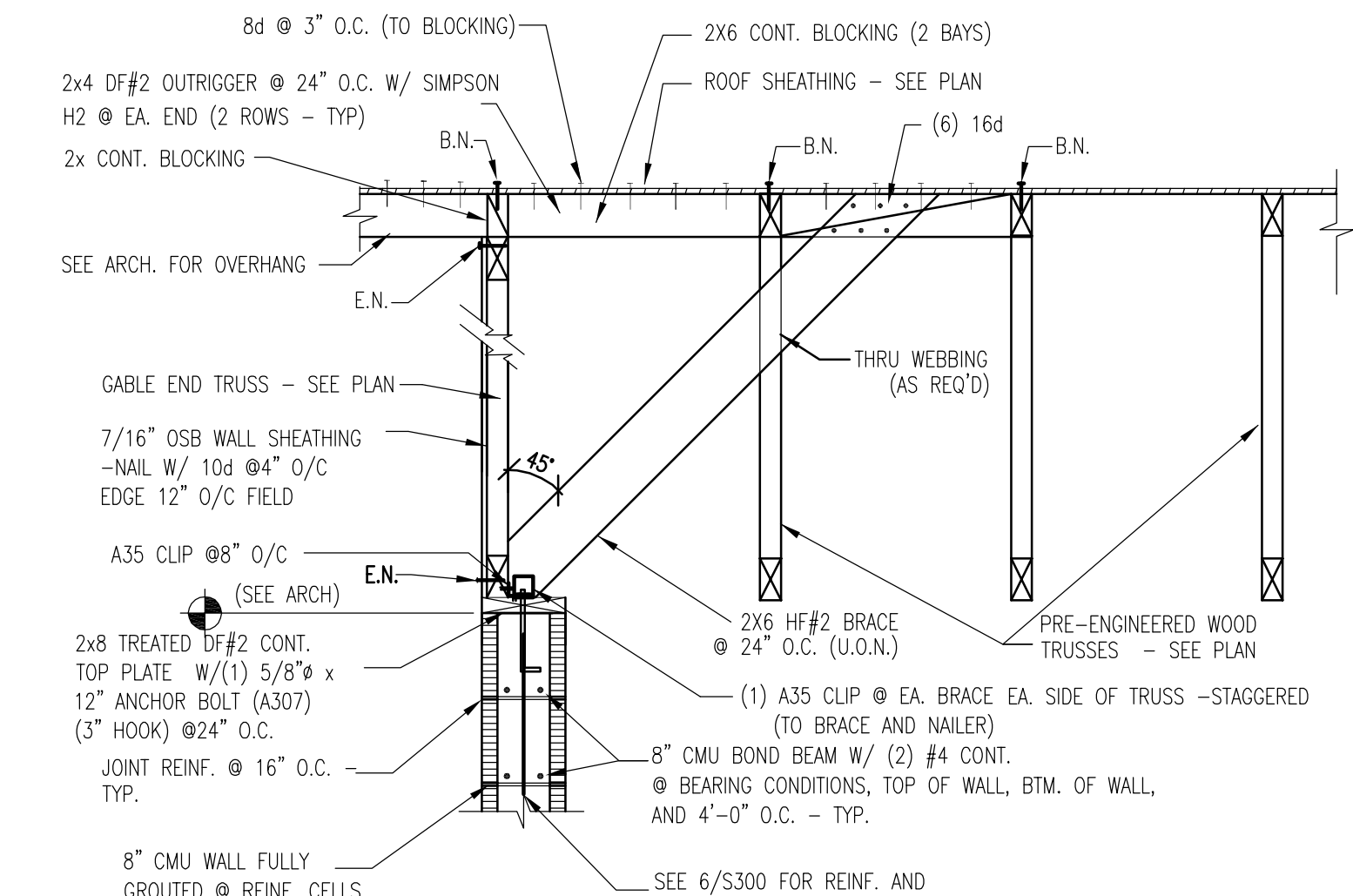
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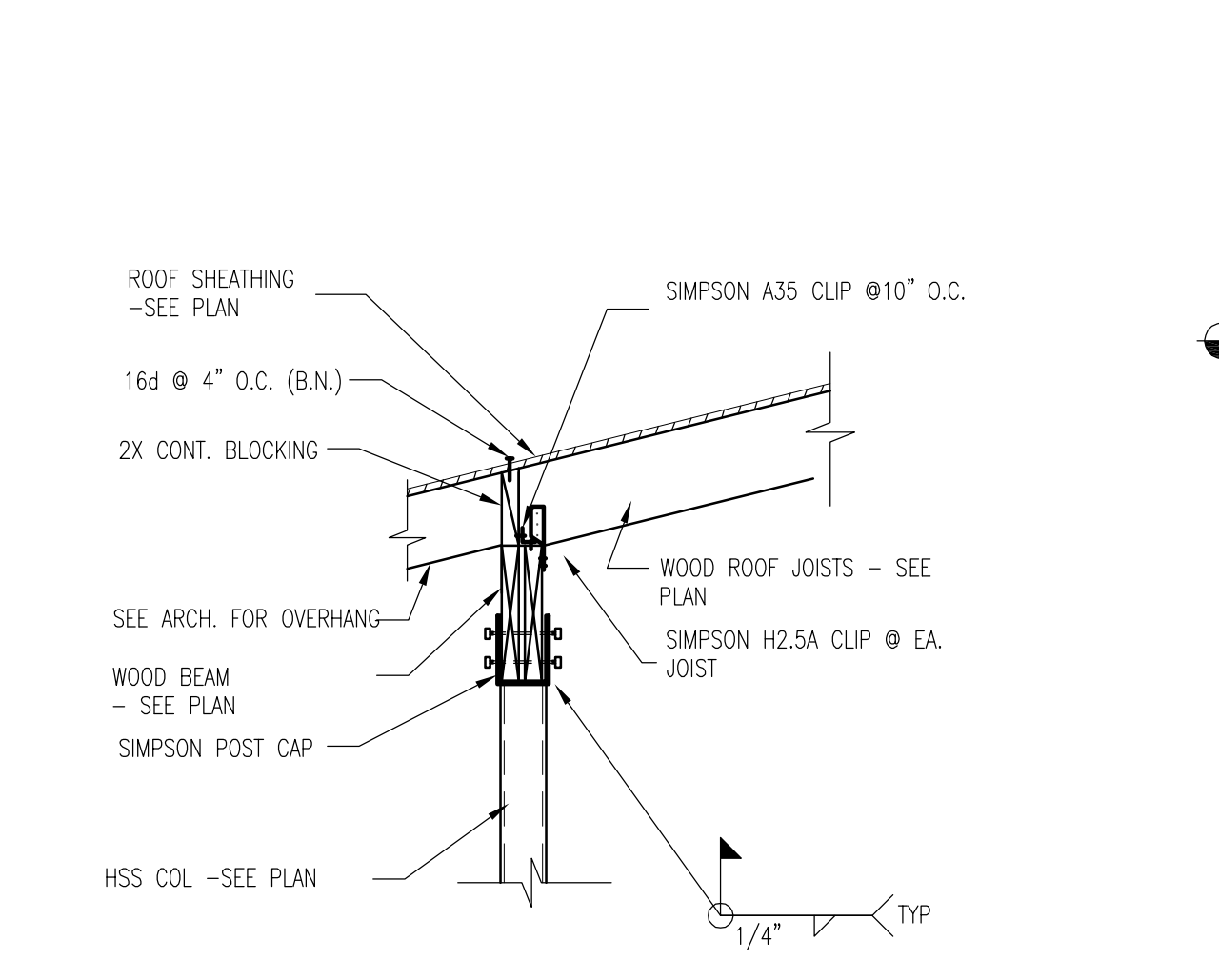
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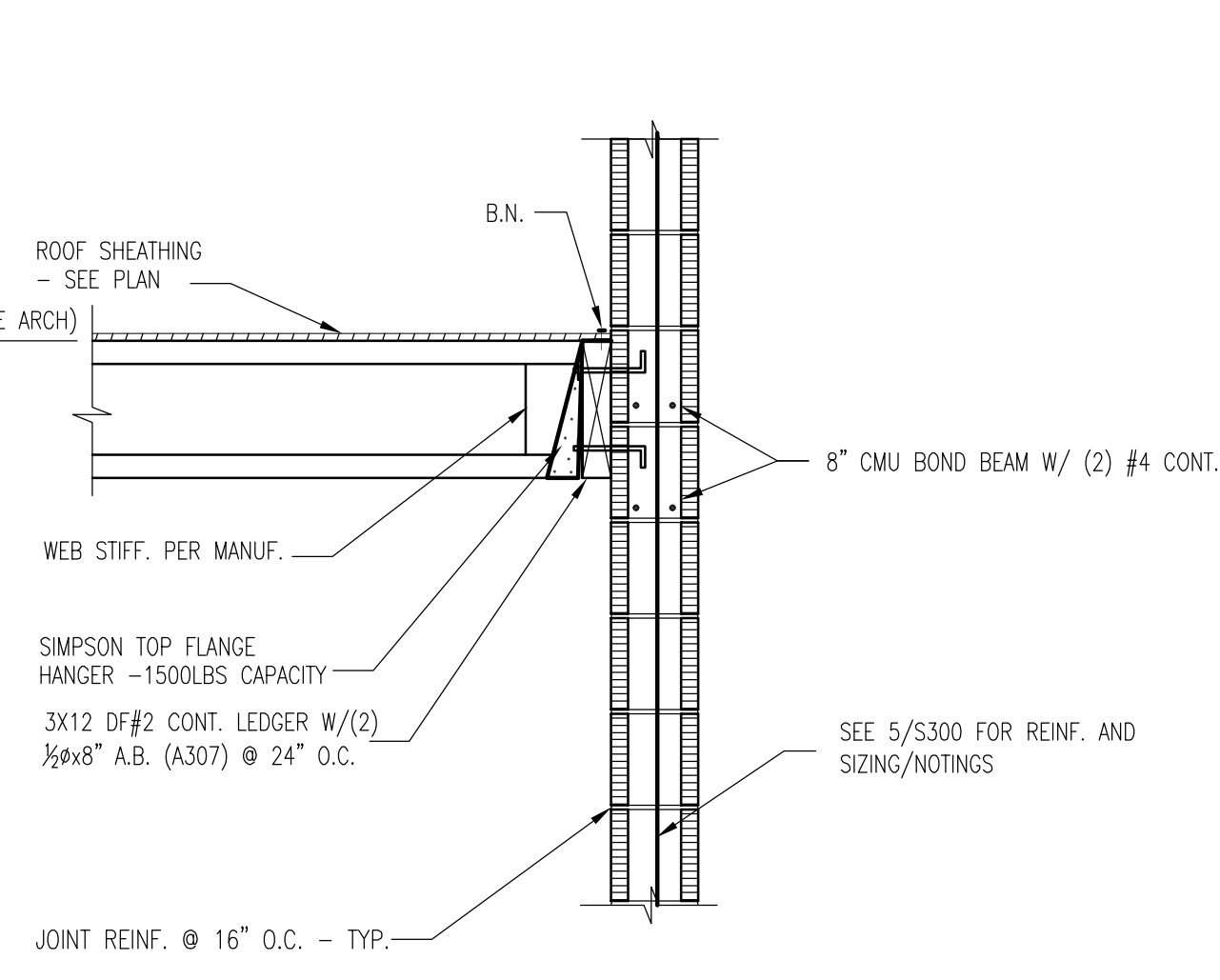
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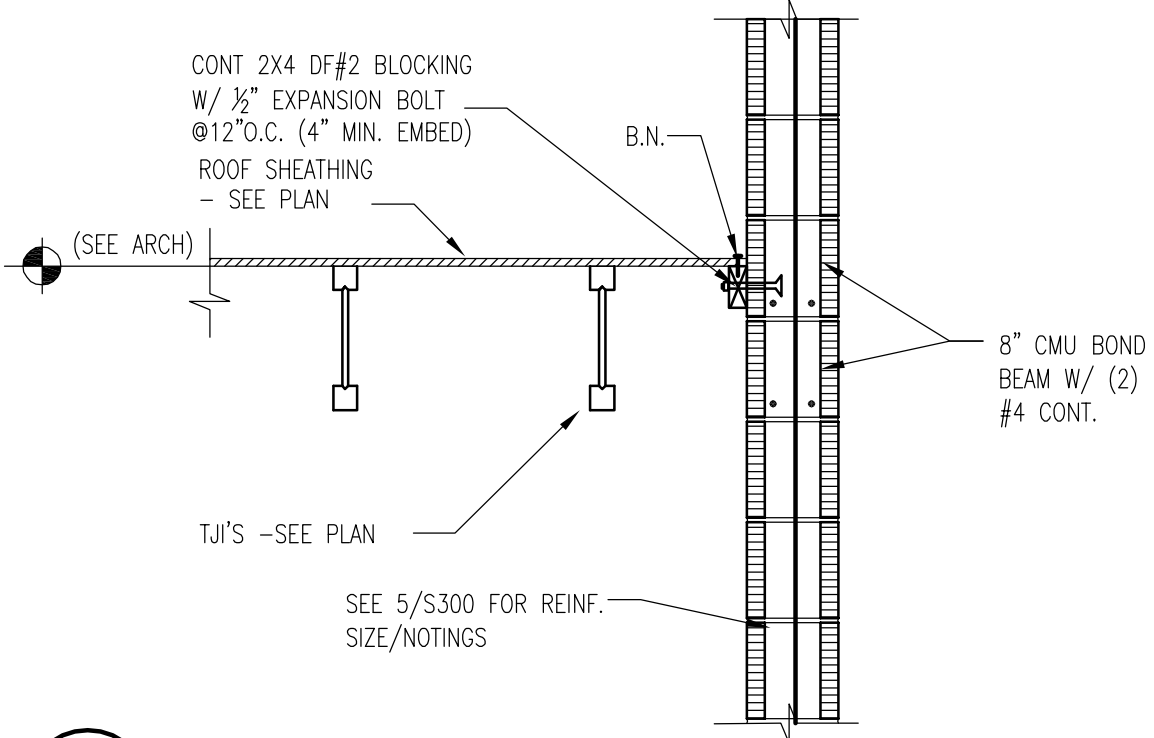
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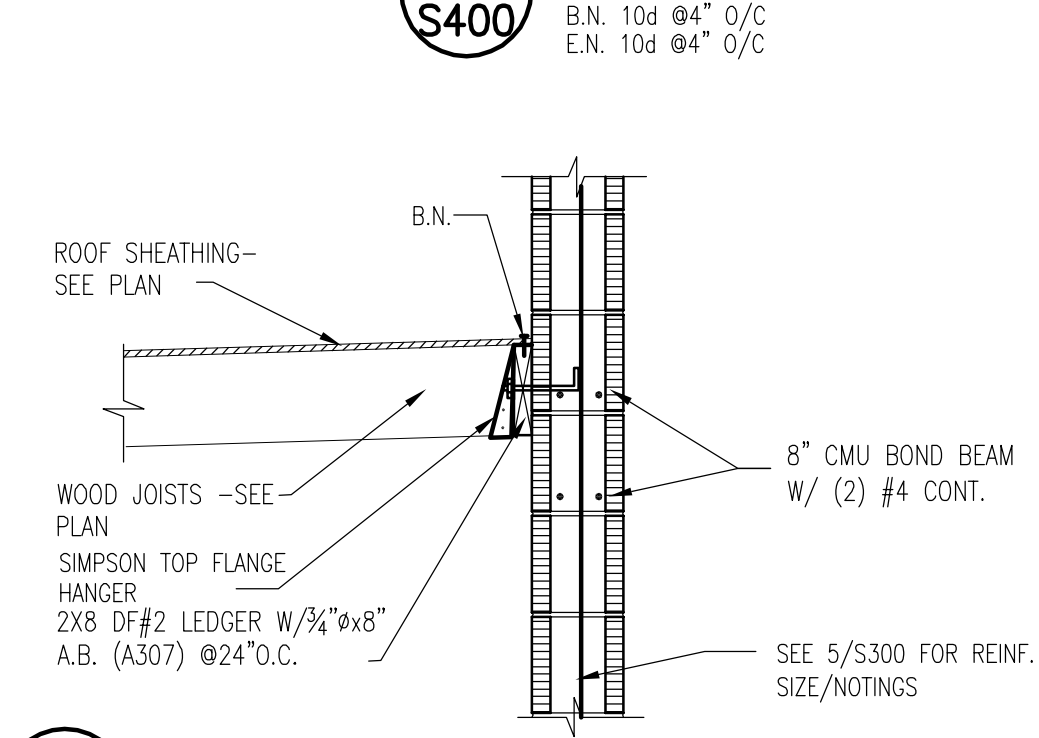
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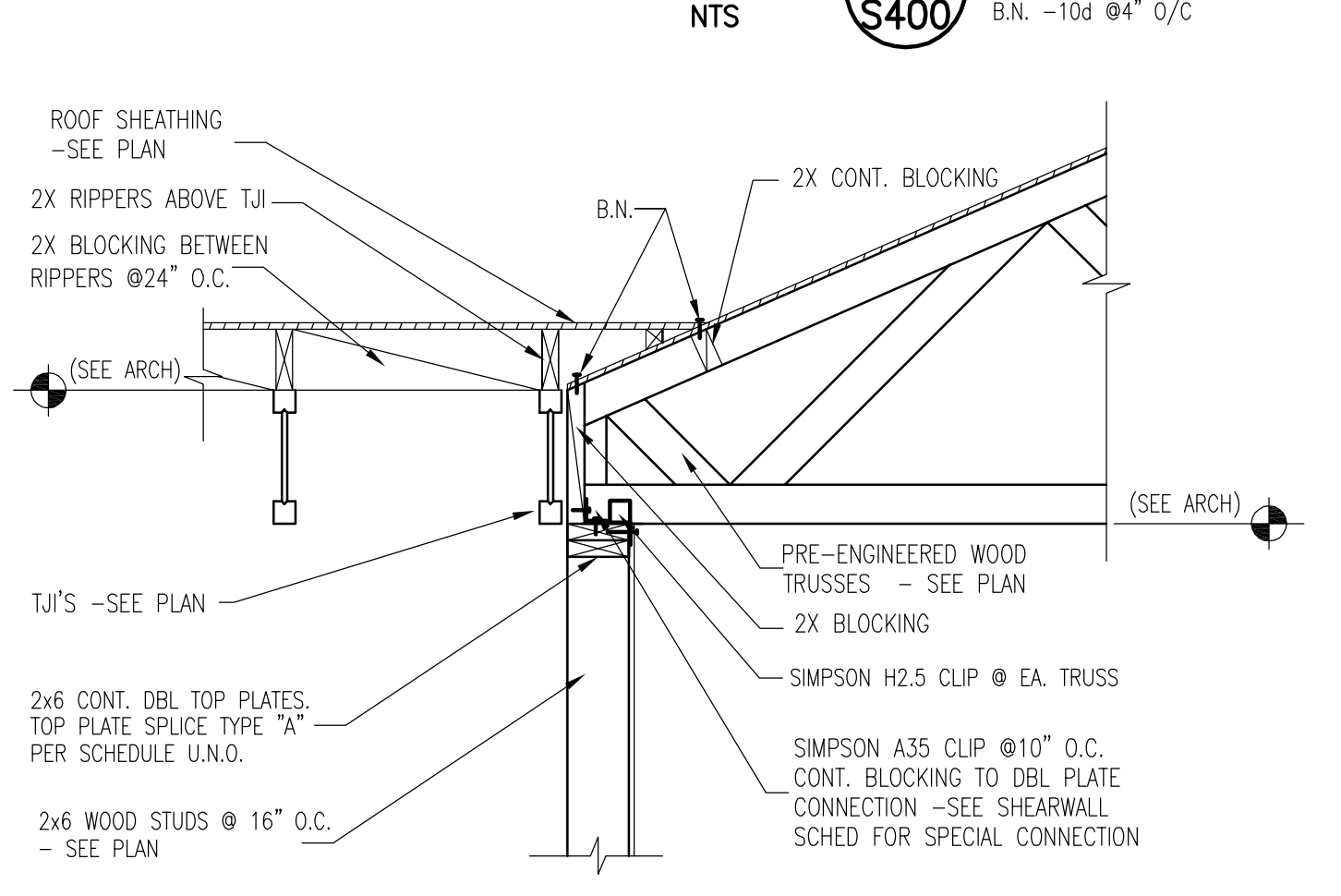
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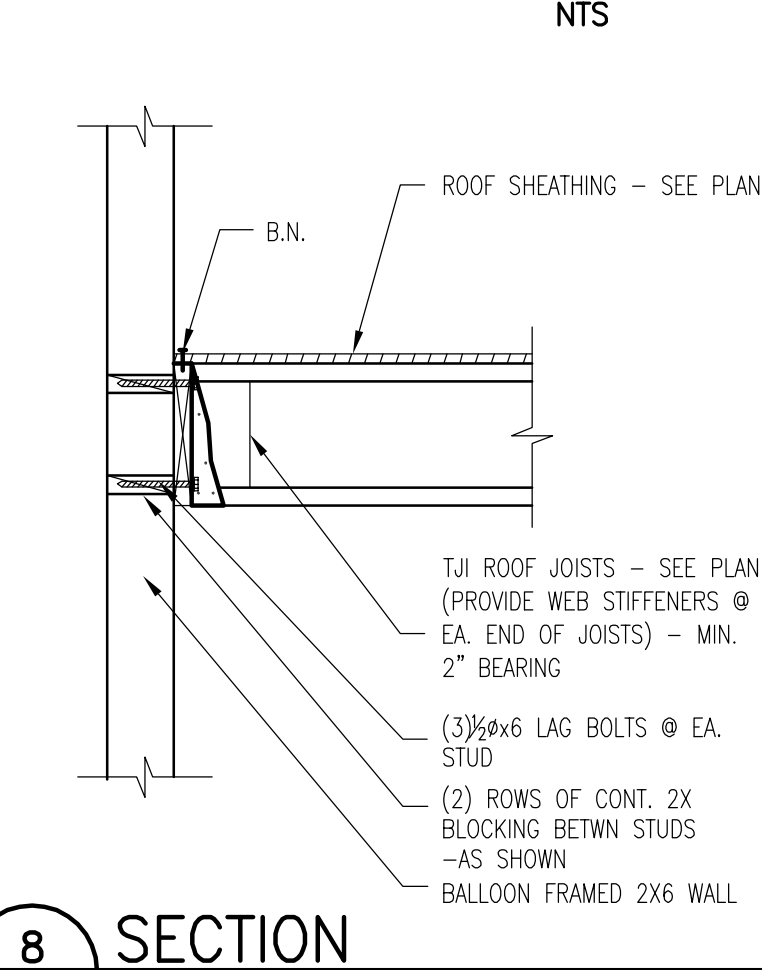
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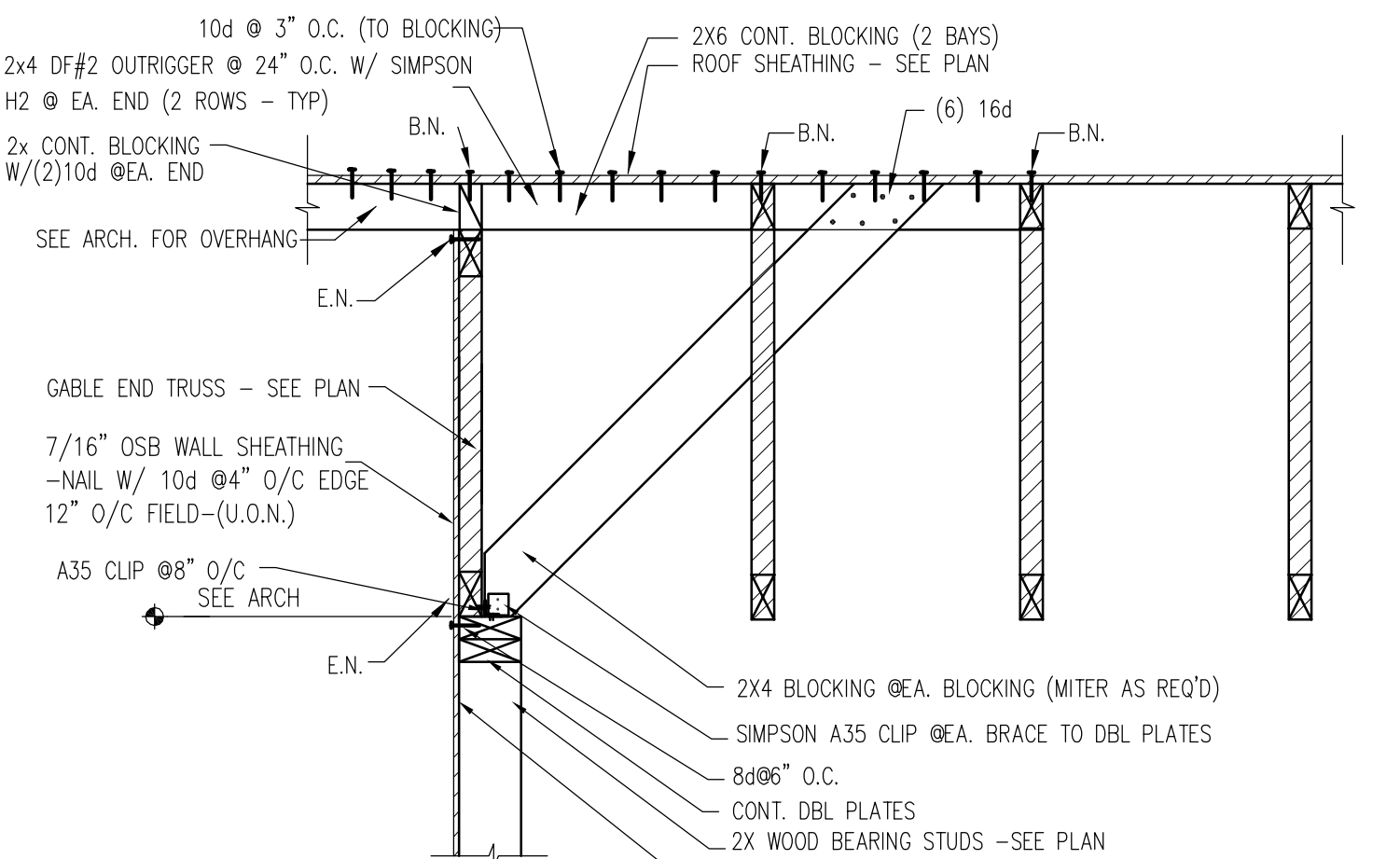
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7 SECTION
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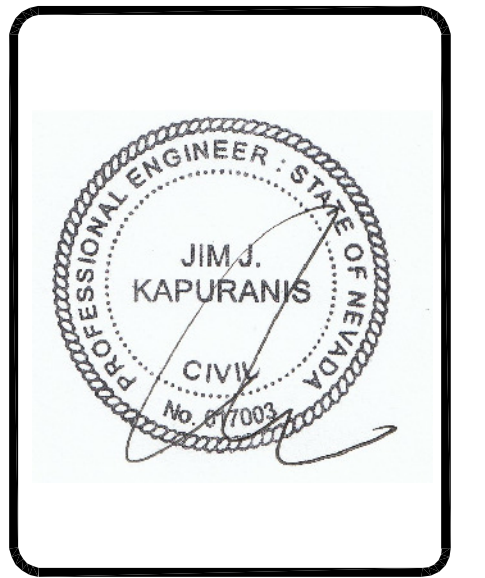
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 E.N. 10d @4" O/C NTS

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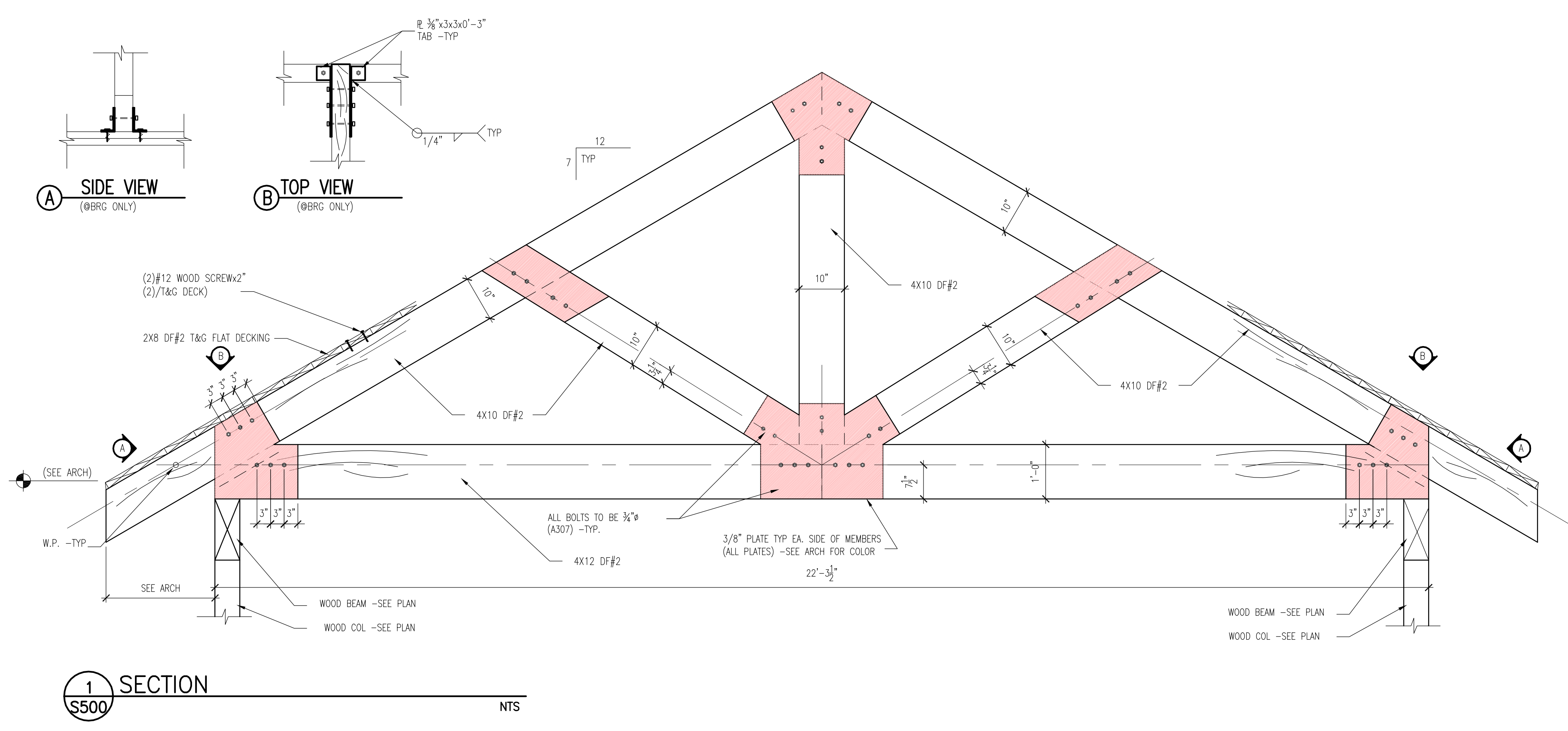
CCAS
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 DATE: 7/23/15
S400
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A SIDE VIEW
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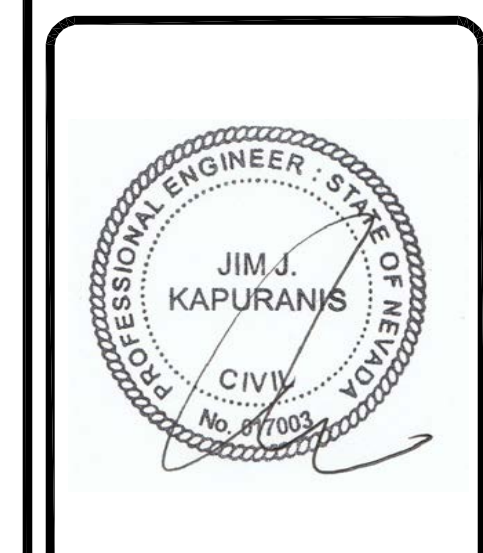
B TOP VIEW
(@BRG ONLY)

1 SECTION
S500

NTS

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